Porous media - Poromechanics Hydro-mechanical couplings

Stéphane Bonelli

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Stéphane Bonelli. Porous media - Poromechanics Hydro-mechanical couplings. France. 2019, pp.184. ⟨hal-02608596⟩

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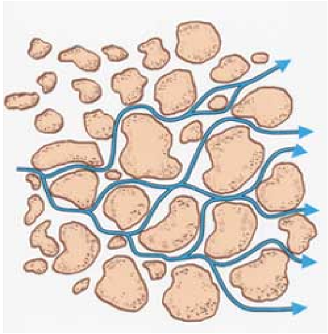
CENTRALE MARSEILLE

Milieux poreux
Poro-Mécanique
Couplages hydro-mécanique

Parcours d'approfondissement – Mécanique – M3S

Stéphane Bonelli

Milieux poreux
Poro-Mécanique
Couplages hydro-mécanique



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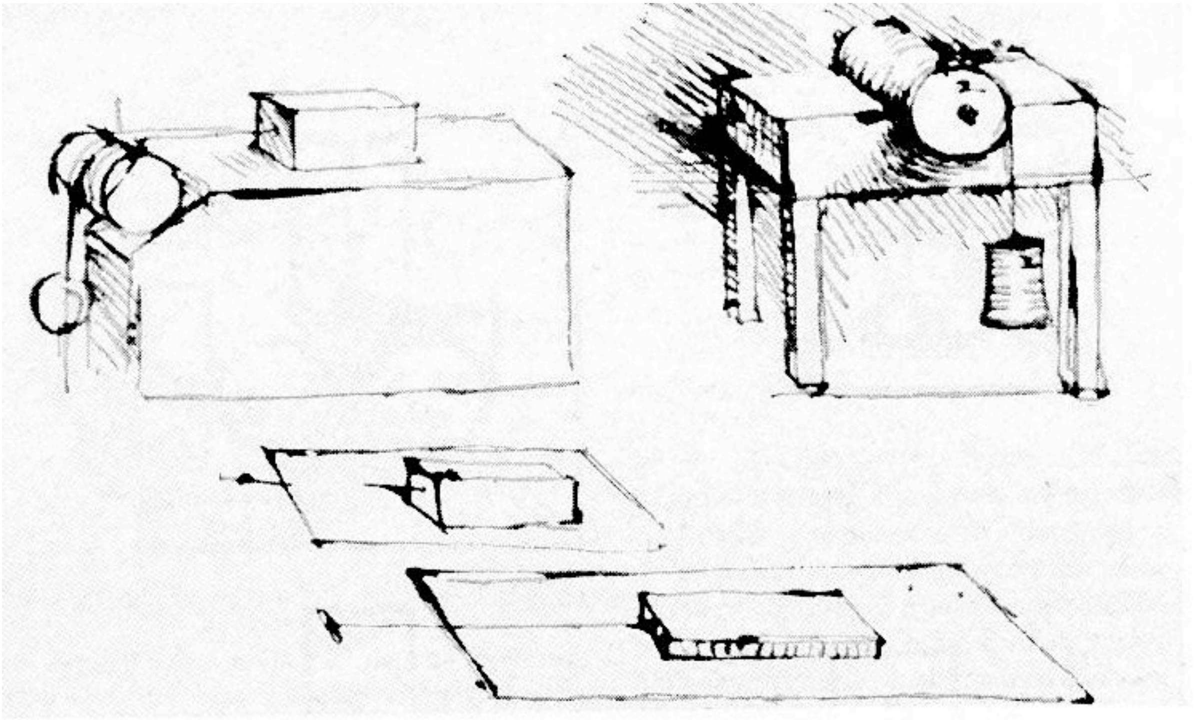
Archimède (287 av. J.-C., 212 av. J.-C.)

Expérience physique sur deux boules de métal,
l'une en or, l'autre en argent,
toutes deux plongées dans l'eau.

Théorème d'Archimède
Tout corps plongé dans un fluide, entièrement mouillé par celui-ci ou traversant sa surface libre, subit une force verticale, dirigée de bas en haut et égale au poids du volume de fluide déplacé ; cette force est appelée « poussée d'Archimède ».
(ce théorème fut ensuite démontré au XVIe siècle).

De Vinci (1452, 1519)



Machine à étudier les frottements de De Vinci (1508)

Galilée (1564, 1642)

DISCORSI
E
DIMOSTRAZIONI
MATEMATICHE,
intorno à due nuoue scienze
Attenenti alla
MECANICA & i MOVIMENTI LOCALI;
del Signor
GALILEO GALILEI LINCEO,
Filosofo e Matematico primario del Sereniſſimo
Grand Duca di Toscana.
Con vna Appendice del centro di grauità d'alcuni Solidi.

IN LEIDA.
Appreſſo gli Elsevirii. M. D. C. XXXVIII.

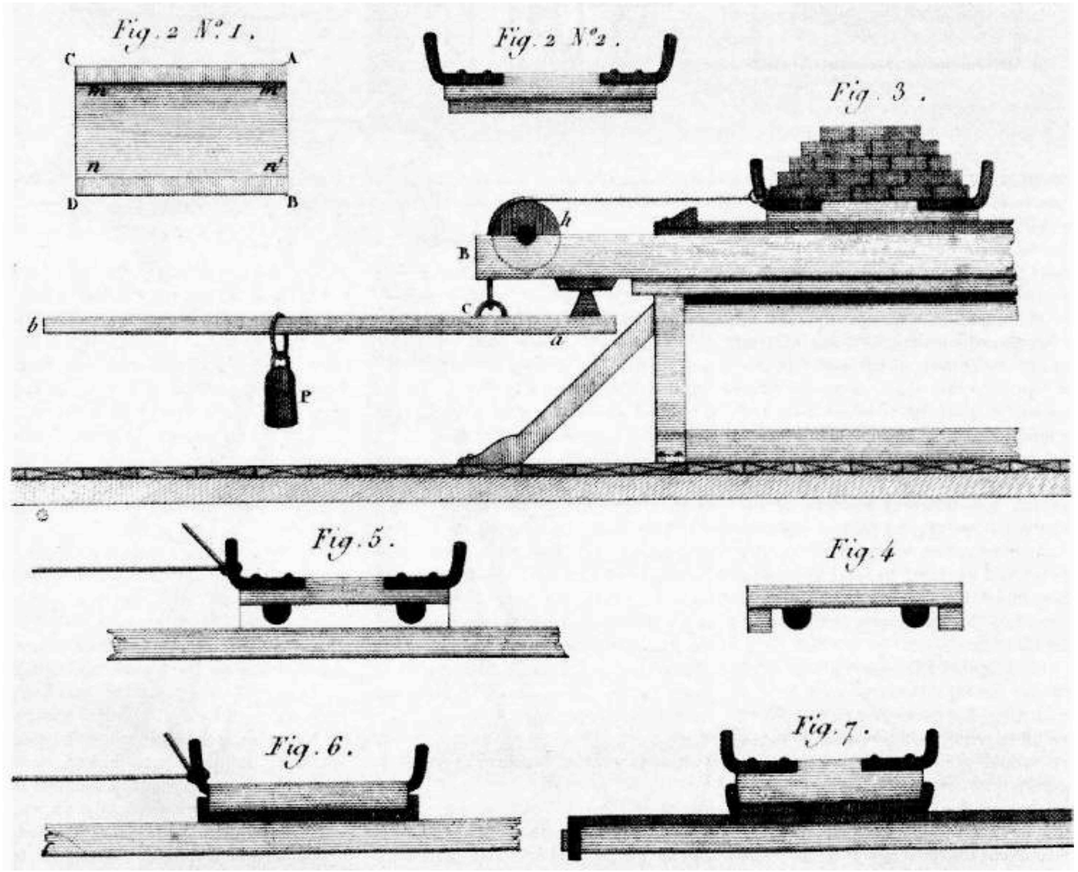
Title of the famous book of Galileo (1638) which founded mechanics of materials

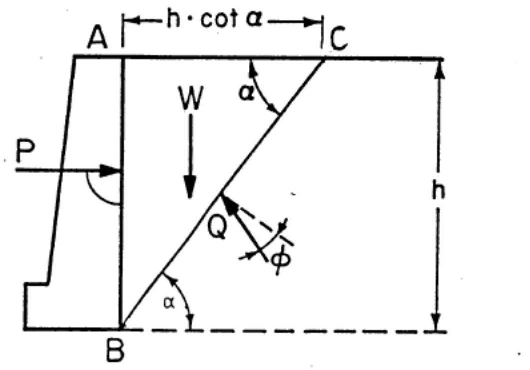
114 DIALOGO SECONDO

fin qui dichiarate, non sarà difficile l'intender la ragione, onde auuenga, che vn Prisma, ò Cilindro solido di vetro, acciaio, legno, ò altra materia frangibile, che sospeso per lungo sosterrà grauissimo peso, che gli sia attaccato, mà in trauerso (come poco fa diceuamo) da minor peso assai potrà tal volta essere spezzato, secondo che la sua lunghezza eccederà la sua grossezza. Imperò che figuriamoci il Prisma solido AB, CD *fitto in vn muro dalla parte* AB, *e nell'altra estremità s'intenda la forza del Peso* E. *(intendendo sempre il muro esser eretto all'Orizonte, & il Prisma, ò Cilindro fitto nel muro ad angoli retti) è manifesto che douendosi spezzare si romperà nel luogo* B, *doue il taglio del muro serue per sostegno, e la* BC *per la parte della Leua, doue si pone la forza, e la grossezza del solido* BA *è l'altra parte della Leua, nella quale è posta la resistenza, che consiste nello staccamento, che s'hà da fare della parte del solido* BD, *che è fuor del muro, da quella che è dentro; e per le cose dichiarate il momento della forza posta in* C *al momento della resistenza che stà nella*

GALILEI (1638)

Coulomb (1736, 1806)





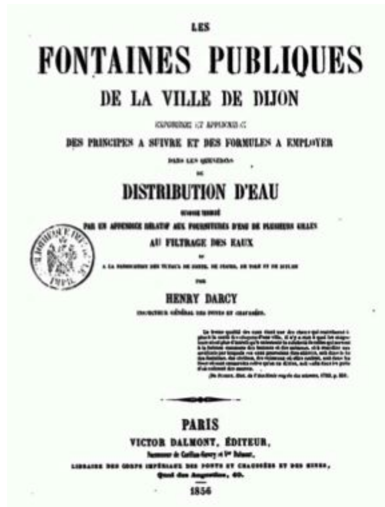
C.A. COULOMB, 1773

$$\alpha = \frac{\pi}{4} + \frac{\phi}{2};$$

$$P = \frac{1}{2}\rho g h^2 (1 - \sin\phi)/(1 + \sin\phi)$$

Machine à étudier les frottements de Coulomb

Darcy (1803, 1858)



LES

FONTAINES PUBLIQUES

DE LA VILLE DE DIJON

DES PRINCIPES A SUIVRE ET DES FORMULES A EMPLOYER

DISTRIBUTION D'EAU

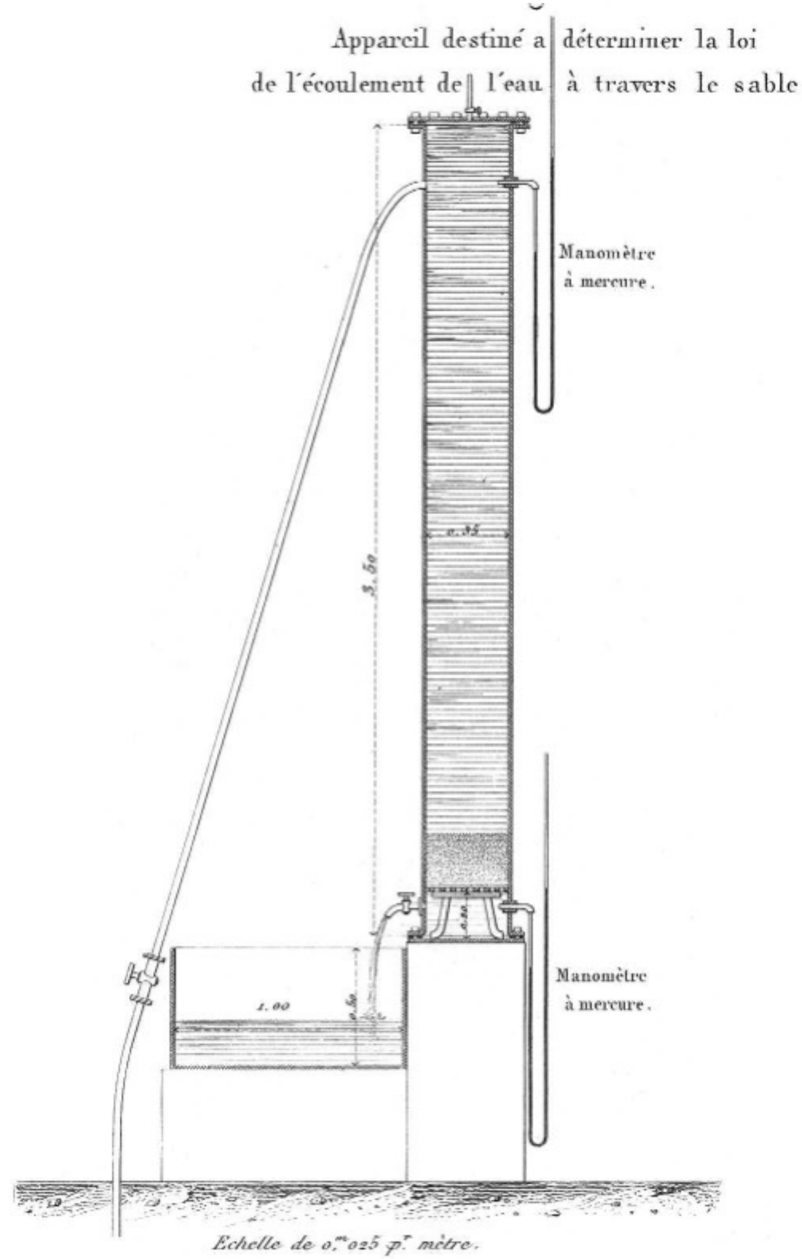
AU FILTRAGE DES EAUX

HENRY DARCY

PARIS

VICTOR DALMONT, ÉDITEUR,

1856



Appareil destiné a déterminer
la loi de l'écoulement de l'eau à travers le sable
de Darcy.

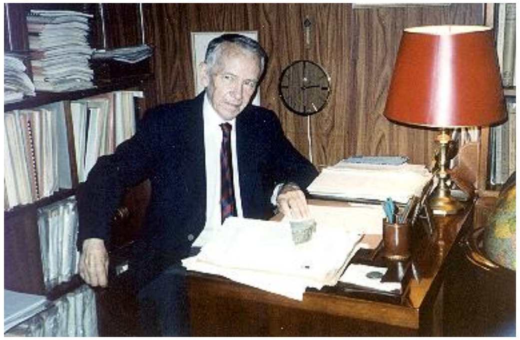
Terzaghi (1883, 1963)

Karl von Terzaghi first proposed the relationship for effective stress in 1936.

$$\sigma' = \sigma - p$$

For him, the term ‘effective’ meant the calculated stress
that was effective in moving soil, or causing displacements.
It represents the average stress carried by the soil skeleton.

Biot (1905, 1985)

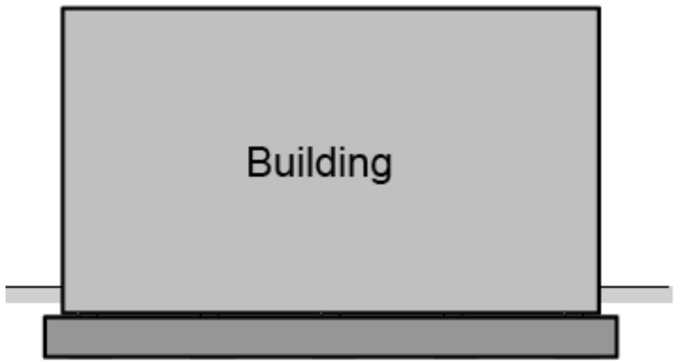


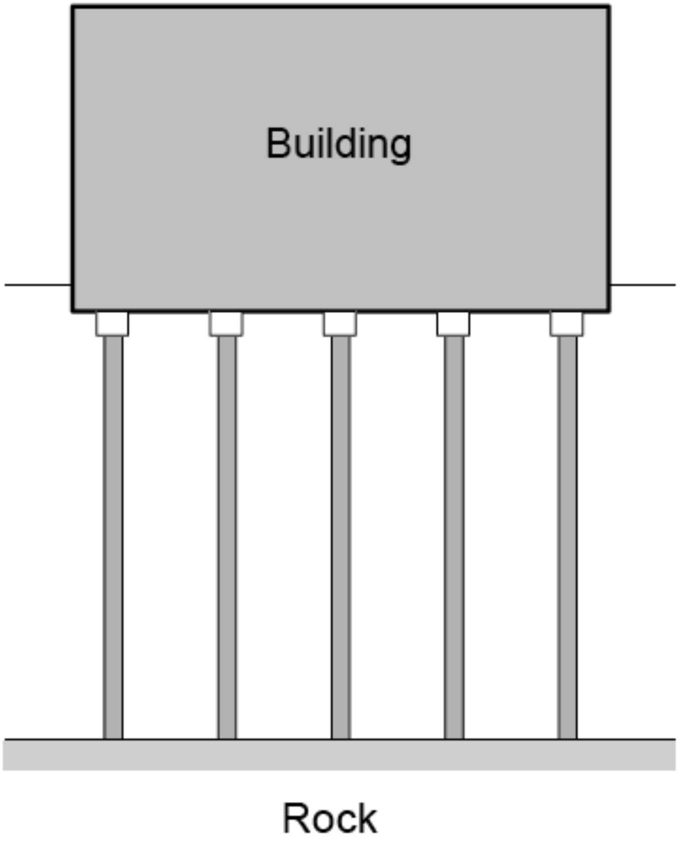
A theory for acoustic propagation in a porous and elastic medium developed by M.A. Biot.
Compressional and shear velocities can be calculated by standard elastic theory from the composite density, shear and bulk modulus of the total rock.
The problem is how to determine these from the properties of the constituent parts.
Biot showed that the composite properties could be determined from the porosity and the elastic properties (density and moduli) of the fluid, the solid material, and the empty rock skeleton, or framework.
To account for different frequencies of propagation, it is also necessary to know the frequency, the permeability of the rock, the viscosity of the fluid and a coefficient for the inertial drag between skeleton and fluid.

❑ WHAT PROBLEMS DOES IT ADDRESS?

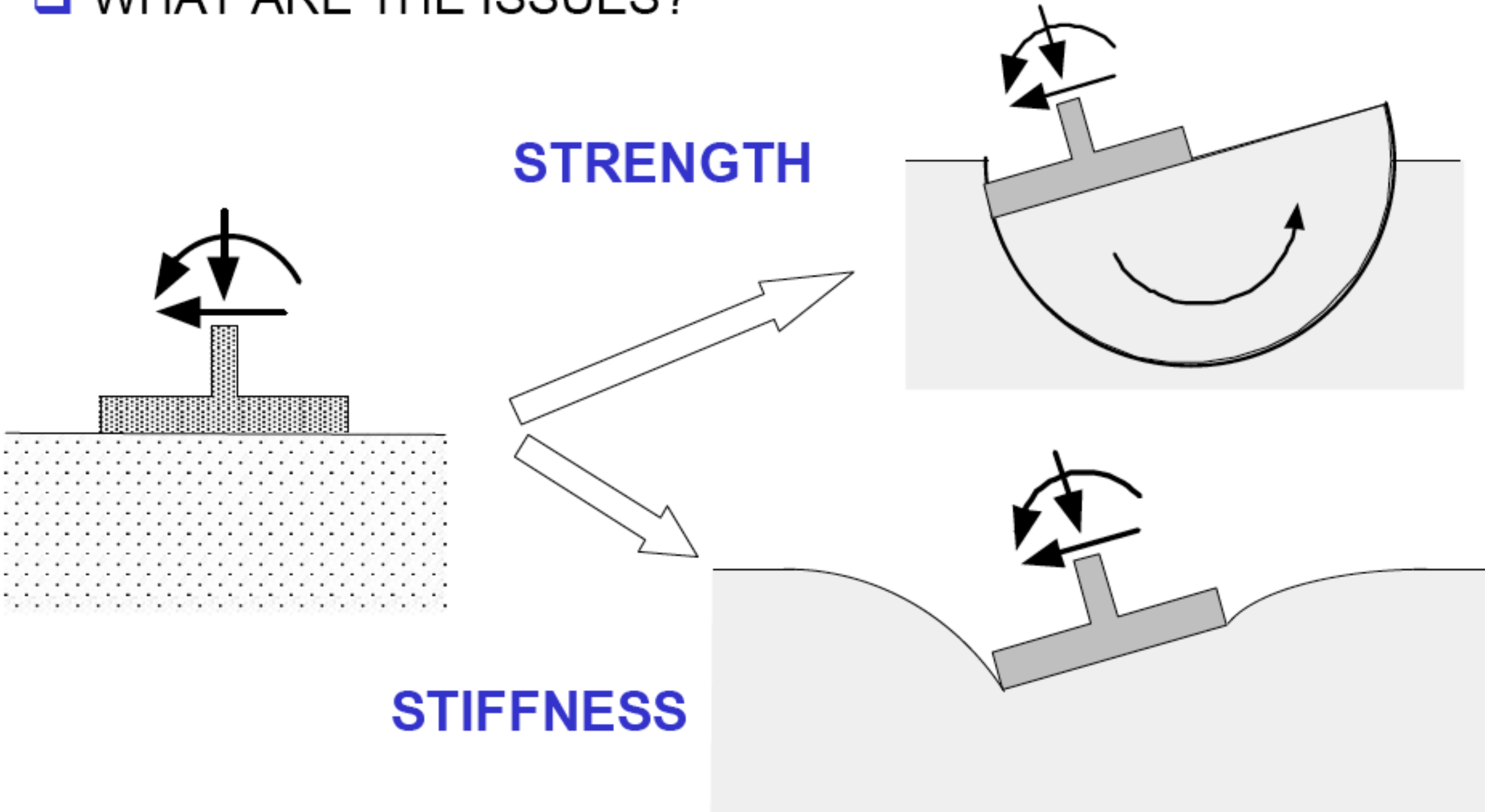
FOUNDATIONS

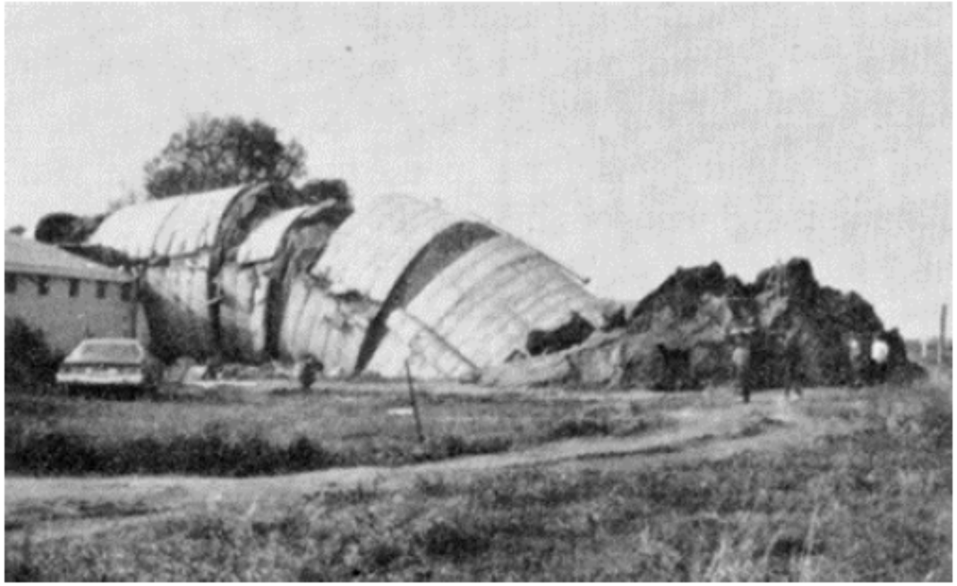
SHALLOW vs. DEEP



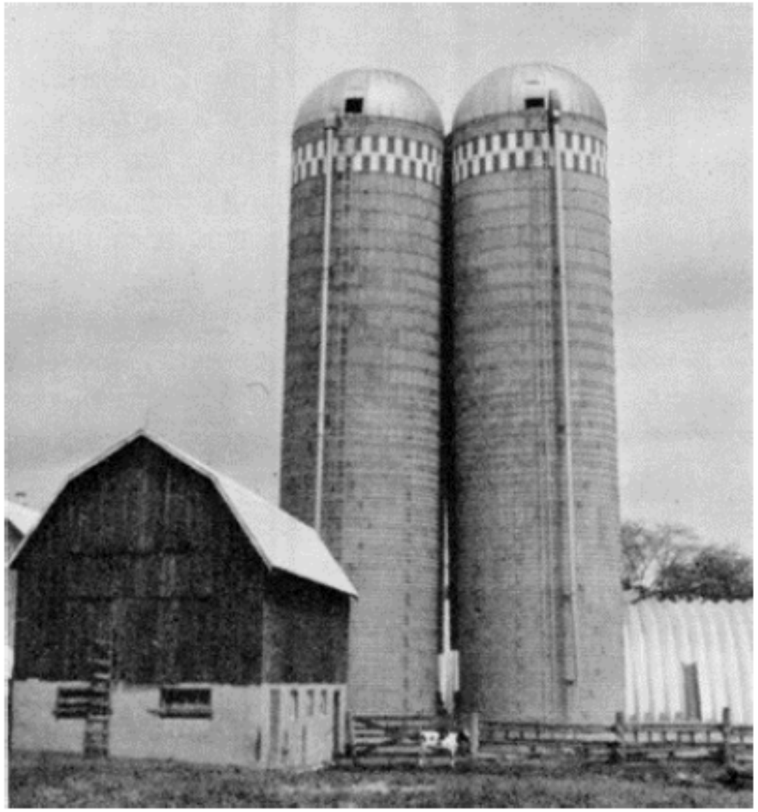


- WHAT ARE THE ISSUES?



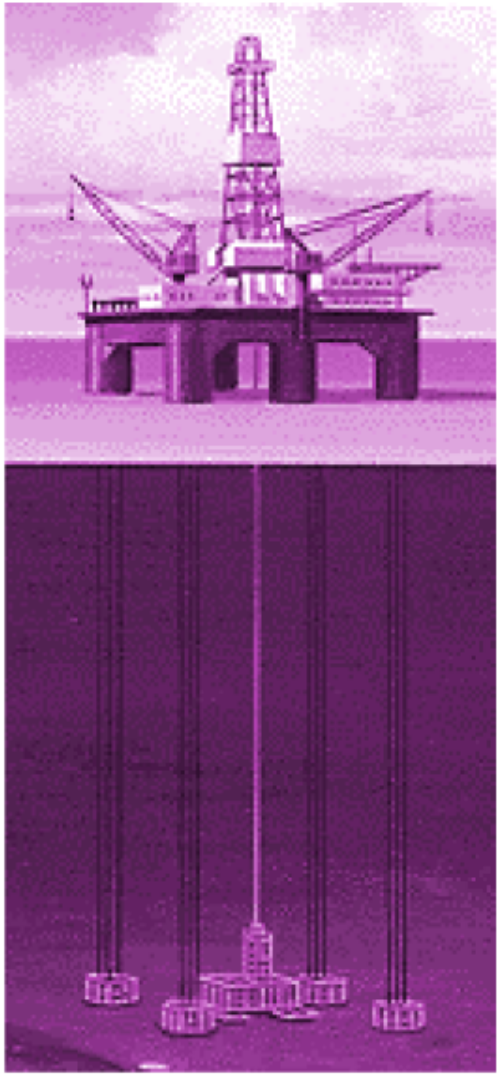


STRENGTH



STIFFNESS

Including ones on the sea bottom

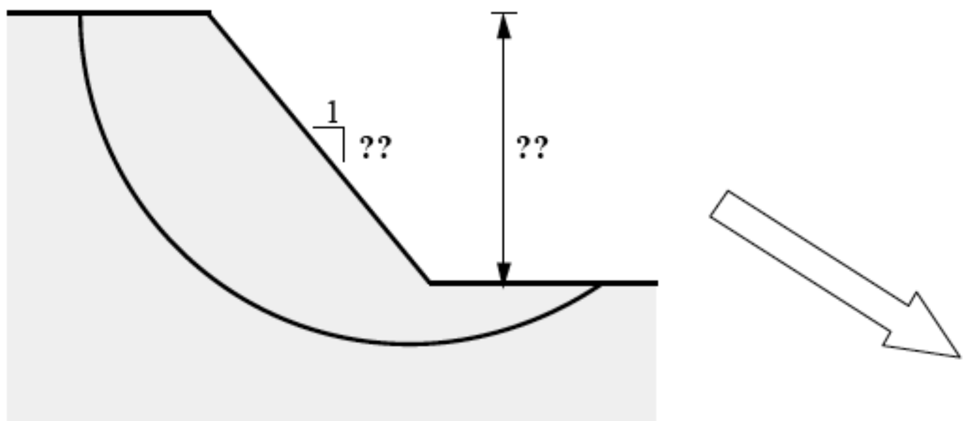


FOUNDATIONS

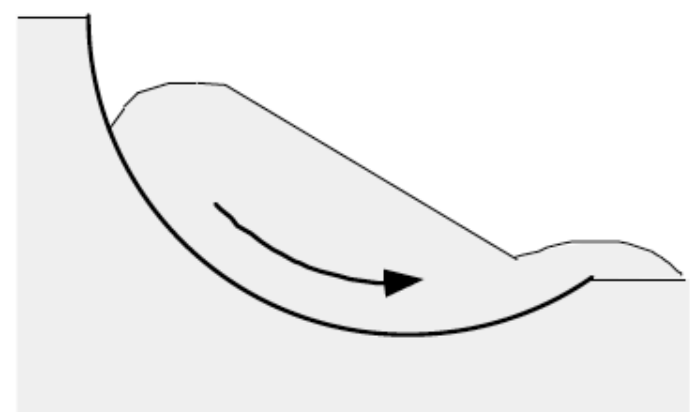
Metro Link Light Rail Stations,
St. Louis, MO

California Palace of the
Legion of Honor,
San Francisco, CA

❑ WHAT ARE THE ISSUES?



STABILITY



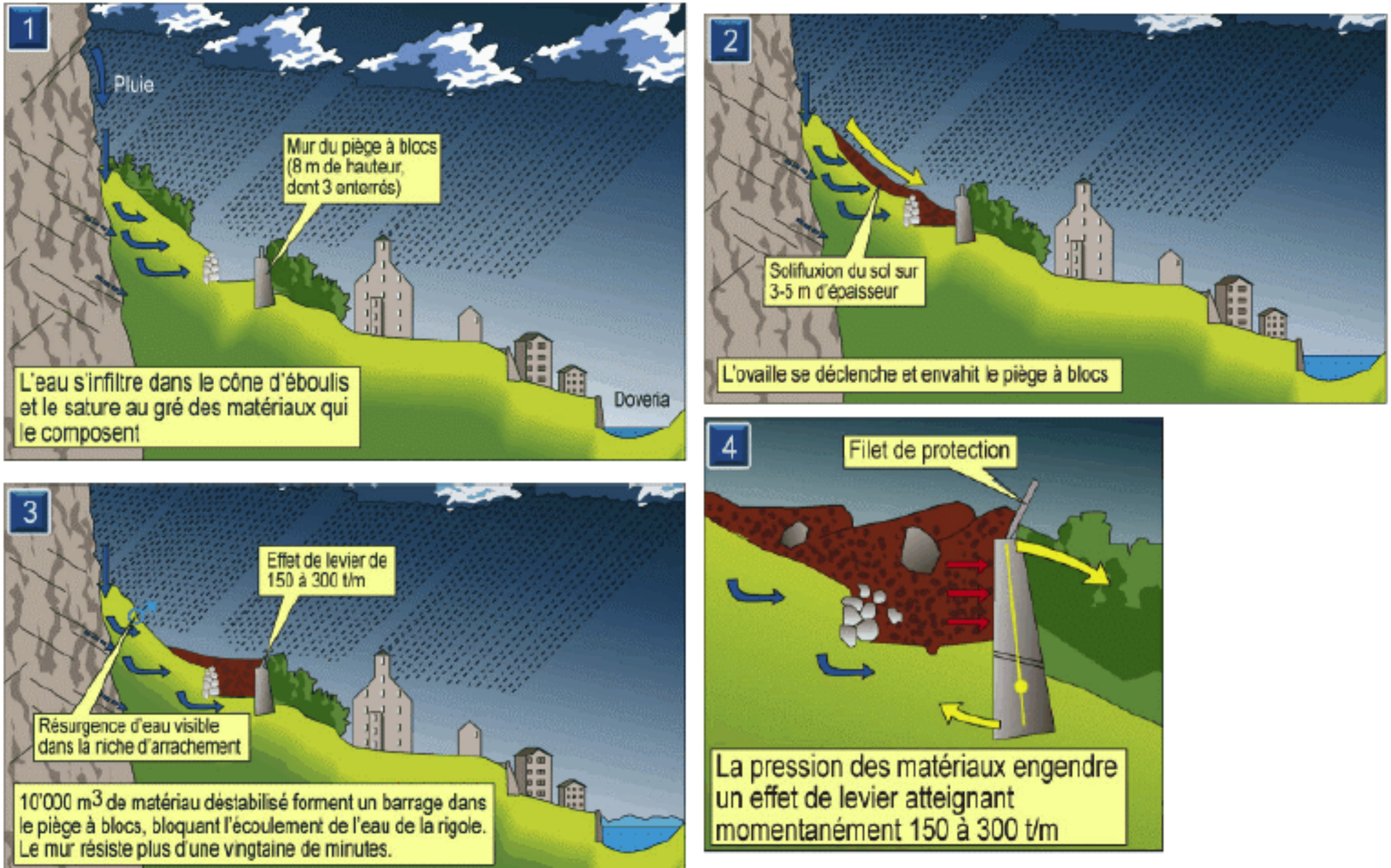
Jizukiyama Landslide, Japan, 1985

Vagnhärad Landslide, Sweden, 1997

Gondo (VS): avant, après

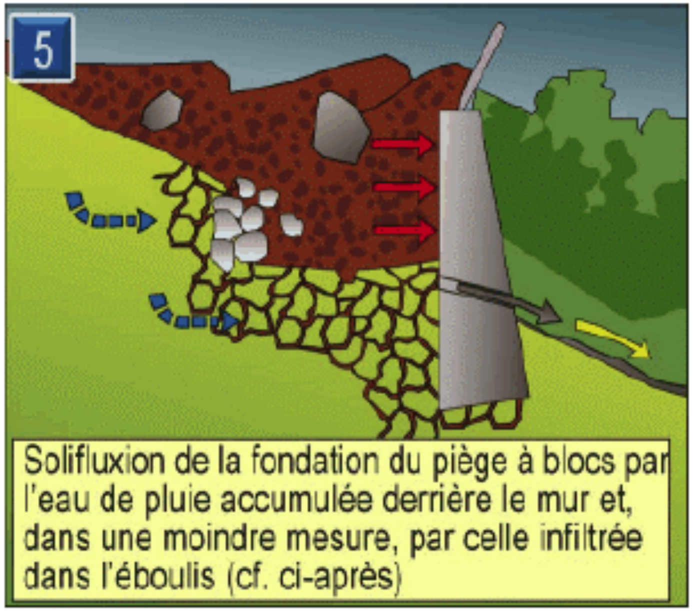
Photo L'Illustré

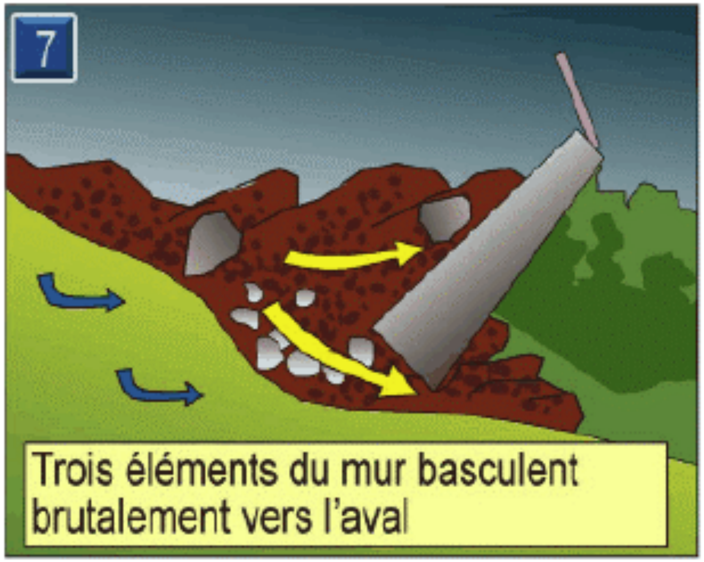
Gondo: mécanisme possible (source: CREALP, SION)

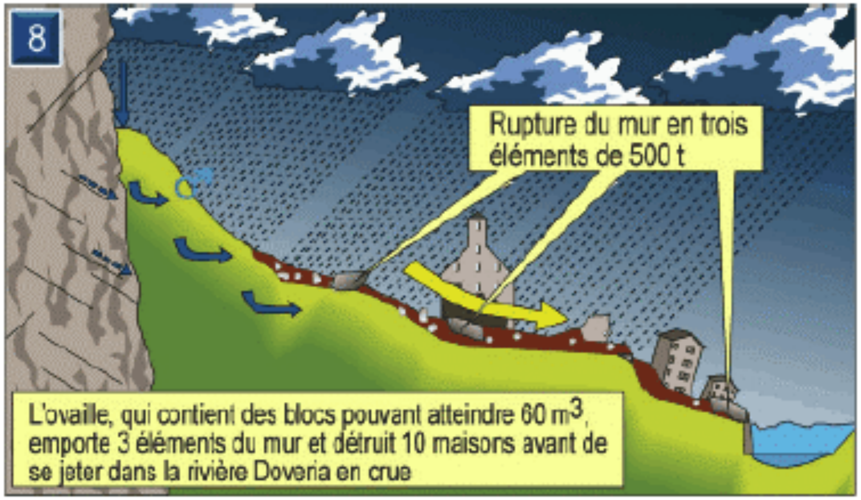


http://www.crealp.ch/f_principal.html

Gondo: mécanisme possible (source: CREALP, SION)







http://www.crealp.ch/f_principal.html

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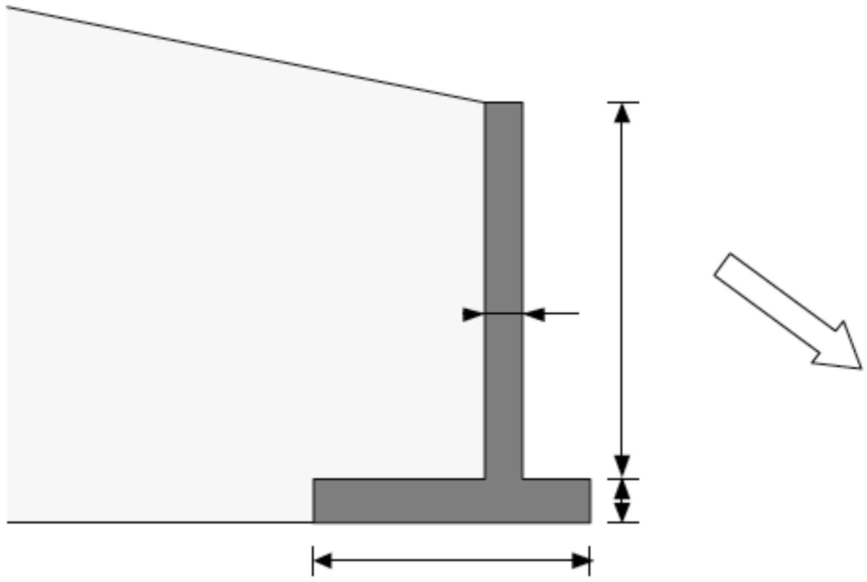
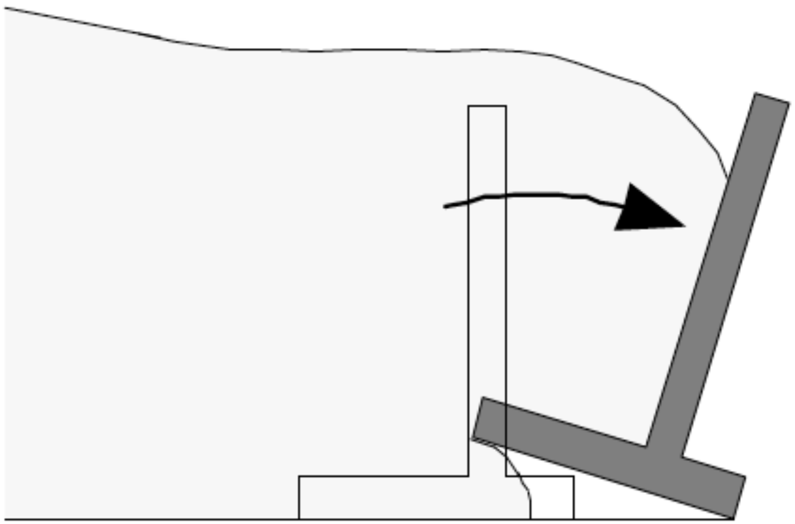


STABILITY



RETAINING STRUCTURES

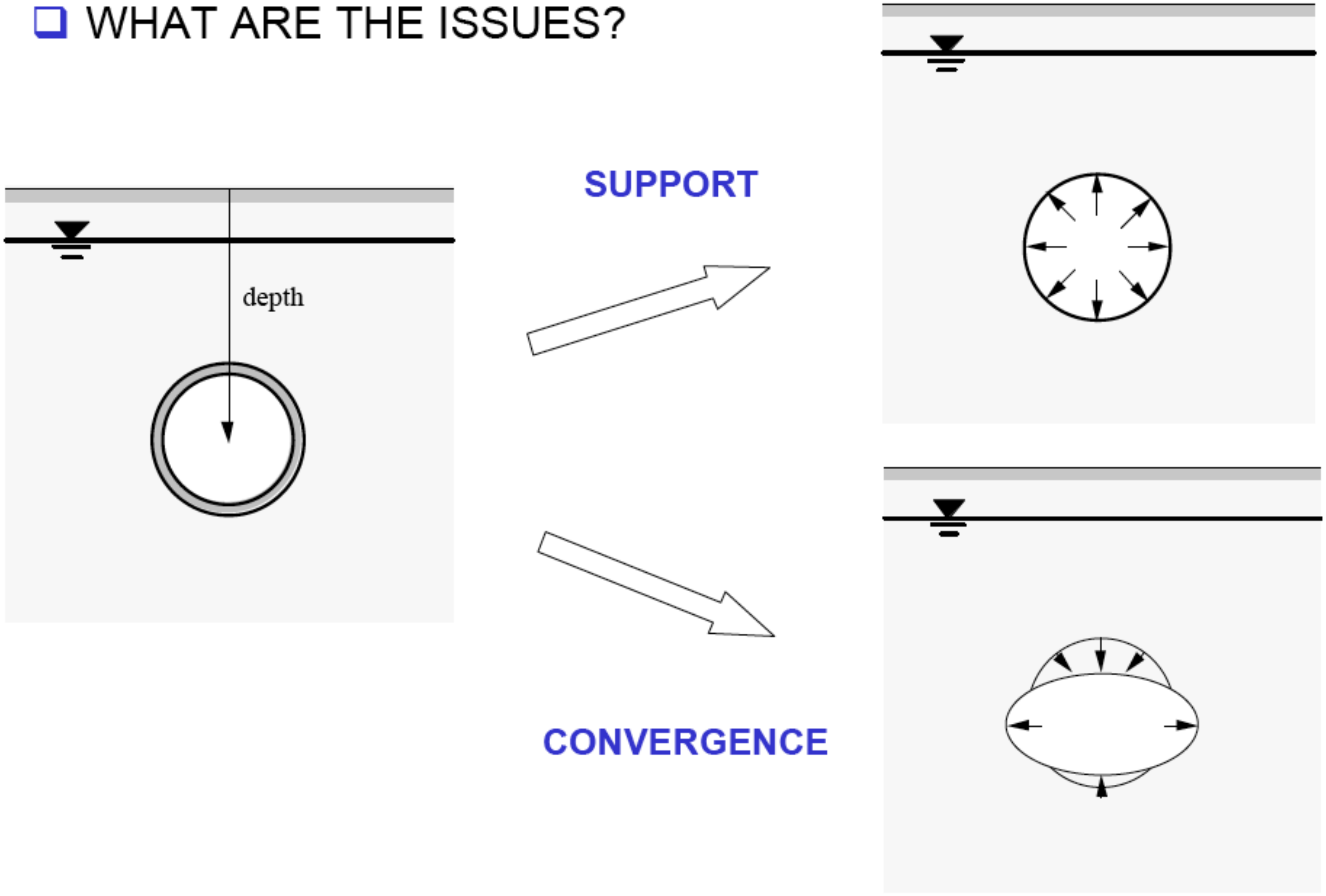
Cantilever Wall

Anchored Wall

RETAINING STRUCTURES

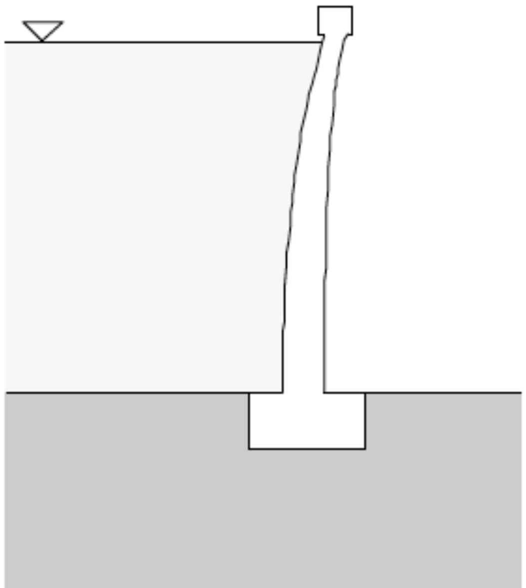
Boston Central Artery Project – ***The Big Dig***

❑ WHAT ARE THE ISSUES?

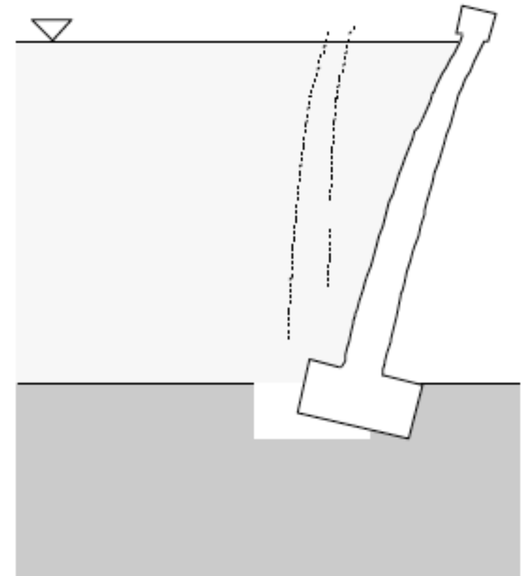


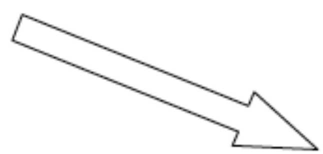
English Channel Tunnel - ***Chunnel***

❑ WHAT ARE THE ISSUES?

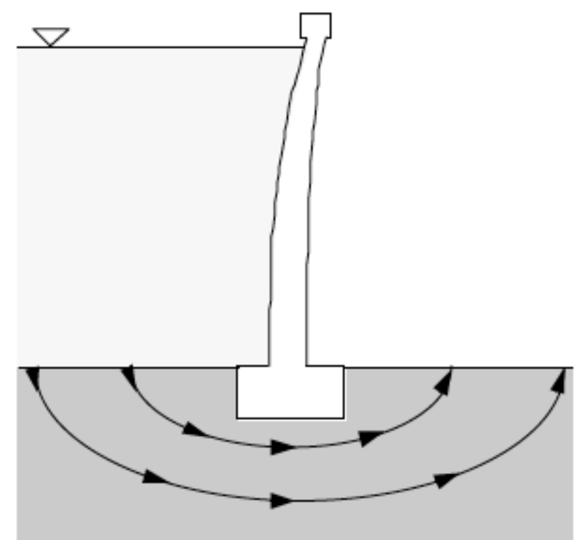


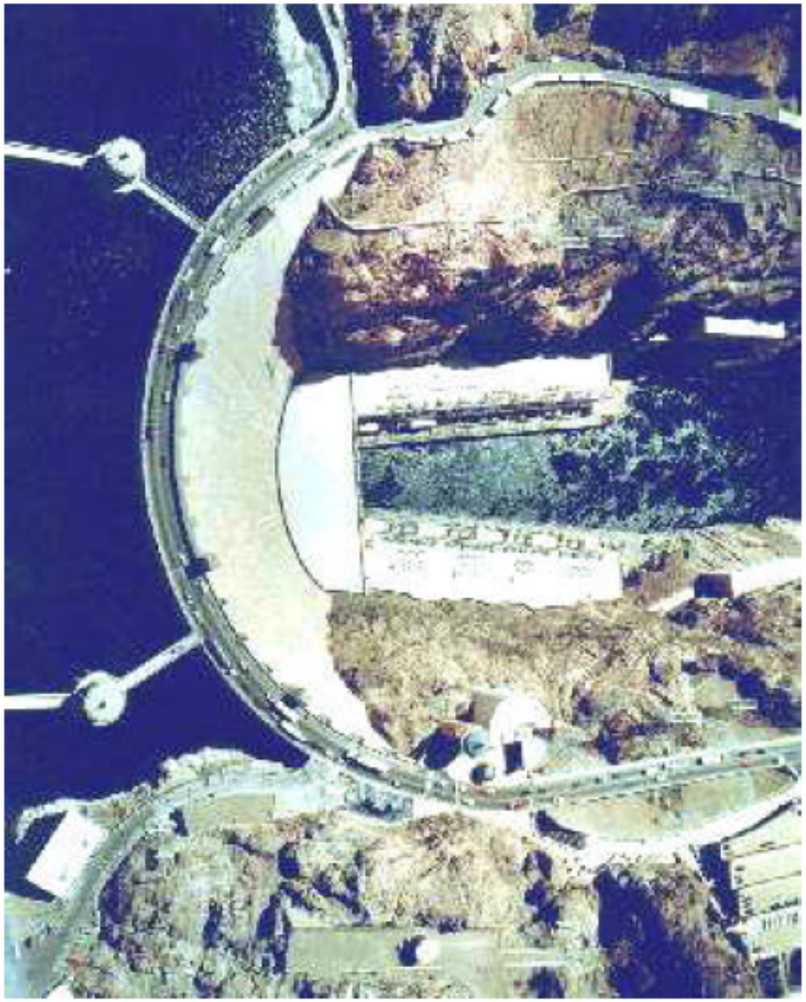
STABILITY





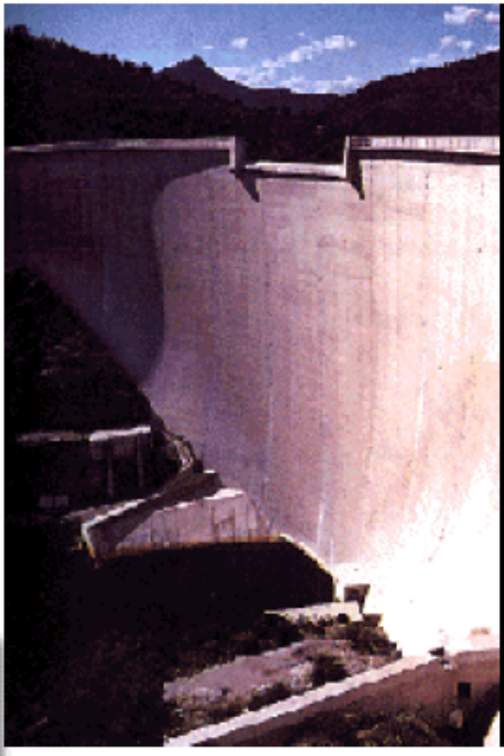
SEEPAGE

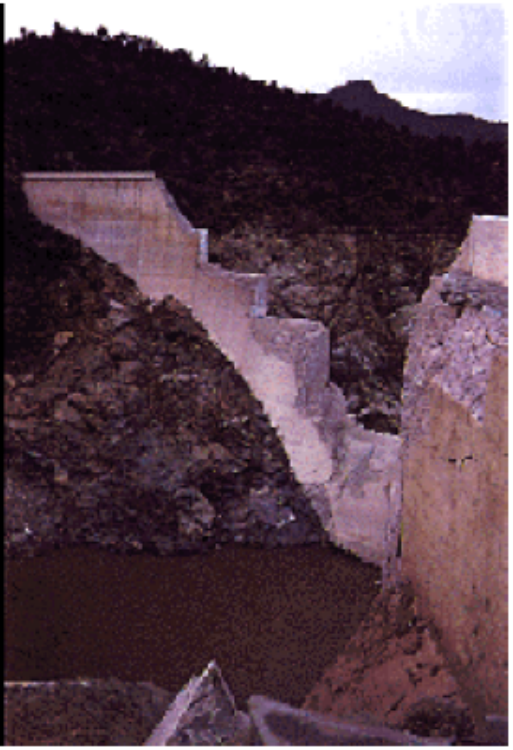




Hoover Dam,
Colorado River, Nevada

East Side Reservoir Project
Riverside County, California





Before

After

Malpasset Dam, France.
Failed December 2, 1959

Before

…during

…and after failure

Teton Dam, Idaho.
Failed June 5, 1976

Niigata, Japan 1964

Adapazari, Turkey 1999

A partially sunken house illustrates the challenge of understanding how grains of soil interact with each other and under what conditions they will support structures. (National Geophysical Data Center)

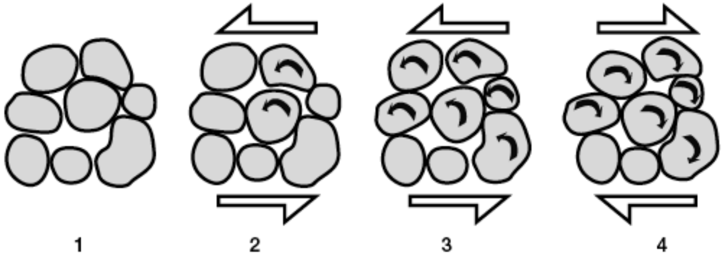


Figure 1. The packing of particles can change radically during cyclic shear; (1) a large hole is maintained by the particle interlocking; (2) a small counterclockwise strain causes the hole to collapse; (3) large shear strain causes more holes to form; (4) holes will collapse when the strain direction is reversed (*Youd, 1977*).

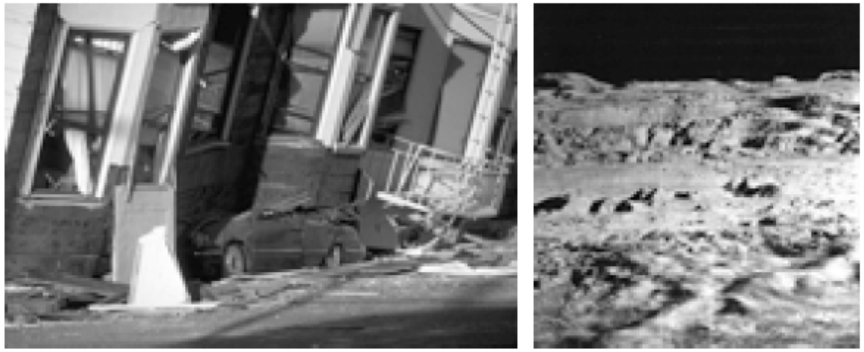


Figure 2. Partially sunken houses in San Francisco and the slumped sides of Copernicus crater on the Moon share one geologic fact: soil liquefaction. (USGS, NASA)

❑ WHAT WE DESIGN FOR?

- **AVOID FAILURE**

- **LIMIT DEFORMATIONS**

… WE LEARN

Some of the problems are not new,
…but now we know (many of) the answers

Questions

1. A possible explanation of the leaning of the Pisa tower is that the subsoil contains a compressible Clay layer of variable tkickness.
On what side of the tower would that clay layer be thickest ?

2. Another possible explanation for the leaning of the Pisa tower is that in earlier ages (before the start of the bluiding of the tower, in 1400), a heavy structure stood near that location.
On what side of the tower would that building have been ?

Equations de bilan (HM linéaire)

2 phases (solide=matrice minérale, fluide=eau)

donc 2 équations de bilan de masse

et 2 équations d'équilibre

Masses	Equilibres
$\dot{n} = (1-n)\mathrm{tr}\dot{\underline{\underline{\varepsilon}}}$	$\mathrm{div}\,\underline{\underline{\sigma}} + \rho\vec{g} = \vec{0}$
⬆ *variation de porosité*	⬆ *contraintes du milieu diphasique*
$n\dot{\rho}_f + \rho_f\,\mathrm{tr}\,\dot{\underline{\underline{\varepsilon}}} + \mathrm{div}(\rho_f\vec{q}) = 0$	$-\overrightarrow{\mathrm{grad}}p + \rho_f\vec{g} = \vec{I}$
⬆ *vitesse moyenne d'écoulement du fluide*	⬆ *force de volume d'interaction fluide/solide*

Lois de comportement (HM linéaire)

2 phases (solide=matrice minérale, fluide=eau)

donc 3 modèles de comportement (au moins)

Fluide (compressibilité) : $$p = \chi_f \log \frac{\rho_f}{\rho_f^0}$$

Solide (élasticité) : $$\underline{\underline{\sigma}} + p\underline{\underline{1}} = 2G\underline{\underline{\varepsilon}} + \left(\chi - \tfrac{2}{3}\mathrm{G}\right)(\mathrm{tr}\underline{\underline{\varepsilon}})\underline{\underline{1}}$$

Interaction (diffusion) : $$\vec{q} = -\frac{k}{\mu_f}\vec{I}$$

Contenu du cours

Milieu poreux diphasique : lois de bilan, PPV à deux champs de vitesse

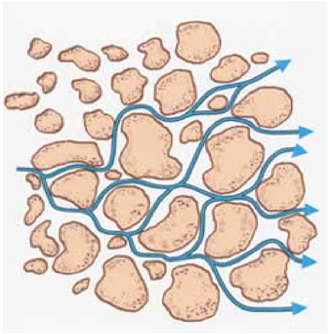
Diffusion linéaire (Darcy), Consolidation linéaire (Biot)

Elastoplasticité (Mohr-Coulomb, Cam-Clay)

Apercu de quelques enrichissements de modélisation sur études de cas :

- HM non saturé (solide/eau/air)
- THM (T=thermique) : stockage profond de déchets nucléaires (exothermiques) , injection d'eau froide dans un puit de forage pétrolier
- THMC (C=chimique) : transport de polluant (huile lourde, pesticide) dans les nappes phréatique

Milieux poreux
Poro-Mécanique
Couplages hydro-mécanique



Micro-scale: grains, pores, examples
Macro-scale: porosity, specific surface

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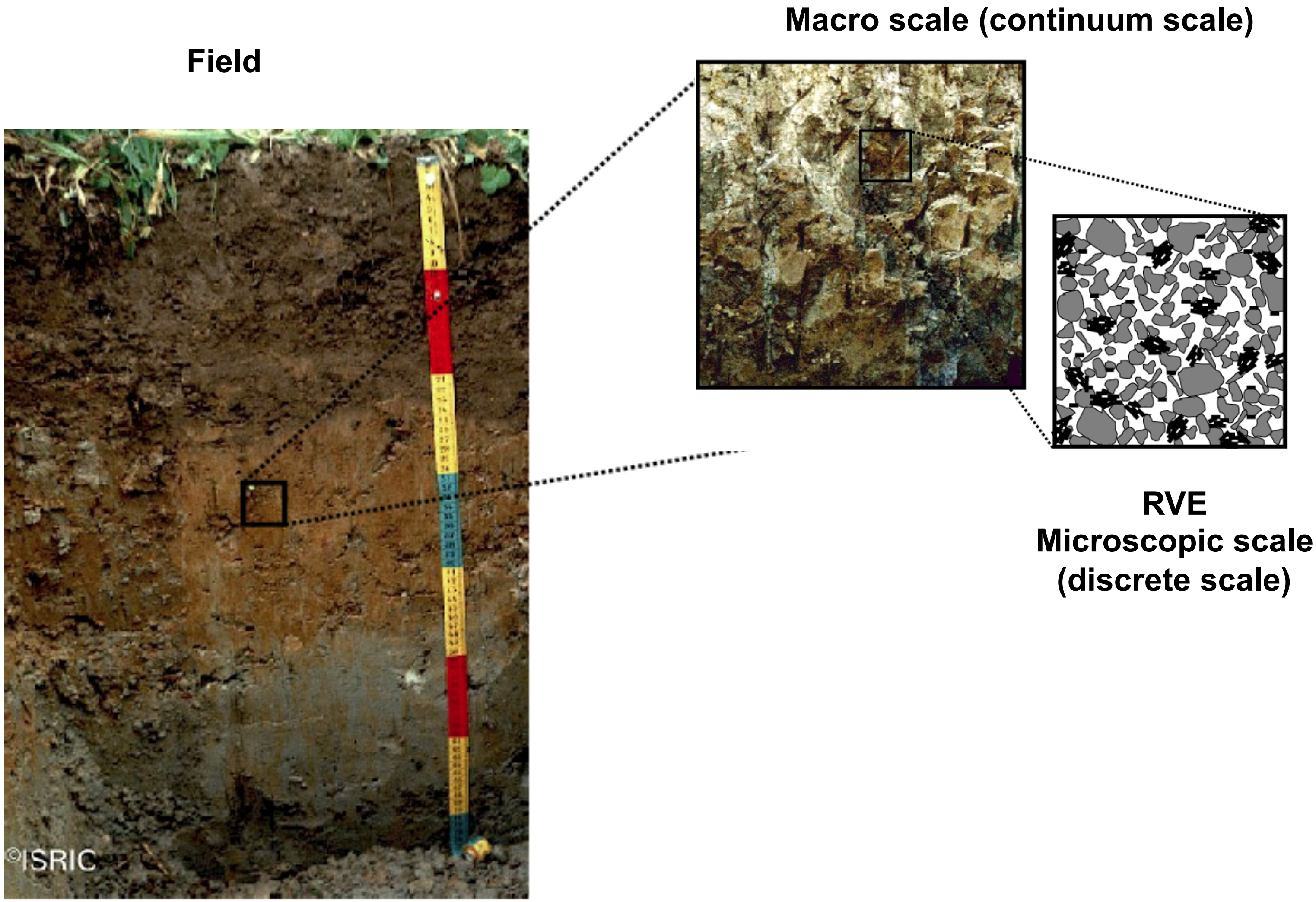
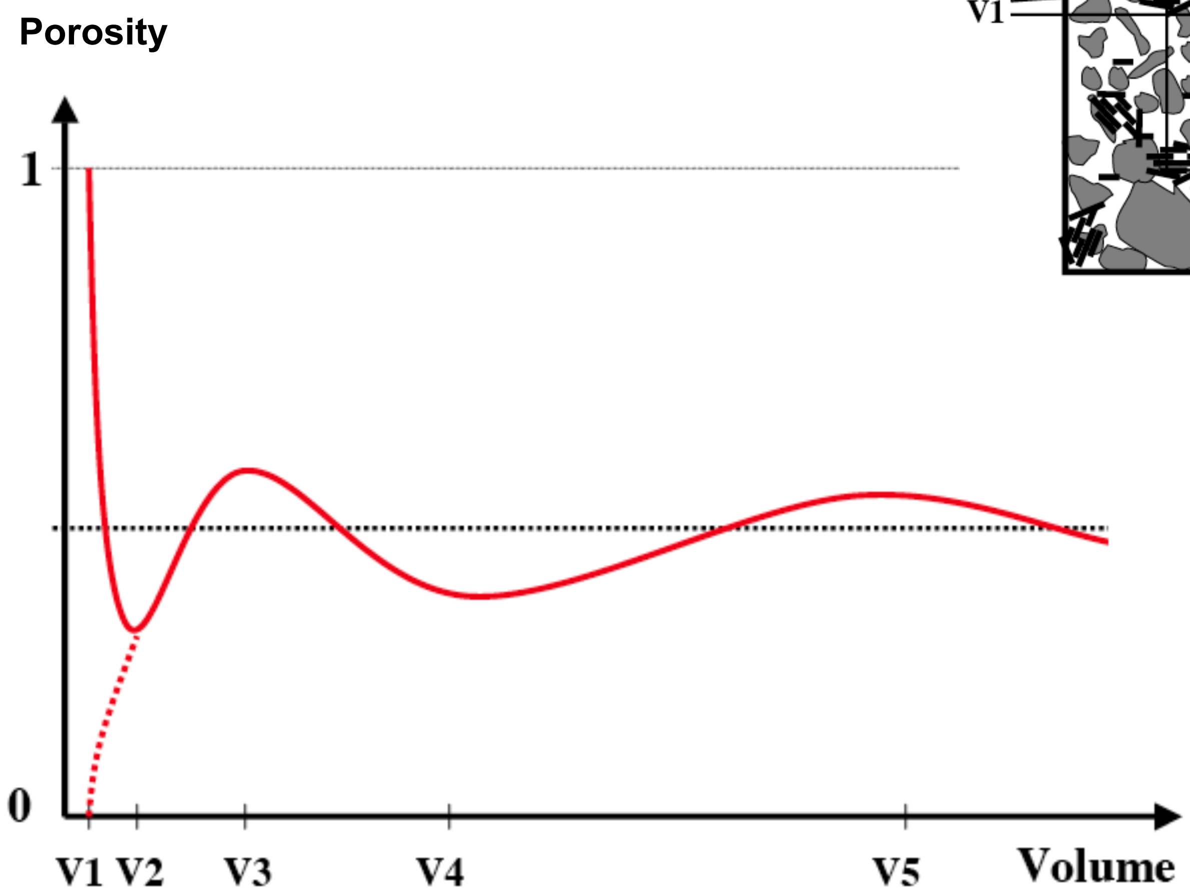
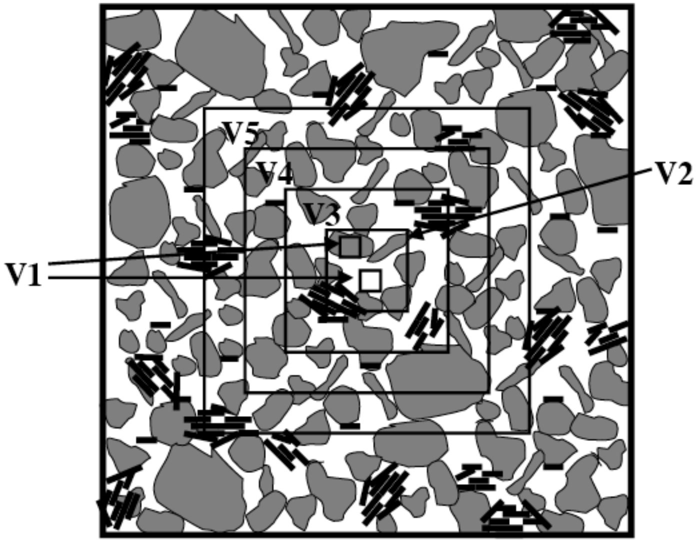
stephane.bonelli@irstea.fr

Different scales



Influence of the RVE size



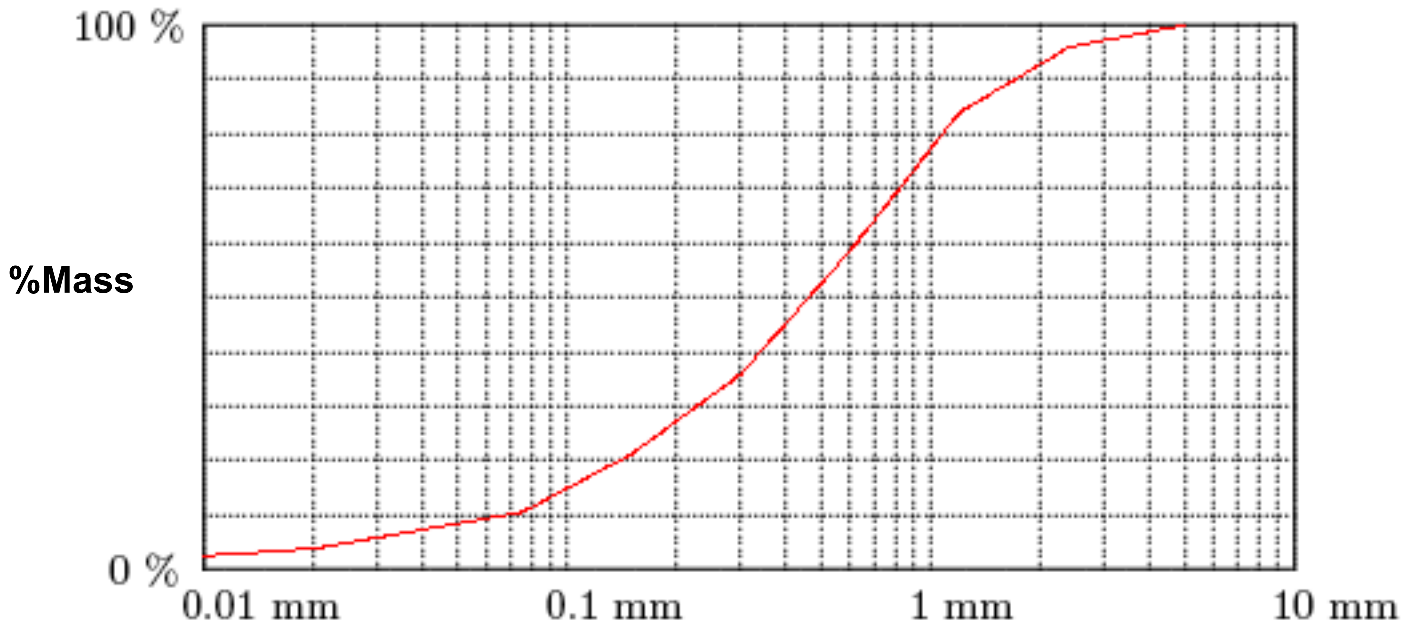


Grain size

Soil type	min.	max.
clay		0.002 mm
silt	0.002 mm	0.063 mm
sand	0.063 mm	2 mm
gravel	2 mm	63 mm

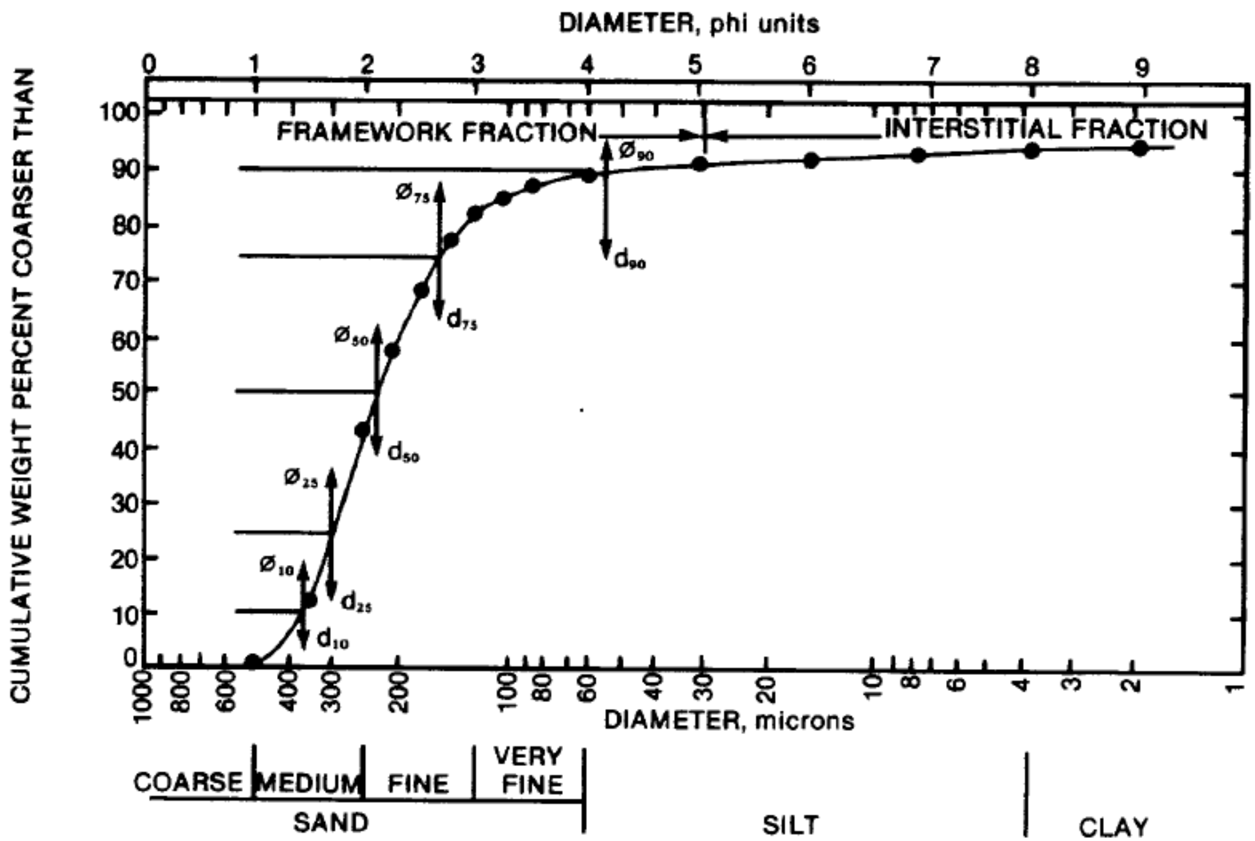
Density

$\rho_{solids} \approx 2700 \pm 50 \text{ kg/m}^3$



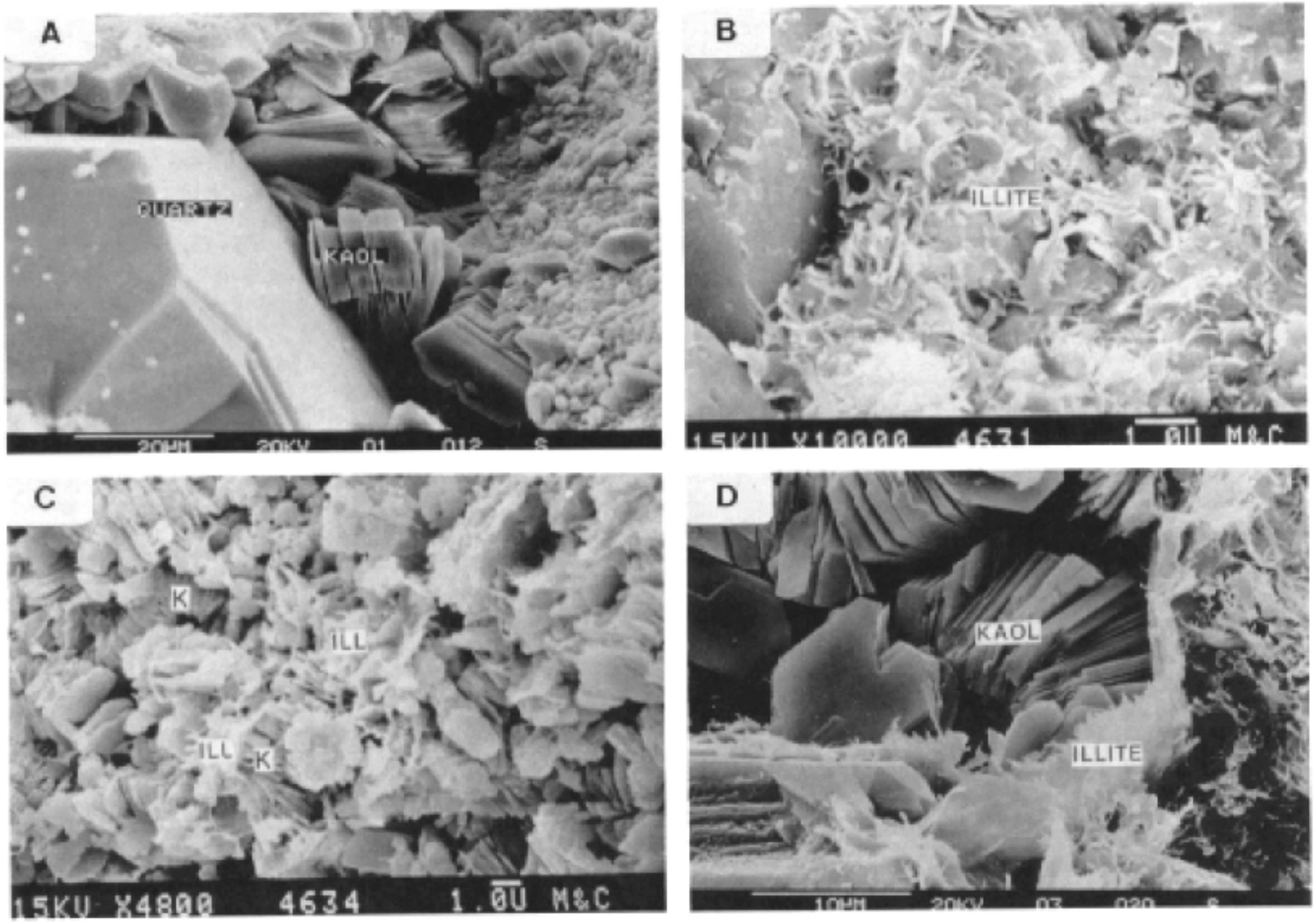
Grain size diagram

Grain size



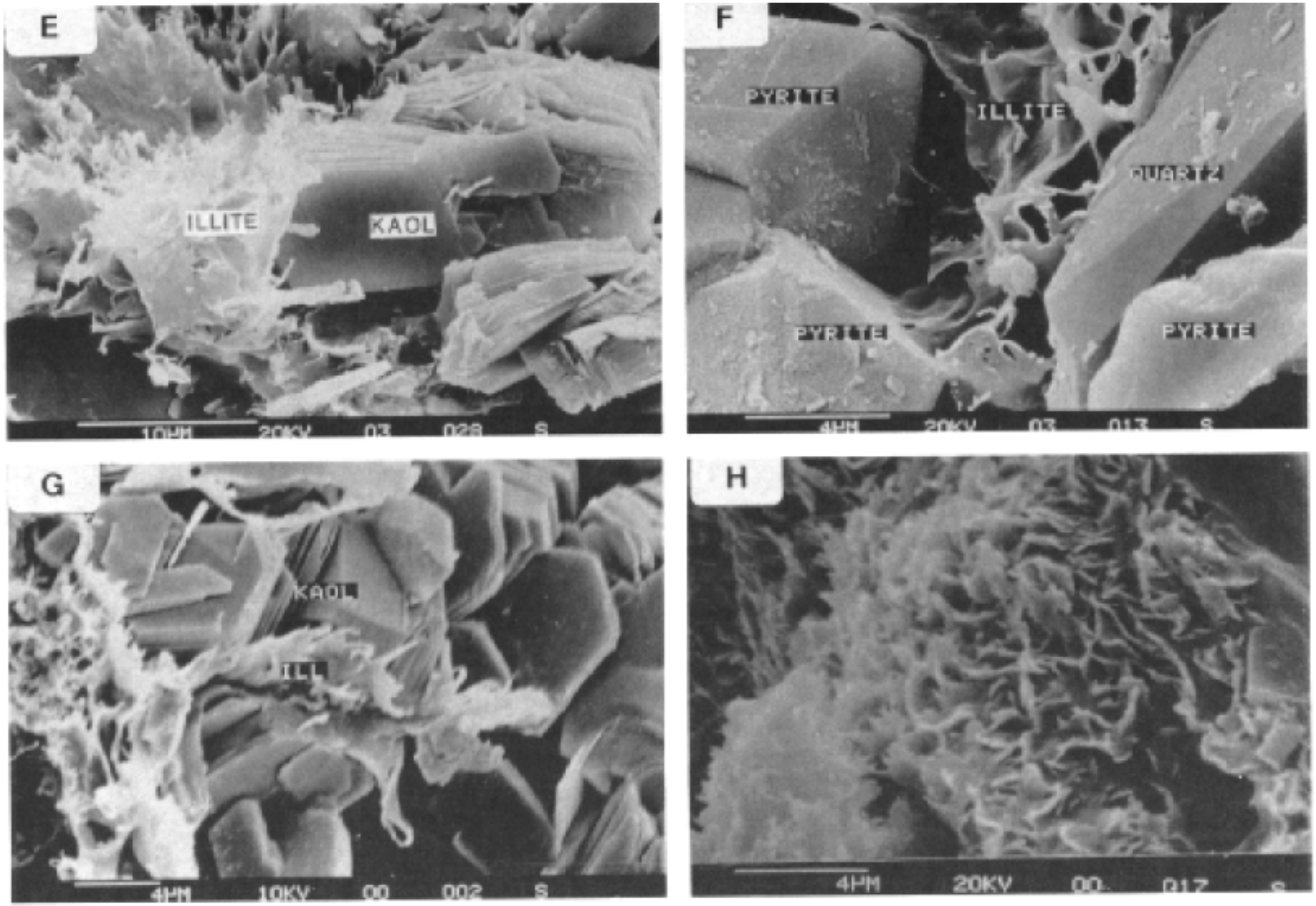
Cumulative grain size distribution; percent by weight greater than a given diameter on a logarithm scale (Jorden and Campbell, 1984)

Grains



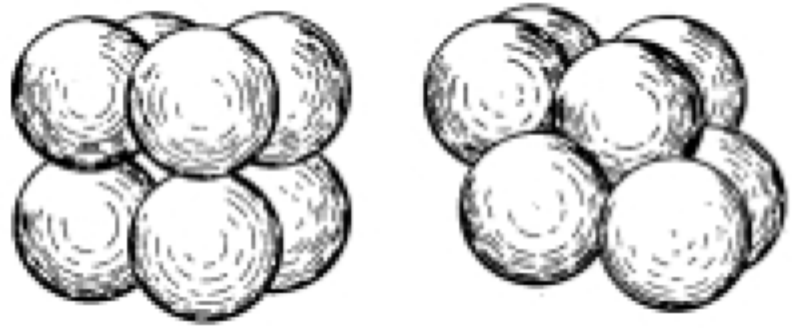
SEM photomicrographs of kaolinite and illite in sandstone (Houseknecht and Pittman, 1992)

Grains

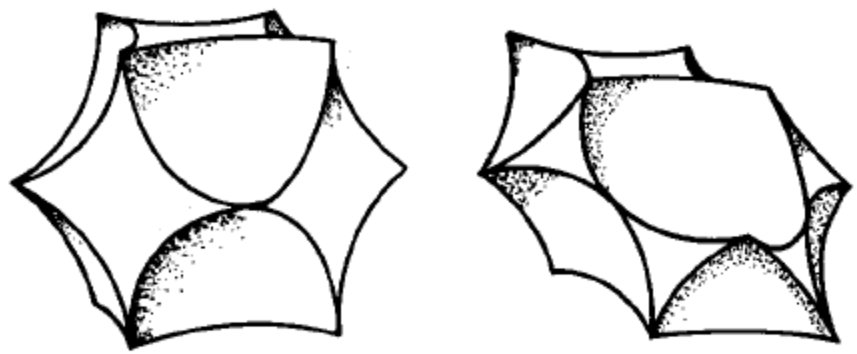


SEM photomicrographs of kaolinite and illite in sandstone (Houseknecht and Pittman, 1992)

Pores

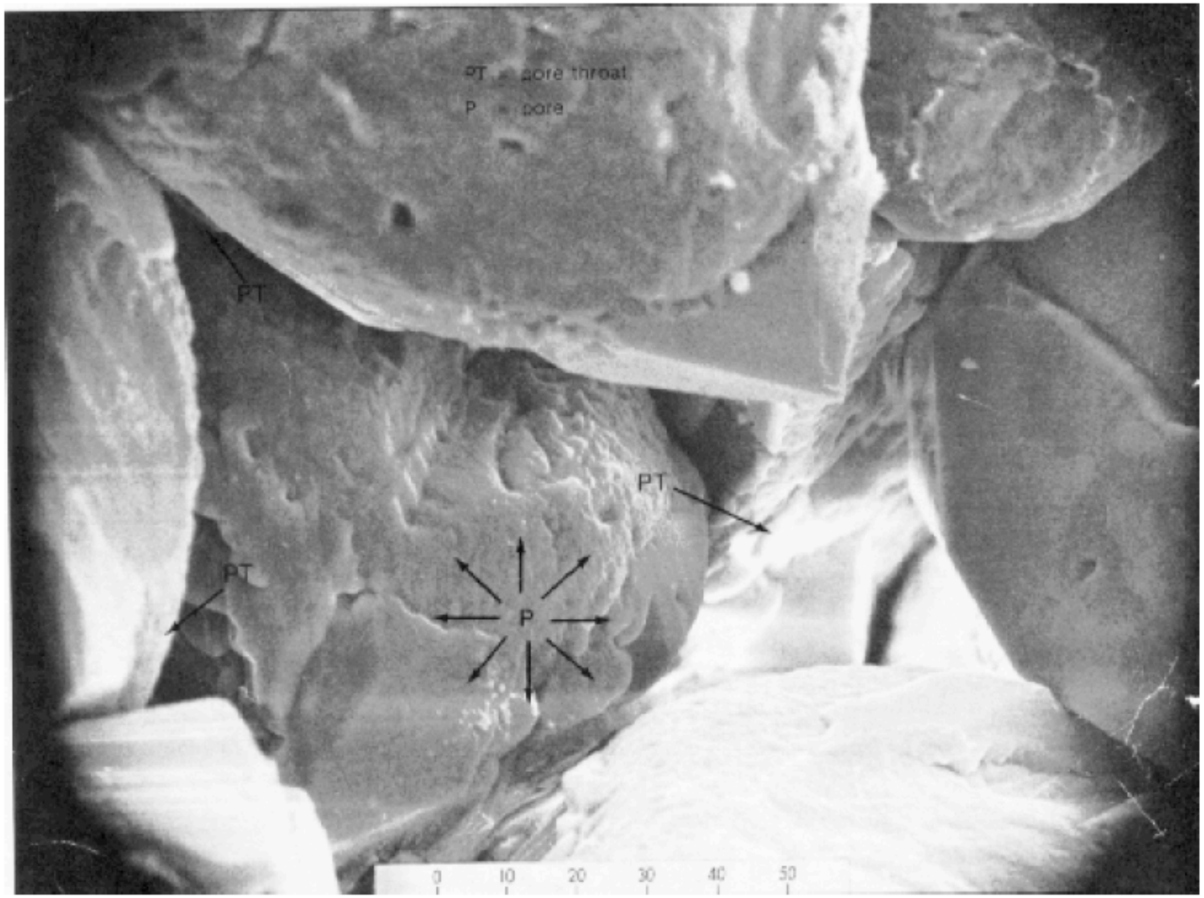


Packs of spherical beads (Collins 1961)



Pore space of spherical bead pack (Collins 1961)

Pores



SEM microphotograph of bore body and pore throat (Jordon 1984)

Pores

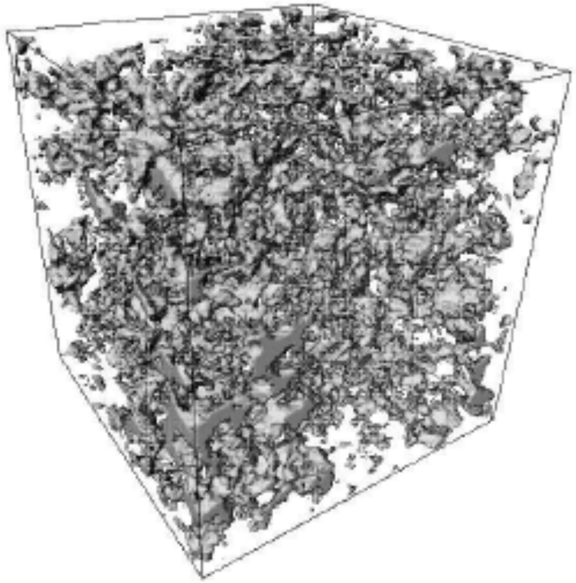
Kaolinite

SEM of micropores in kaolinite. Note 10 μm scale. (Swanson 1985)

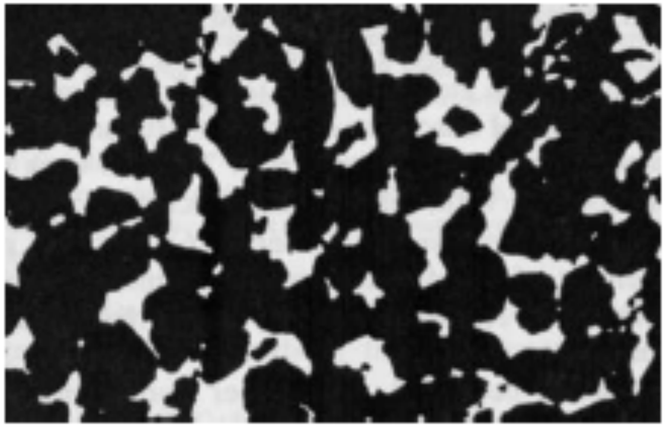
Chert

SEM of micropores in chert. Note 10 μm scale. (Swanson 1985)

Illustration of the micro scale



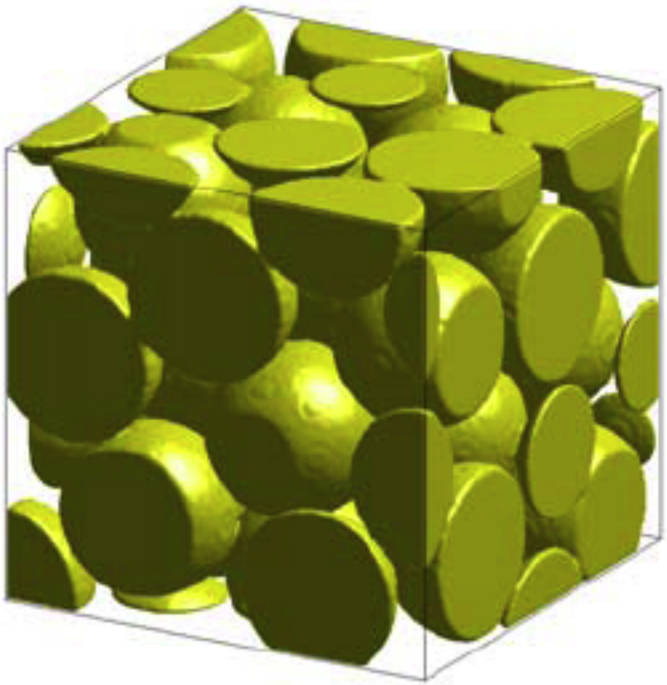
Espace des pores d'un grès de la mer du Nord (données Statoil).



Exemple de coupe d'un poreux (grès).

Illustration of the micro scale

Finney-pack
(Random-dense pack of spheres)



Fontainebleau Sandstone
(By X-ray microtomography)

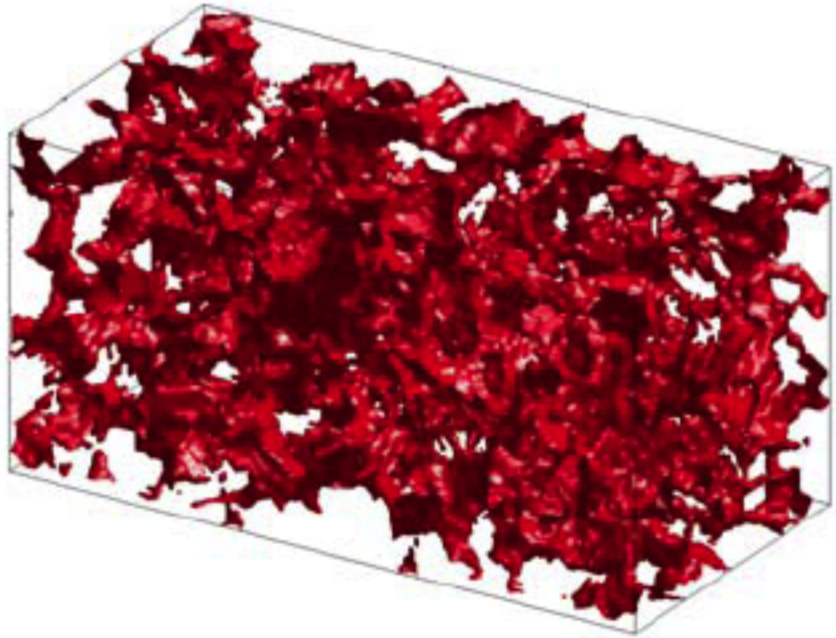


Illustration of the micro scale

Thin section of about 1 mm length from a loamy-clay soil. Clearly distinguishable are the system of macropores with diameters of some 0.1 mm, small soil aggregates (lighter shades of brown) with sizes of about 0.3 mm, and the system of meso- and micropores. (Image courtesy of H.-J. Vogel)

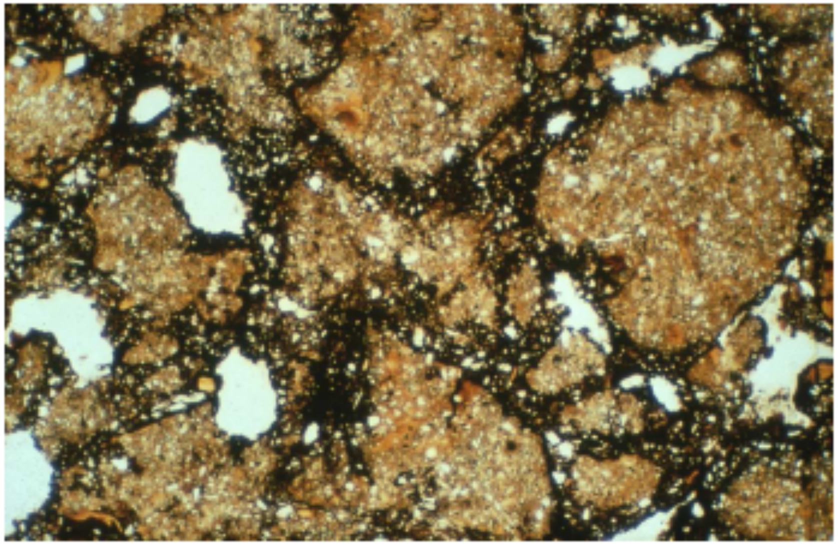
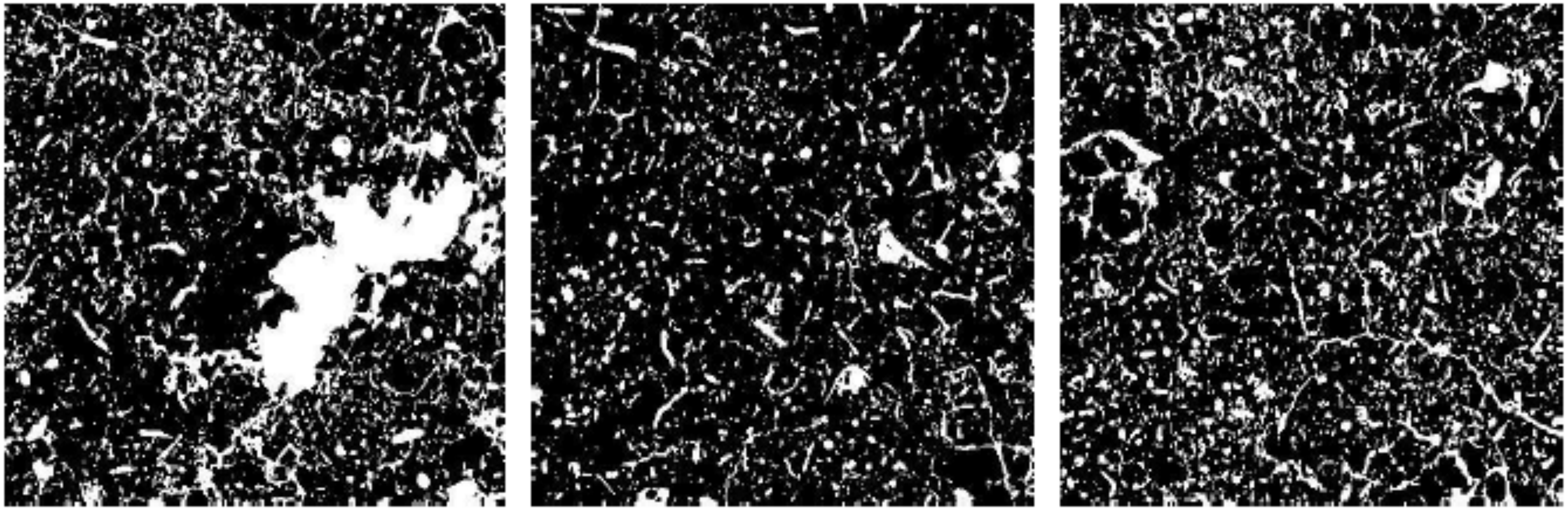
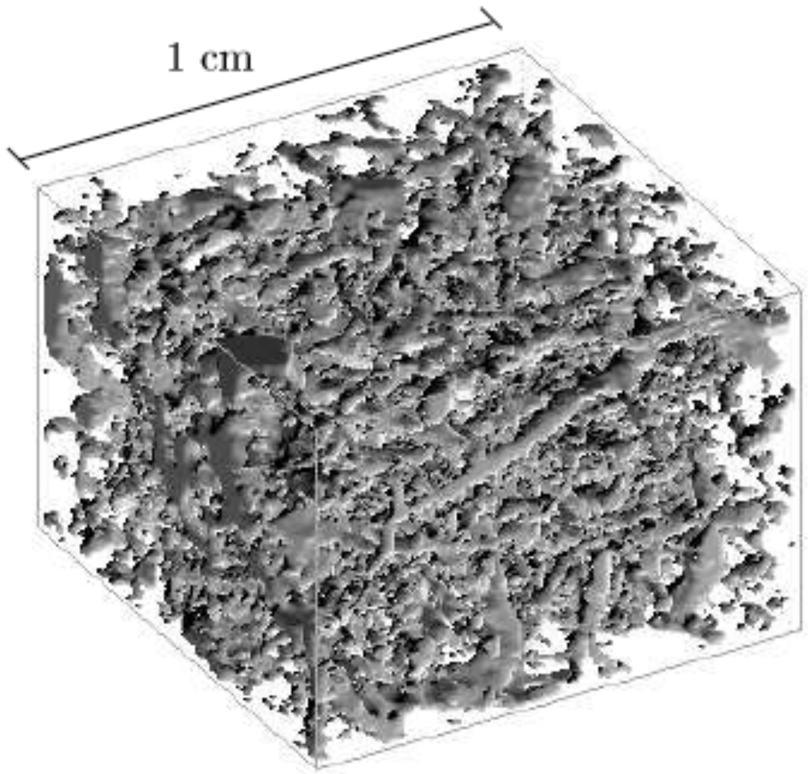


Illustration of the micro scale



Horizontal cross-sections through a sample taken at 0.4 m depth from a loamy-clay soil near Beauce, France [*Cousin et al.*, 1996]. The side length of the square sections is 48 mm with a resolution of 0.12 mm. The smallest visible pores thus are comparable to the largest pores in Figure 3.1. The vertical distance between the sections shown here is 6 mm. White represents the pore space, black the soil matrix which itself is again porous at a smaller scale. (Data courtesy of I. Cousin)

Illustration of the micro scale



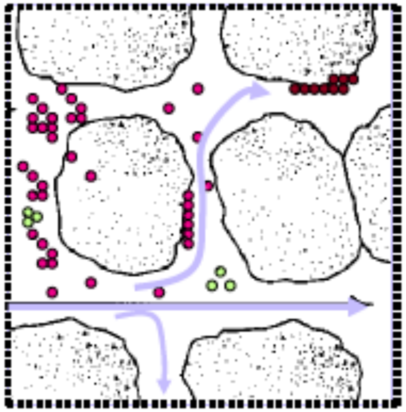
Three-dimensional reconstruction of the macropore system for a selection from the dataset shown in Figure 3.2. Resolution is 0.12 mm horizontally and 0.10 mm vertically. (Image courtesy of H.-J. Vogel)

Problème multi-échelle

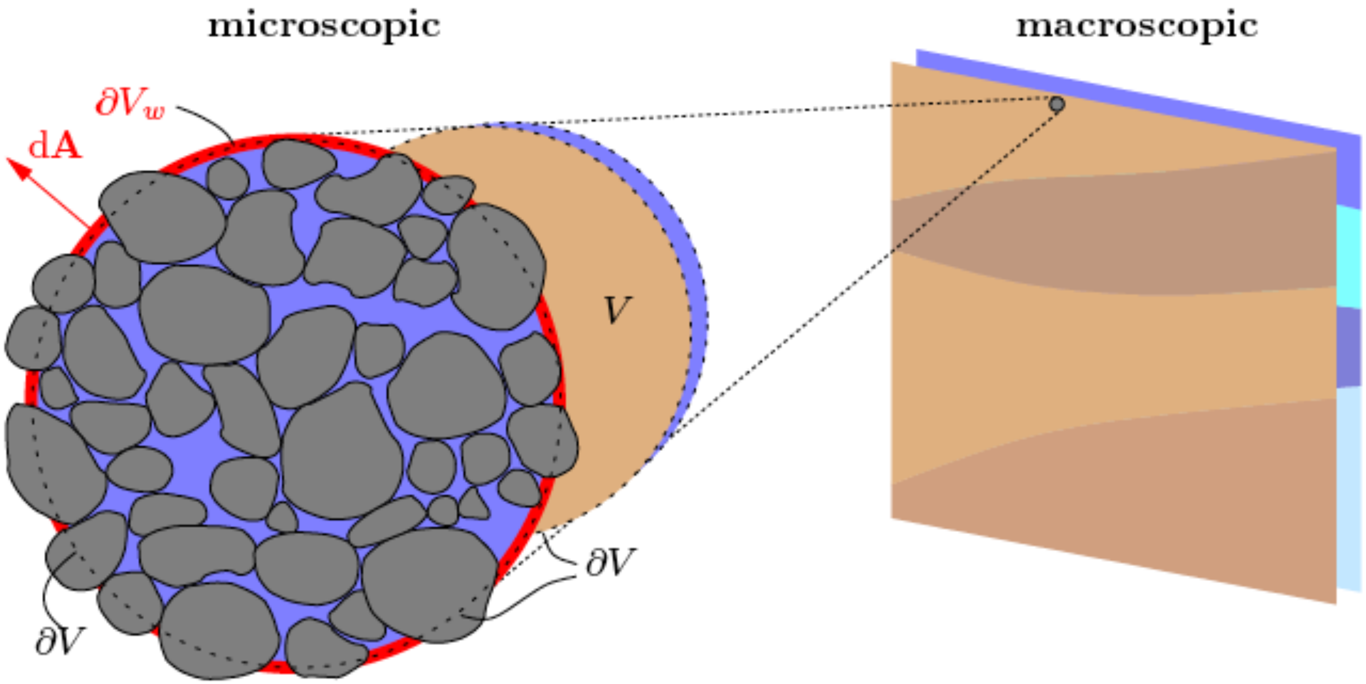
- Approche microscopique
  - de mettre en évidence les phénomènes à prendre en compte par la loi macroscopique
  - de donner des moyens pour obtenir les paramètres des lois macroscopiques (e.g. perméabilité)
- Approche macroscopique
  - on moyenne le phénomène sur un volume plus grand que les pores (VER)
  - l'acte de moyennation implique une perte d'information, donc des informations supplémentaires éventuellement empiriques doivent être considérées (ex: loi de Darcy)

Problème multi-physique

- Transport de masse
  - Écoulement
  - Diffusion
  - Dispersion
- Transfert de masse
  - Sorption (Adsorption, chimisorption, échange de ions)
  - Atténuation (biodégradation, décomposition radioactive)
  - Transformations (dissolution/précipitation)



Transition to the continuum scale



Transition from pore-scale (microscopic) to continuum (macroscopic) representation. Consider a macroscopic volume V with boundary ∂V (dotted line). Microscopically, the detailed distribution of all the phases is available, e.g., of the water phase $V_w \subset V$ with external boundary $\partial V_w \subset \partial V$ (red line). Macroscopically, the phases and possibly other quantities are replaced by the superposition of continuous fields (uniformly colored regions). These fields may vary in space, but on a much larger scale than that of the averaging volume.

Transition to the continuum scale

Consider a representative elementary volume (REV) constitued of two-phases, Solids and pores (always filled with some fluid in the real world)

Phase indicator function

$$\chi_i(\mathbf{x}) = \begin{cases} 1 \text{ if } \mathbf{x} \in \text{ phase } i \\ 0 \text{ otherwise} \end{cases}$$

Pore volume

$$\chi_{pore}(\mathbf{x}) + \chi_{solids}(\mathbf{x}) = 1$$

$$V_{pore} = \int_V \chi_{pore}(\mathbf{x}) dV$$

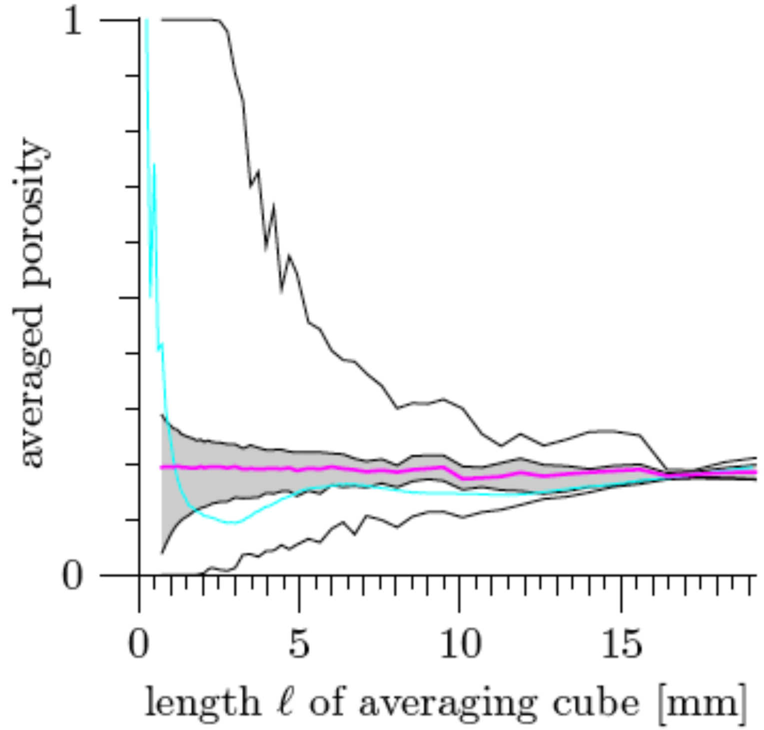
$$V_{pore} + V_{solids} = V$$

Solid volume

$$V_{solids} = \int_V \chi_{solids}(\mathbf{x}) dV$$

Transition to the continuum scale

Estimated porosity of soil sample from Figure 3.2 as a function of averaging cube's length. The cyan curve represents a particular location. The other curves represent the ensemble of all cubes: average (magenta), minimum and maximum, and the two quartiles. Half of all values are within the gray band. The linear extent of a reasonable REV would be some 17 mm.

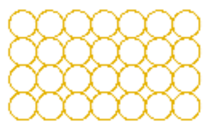


Porosity

Porosity: $n \underset{def}{=} \frac{V_{pore}}{V_{total}}$ **Compacity:** $1-n \underset{def}{=} \frac{V_{solids}}{V_{total}}$

N: **nb spheres**
***D*$_{Grain}$: diameter**

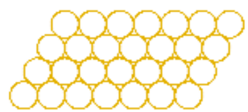
$$V_{solids} = N\frac{\pi D_{Grain}^3}{6}$$



Cubic array.

Loose arrangement

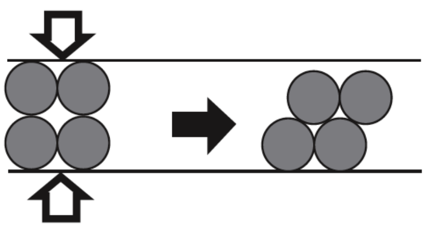
$$V_{total} = ND_{Grain}^3 \Rightarrow n = 1-\frac{\pi}{6} \approx 0.476$$



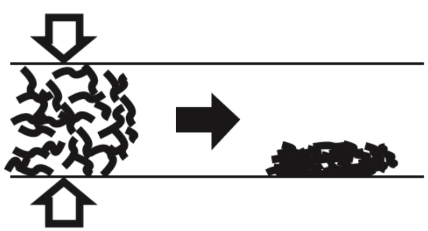
Rhombic array.

Dense arrangement

$$V_{total} = N\frac{D_{Grain}^3}{\sqrt{2}} \Rightarrow n = 1-\frac{\pi}{\sqrt{18}} \approx 0.259$$



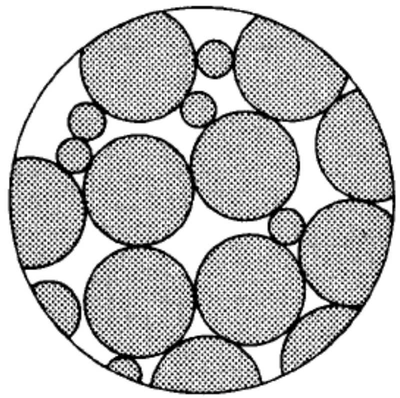
Practical situations for granular materials: $0.25 \le n \le 0.45$



Practical situations for fine materials: $0.05 \le n \le 0.70$

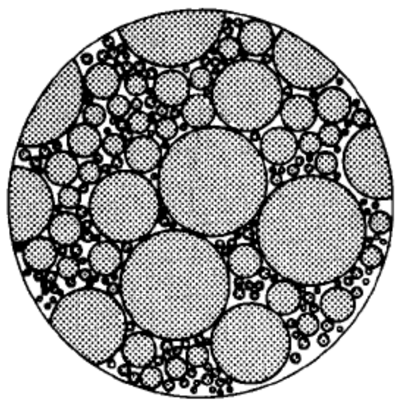
Porosity, schematic

Effect of sorting on porosity (Bear, 1972)



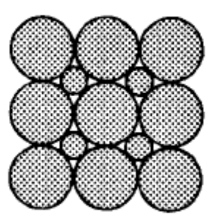
(a) WELL SORTED MATERIAL n = ~32%

$$n = 32\%$$



(b) POORLY SORTED MATERIAL n = ~17%

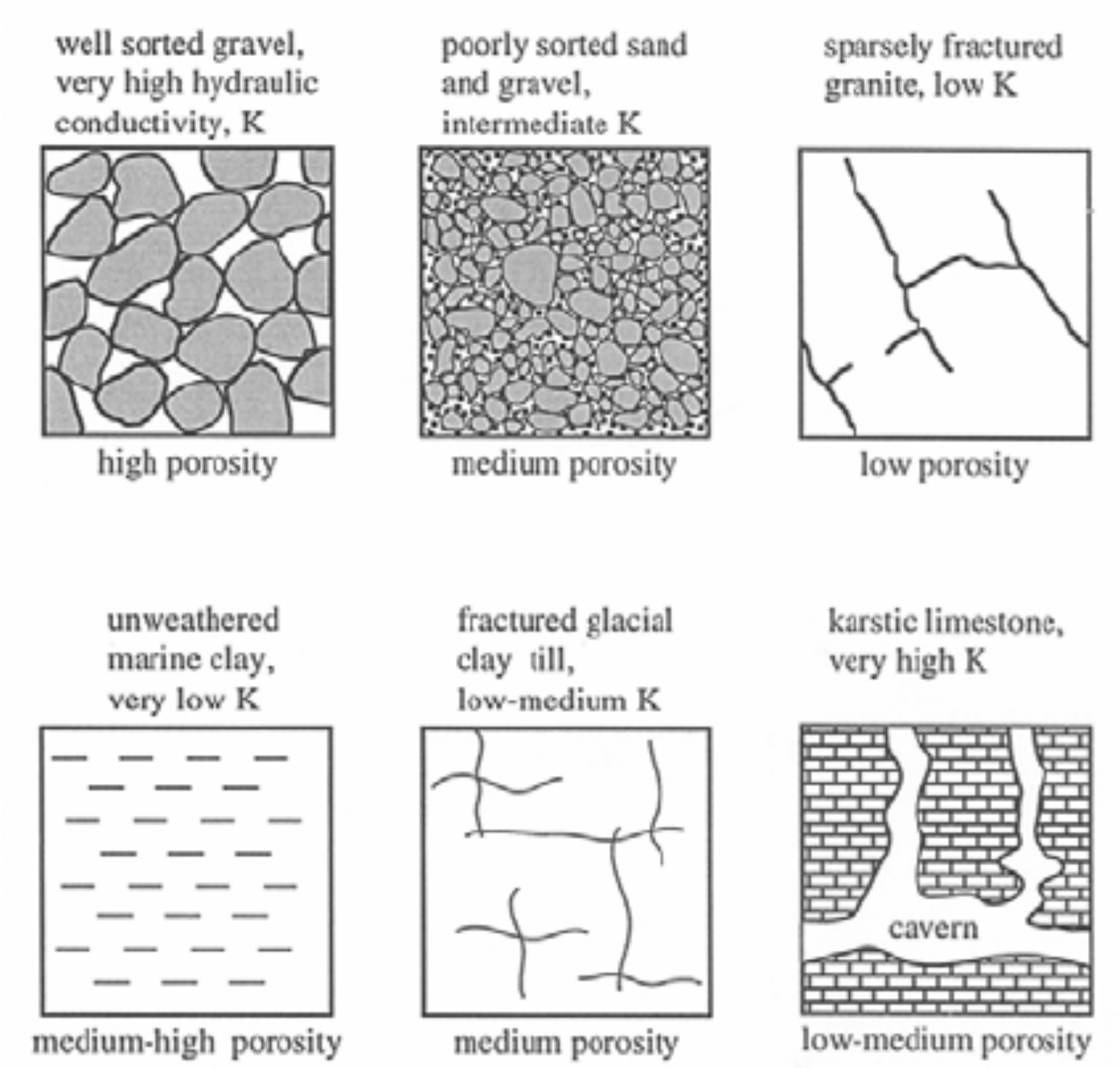
$$n = 17\%$$



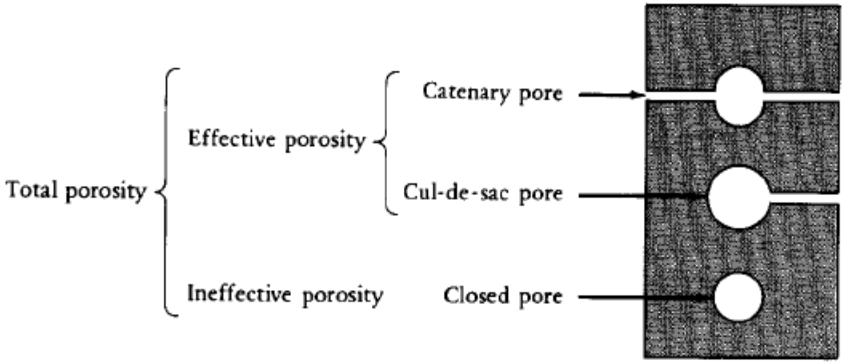
(c) CUBIC ARRANGEMENT OF SPHERICAL GRAINS OF TWO SIZES n = ~12.5%

$$n = 12.5\%$$

Porosity, schematic

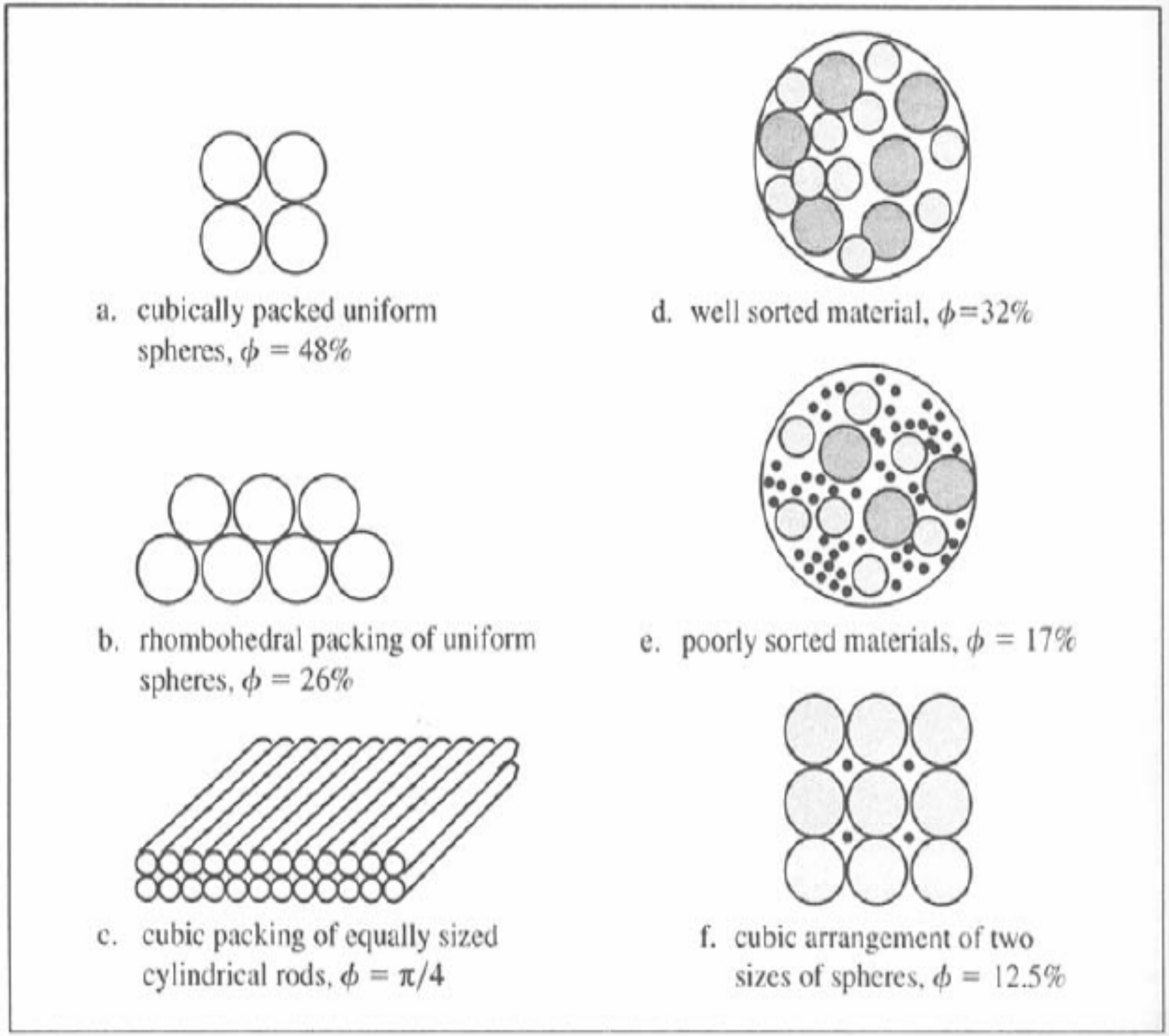


Porosity, schematic

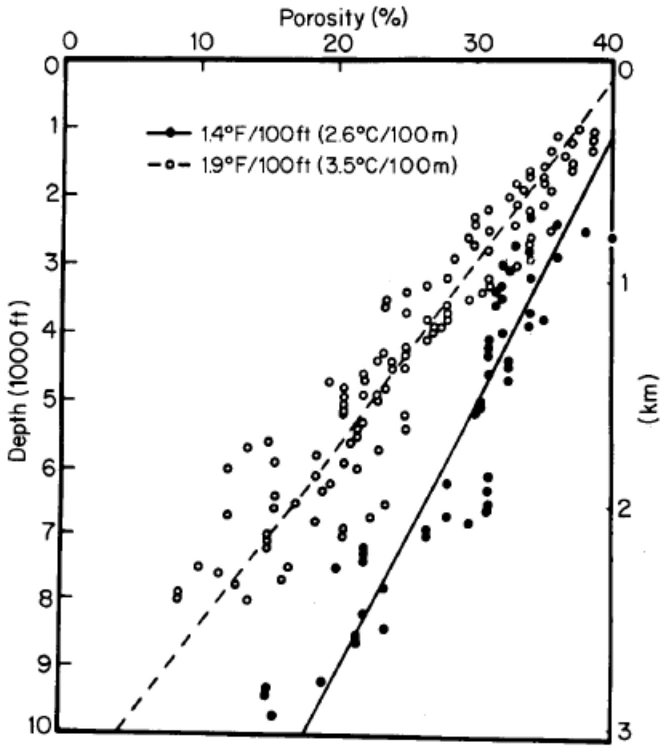


The three basic types of porosity. (Selley 1985)

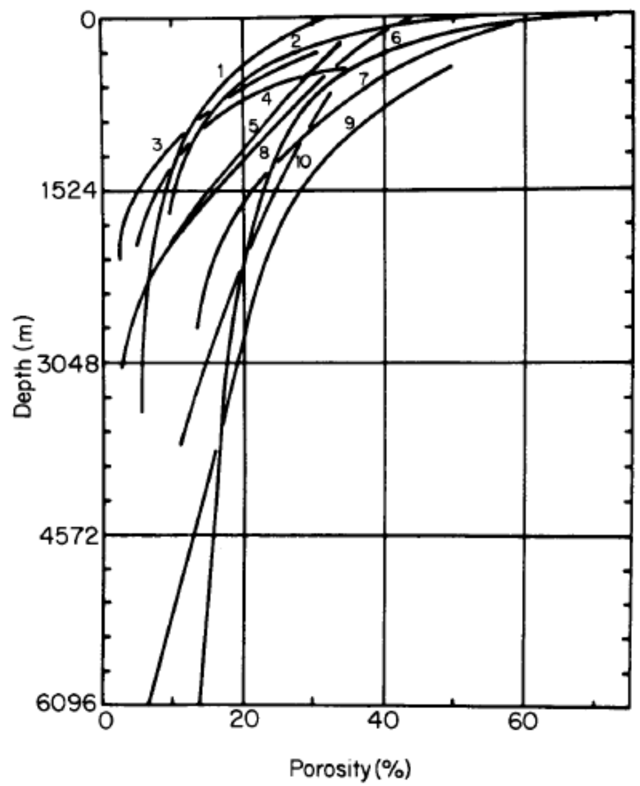
Porosity, examples



Porosity, examples



Porosity of sandstones with depth for two geothermal gradients [Jenyon 1990 (Magara 1980)]



Porosity of clay/shale as a function of depth [Jenyon 1990 (Magara 1980)]

Specific surface

Pore surface (m²)

$$A_{pore} = \partial V_{pore} = \int_V \left\| \nabla \chi_{pore}(\mathbf{x}) \right\| dV$$

Solid surface

$$A_{solids} = \partial V_{solids} = \int_V \left\| \nabla \chi_{solids}(\mathbf{x}) \right\| dV$$

Pore specific surface (m⁻¹)
(pore surface per unit REV volume)

$$a_{V_{pore}} \underset{def}{=} \frac{A_{pore}}{V_{total}}$$

Solids specific surface (m⁻¹)
(solids surface per unit REV volume)

$$a_{V_{solids}} \underset{def}{=} \frac{A_{solids}}{V_{total}}$$

Solids mass specific surface (m²/kg)
(solids surface per unit REV mass)

$$a_{W_{solids}} \underset{def}{=} \frac{A_{solids}}{M_{solids}}$$

$$\partial V_{solids} - \partial V_{pore} = \text{contact area}$$

Negligible in granular soils

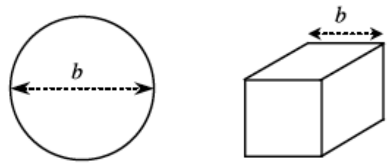
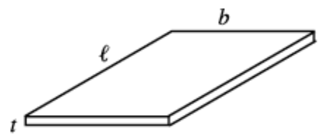
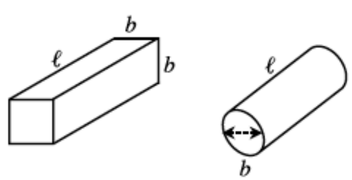
$$a_{V_{pore}} \approx a_{V_{solids}}$$

but not in fine soils

$$a_{V_{solids}} = (1-n)\rho_{solids} a_{W_{solids}}$$

Specific surface

Table 1. Specific surface and particle geometry: the smallest dimension controls the specific surface.

Geometry	Equations $S_s = \dfrac{A_s}{M}$	Examples
Sphere or cube: 	$S_s = \dfrac{6}{b\rho_w G_s}$	Amorphous clay minerals Allophane, hollow spherules $b = 50$ Å, $G_s = 2.65$ $S_s = 453\ m^2/g$ (Extreme case: $b = 9.6$ Å, then $S_s = 2358\ m^2/g$)
Thin plate:  $\ell >> t$ and $b >> t$	$S_s = \dfrac{2}{t\rho_w G_s}$	Sheet structure clay minerals Montmorillonite (extreme case: fully swollen) $t = 9.6$ Å, $G_s = 2.65$ $S_s = 786\ m^2/g$
Prism and rod:  $\ell >> b$	$S_s = \dfrac{4}{b\rho_w G_s}$	Chain structure clay minerals Palygorskite, thread $b = 100$ Å, $G_s = 2.65$ $S_s = 151\ m^2/g$

Note: G_s, specific gravity of the particle mineral; (S_s platy particle dimensions $b \times b \times t$)/(S_s cube $b \times b \times b$) = $(\beta + 2)/3$, where $\beta = b/t$; ρ_w, mass density of water (1 g/cm^3).

Specific surface

Examples	Mass specific surface $a_{W_{solids}}$ (m^2 / g)	Volume specific surface $a_{V_{solids}}$ (m^{-1})
Gravel $D_{grain} = 1\ \mathrm{cm}$	10^{-4}	400
Fine silt $D_{grain} = 200\ \mu\mathrm{m}$	10^{-2}	10^4 (1 ha/m^3)
Montmorillonite $e = 10\ \mathring{A} = 10^{-6}\ \mathrm{mm}$	750	8×10^8 (800 km^2/m^3 or 800 m^2/cm^3) *the surface of a football field in a thimble*

Granular material

$$\begin{cases} V_{solids} = N \dfrac{\pi D_{Grain}^3}{6} \\ A_{solids} = N \pi D_{Grain}^2 \end{cases}$$

N: **nb spheres**
D

$$\Rightarrow$$

$$V_{solids} = (1-n) V_{total}$$

$$\begin{cases} a_{V_{solids}} = \dfrac{6(1-n)}{D_{Grain}} \\ a_{V_{solids}} = \dfrac{6}{\rho_{solids} D_{Grain}} \end{cases}$$

Hydraulic radius

$$\text{Pore hydraulic radius} \underset{def}{=} \frac{\text{Cross section available for flow}}{\text{Wetted perimeter}}$$

$$= \frac{\text{Volume available for flow}}{\text{Wetted surface}}$$

$$= \frac{\text{(Volume of pores)/(Total volume)}}{\text{(Surface of pores)/(Total Volume)}}$$

$$= \frac{n}{a_{V_{Pore}}}$$

$$\text{Mean pore radius} = \frac{2n}{a_{V_{Pore}}} \approx \frac{2n}{(1-n)\rho_{solids} a_{W_{solids}}}$$

Tube

$$\text{Tube hydraulic radius} = \frac{R_{tube}}{2}$$

$$\text{Mean pore radius} = \frac{nD_{Grain}}{3(1-n)}$$

(granular material)

Granular material

$$\text{Pore hydraulic radius} = \frac{nD_{Grain}}{6(1-n)}$$

Questions

1. What is the density of a dry soil (i.e filled with air) ?

2. What is the density of a water-saturated soil ?

3. Assume n=0.3 for a sand. What is the weight of 1 m^3 of this sand in dry conditions ?

4. Fill the pores of this sand with water. What is the volume of the water than the sand could contain ? Then, what is the density of the saturated sand ?

5. A building is constructed on a clay layer of 5 m thickness, with initial porosity of 50%, on top of a stiff sand. After the construction, the clay porosity is reduced to 40%. What is the settlement of the soil ?

6. The void ratio is another engineering quantity widely used in porous mechanics. Void ratio is defined as follows:

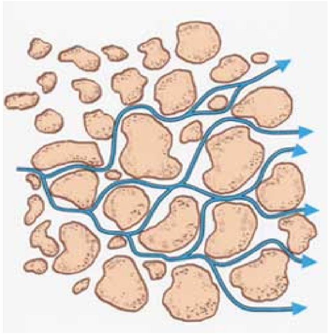
$$e \underset{def}{=} \frac{V_{pore}}{V_{solids}}$$

   Express the void ratio as a function of the porosity.

7. Express the volume strain as a function of the porosity.

8. Express the volume strain rate as a function of the porosity.

Milieux poreux
Poro-Mécanique
Couplages hydro-mécanique



Balance equations:
mass, momentum, energy and entropy
State laws and dissipations

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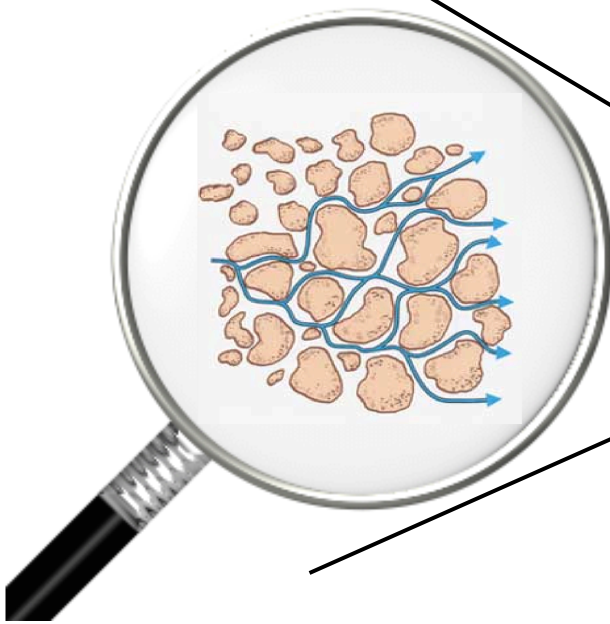
Basic microscopic quantities

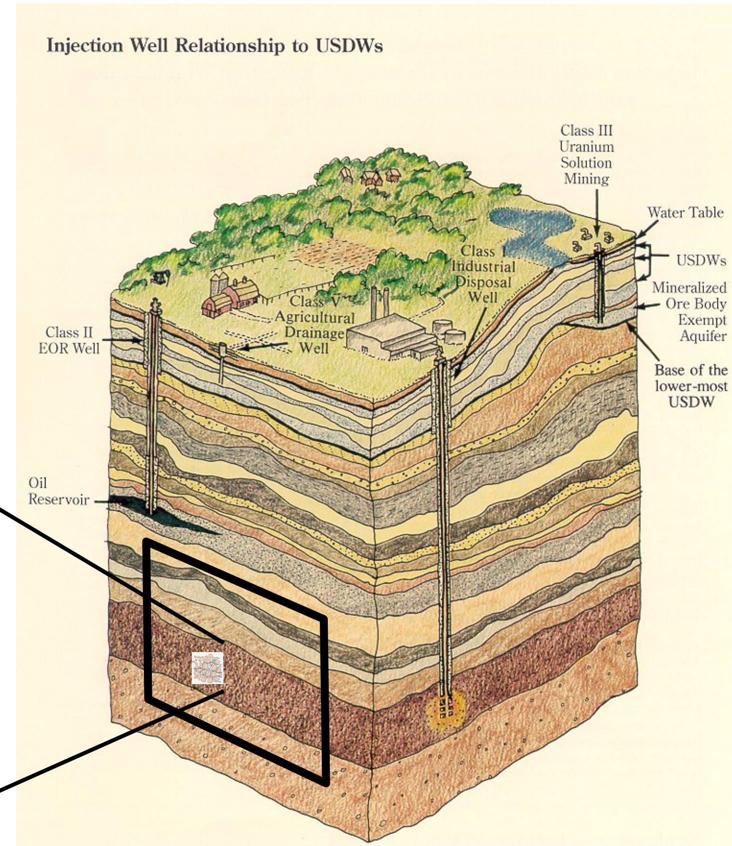
REV: $V = V_{solids} \cup V_{pore}$

Micro solids velocity $\hat{\mathbf{v}}_s$

Micro flow velocity $\hat{\mathbf{v}}_w$

Pore Indicator function χ_{pore}





ω **Macroscopic control volume**

Basic quantities

REV: $V = V_{solids} \cup V_{pore}$

Averaging opérator: $\langle \bullet \rangle = \frac{1}{V} \int_V \bullet \, dV$

Porosity: $n \underset{def}{=} \langle \chi_{pore} \rangle$

Mean pore flow velocity: $\mathbf{v}_w \underset{def}{=} \frac{\langle \chi_{pore} \rho_w \tilde{\mathbf{v}}_w \rangle}{\langle \chi_{pore} \rho_w \rangle}$

Macro control volume: ω

Mean solid velocity: $\mathbf{v}_s \underset{def}{=} \frac{\langle \chi_{solids} \rho_s \tilde{\mathbf{v}}_s \rangle}{\langle \chi_{solids} \rho_s \rangle}$

Control volume velocity: $\mathbf{v}$
(to be defined)

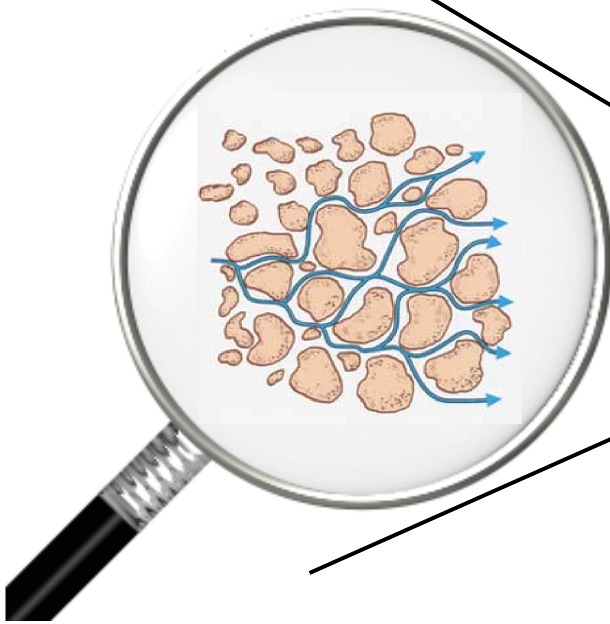
The control volume can *not* be a material volume:
It can not be defined with the same material particles at two different moments
$\mathbf{v}$ **can not be equal at the same time to** $\mathbf{v}_s$ **and to** $\mathbf{v}_w$

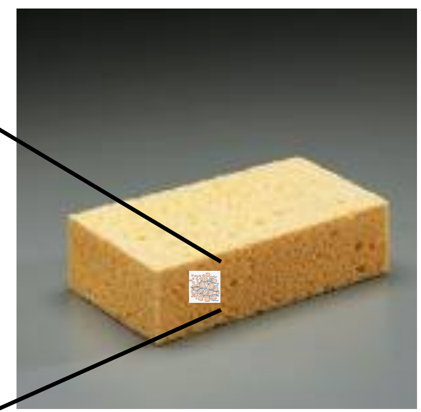
Basic quantities

REV: $V = V_{solids} \cup V_{pore}$

Micro solids velocity $\hat{\mathbf{v}}_s$ **Micro flow velocity** $\hat{\mathbf{v}}_w$

Pore Indicator function χ_{pore}





ω **Control volume**

Total time derivatives

Derivative of quantities (scalars, vectors, tensors):

Derivative with respect to the solid:

$$\frac{d^s}{dt}(\bullet) = \frac{\partial}{\partial t}(\bullet) + \mathbf{v}_s \cdot \nabla(\bullet)$$

Derivative with respect to the fluid:

$$\frac{d^w}{dt}(\bullet) = \frac{\partial}{\partial t}(\bullet) + \mathbf{v}_w \cdot \nabla(\bullet)$$

Reynolds transport theorem (scalars, vectors, tensors):

$$\frac{d}{dt}\int_{\omega}(\bullet)d\omega = \int_{\omega}\frac{\partial}{\partial t}(\bullet)dx + \int_{\partial\omega}(\bullet)\mathbf{v}\cdot\mathbf{n}da$$

$\mathbf{v}$ **: control volume velocity**
(to be defined)

Solid total time derivatives of volume integrals

$\mathbf{v} = \mathbf{v}_s$: the derivative is taken by following the solid in its movement

Derivative with respect to the solid:

$$\frac{d^s}{dt}\int_\omega (\bullet)dx \underset{def}{=} \left.\frac{d}{dt}\right|_{\mathbf{v}=\mathbf{v}_s}\int_\omega (\bullet)dx$$

$$= \int_\omega \frac{\partial}{\partial t}(\bullet)dx + \int_{\partial\omega}(\bullet)\mathbf{v}_s\cdot\mathbf{n}da$$

$$= \int_\omega \frac{\partial}{\partial t}(\bullet)dx + \int_\omega \nabla\cdot\left[(\bullet)\mathbf{v}_s\right]dx$$

$$= \int_\omega \frac{\partial}{\partial t}(\bullet) + \mathbf{v}_s\cdot\nabla(\bullet) + (\bullet)\nabla\cdot\mathbf{v}_s dx$$

$$\boxed{\frac{d^s}{dt}\int_\omega (\bullet)dx = \int_\omega\left[\frac{d^s}{dt}(\bullet) + (\bullet)\nabla\cdot\mathbf{v}_s\right]dx}$$

Fluid total time derivatives of volume integrals

$\mathbf{v} = \mathbf{v}_w$ **: the derivative is taken by following the fluid in its movement**

Derivative with respect to the fluid:

$$\frac{d^w}{dt}\int_\omega (\bullet)dx \underset{def}{=} \left.\frac{d}{dt}\right|_{\mathbf{v}=\mathbf{v}_w} \int_\omega (\bullet)dx$$

$$= \int_\omega \left[\frac{d^w}{dt}(\bullet) + (\bullet)\nabla\cdot\mathbf{v}_w \right] dx$$

And also

$$= \int_\omega \left[\frac{\partial}{\partial t}(\bullet) + \mathbf{v}_w \cdot \nabla(\bullet) + (\bullet)\nabla\cdot\mathbf{v}_w \right] dx$$

$$= \int_\omega \left[\frac{\partial}{\partial t}(\bullet) + \mathbf{v}_s \cdot \nabla(\bullet) + (\mathbf{v}_w - \mathbf{v}_s)\cdot\nabla(\bullet) + (\bullet)\nabla\cdot\mathbf{v}_s + (\bullet)\nabla\cdot(\mathbf{v}_w - \mathbf{v}_s) \right] dx$$

$$= \int_\omega \left[\frac{d^s}{dt}(\bullet) + (\bullet)\nabla\cdot\mathbf{v}_s + \nabla\left[(\bullet)(\mathbf{v}_w - \mathbf{v}_s)\right] \right] dx$$

$$\frac{d^w}{dt}\int_\omega (\bullet)dx = \frac{d^s}{dt}\int_\omega (\bullet)dx + \int_{\partial\omega} (\bullet)(\mathbf{v}_w - \mathbf{v}_s)\cdot\mathbf{n}da$$

Solid mass balance

Solid mass of any domain $M_s(\omega^t, t) = \int_\omega (1-n)\rho_s dx$

Assumption:
- No mass exchange between phases

Mass balance $\forall \omega, \; \frac{d^s}{dt} M_s(\omega, t) = 0$ $\underset{\text{localization theorem}}{\Leftrightarrow}$ $\underbrace{\nabla \cdot \mathbf{v}_s}_{\text{variation of total volume}} = \underbrace{\frac{1}{1-n}\frac{d^s n}{dt}}_{\text{variation of porosity}} - \underbrace{\frac{1}{\rho_s}\frac{d\rho_s}{dt}}_{\text{variation of solid volume}}$

Assumption:
- Homogeneous and rigid solids

(relevant for many - but not all - porous media, irrelevant for rocks, for example)

$\forall \omega,$
$\frac{d^s}{dt} M_s(\omega, t) = 0$ $\underset{\text{localization theorem}}{\Leftrightarrow}$ $\underbrace{\nabla \cdot \mathbf{v}_s}_{\text{variation of total volume}} = \underbrace{\frac{1}{1-n}\frac{d^s n}{dt}}_{\text{variation of porosity}}$ $\Leftrightarrow$ $\mathrm{tr}\,\boldsymbol{\varepsilon} = \ln\left[\frac{1-n^0}{1-n}\right]$

Bulk equation to be used in the following (VPP, energy balance, …)

Bulk equation to be used to evaluate the porosity which appears to be a secondary unknown

Fluid mass balance

Fluid mass of any domain: $$M_w(\omega^t,t)=\int_{\omega} n\rho_w dx$$

Assumption:
- No mass exchange between phases

Mass balance

$$\forall \omega,\quad \frac{d^w}{dt}M_w(\omega^t,t)=0 \quad \underset{\text{localization theorem}}{\Leftrightarrow} \quad \underbrace{\frac{d}{dt}\int_{\omega} n\rho_w dx}_{\text{variation of fluid mass in } \omega} + \underbrace{\int_{\partial\omega} \rho_w \mathbf{q}\cdot\mathbf{n}da}_{\text{mass flux of fluid crossing } \partial\omega} = 0$$

Average relative pore-fluid velocity: $$\mathbf{q}=n(\mathbf{v}_w-\mathbf{v}_s)$$

Fluid mass balance

Assumption:
- Homogeneous and rigid solids

$$\forall \omega, \quad \frac{d^w}{dt} M_w(\omega^t, t) = 0$$

$\Leftrightarrow$ localization theorem

Bulk equation to be discretized (FDM, FEM, FVM, …)

$$\underbrace{n \frac{d^s \rho_w}{dt}}_{\text{fluid density influence}} + \underbrace{\rho_w \nabla \cdot \mathbf{v}_s}_{\text{solid matrix porosity influence}} + \underbrace{\nabla \cdot \left(\rho_w \mathbf{q} \right)}_{\text{fluid mass diffusion}} = 0$$

$$\Leftrightarrow$$

Bulk equation to be used in the following (VPP, energy balance, …)

$$\underbrace{\frac{n}{\rho_w} \frac{d^w \rho_w}{dt}}_{\text{fluid volume strain rate}} + \underbrace{\nabla \cdot \mathbf{v}_s}_{\text{total volume strain rate}} + \underbrace{\nabla \cdot \mathbf{q}}_{\text{fluid diffusion}} = 0$$

Total time derivatives of mass integrals

Accounting for the solid mass balance equations,

the derivative of mass integrals with respect to the solid reads

$$\frac{d^s}{dt}\int_{\omega}(1-n)\rho_s(\bullet)dx = \int_{\omega}(1-n)\rho_s\frac{d^s}{dt}(\bullet)dx$$

Accounting for the fluid mass balance equations,

the derivative of mass integrals with respect to the fluid reads

$$\frac{d^w}{dt}\int_{\omega}n\rho_w(\bullet)dx = \int_{\omega}n\rho_w\frac{d^w}{dt}(\bullet)dx$$

The total time derivative of mass integrals of mixture quantities are therefore

$$\frac{D}{dt}\int_{\omega}n\rho_w(\bullet)dx = \int_{\omega}(1-n)\rho_s\frac{d^s}{dt}(\bullet)dx + \int_{\omega}n\rho_w\frac{d^w}{dt}(\bullet)dx$$

where $\rho(\bullet) = (1-n)\rho_s(\bullet) + n\rho_w(\bullet)$

Kinetic energy

Mixture kinetic energy

$$K(\omega,\mathbf{v}_s,\mathbf{v}_w)=\int_\omega \underbrace{\tfrac{1}{2}(1-n)\rho_s\left(\mathbf{v}_s\right)^2}_{\text{solid kinetic energy}}+\underbrace{\tfrac{1}{2}n\rho_w\left(\mathbf{v}_w\right)^2}_{\text{fluid kinetic energy}}\,dx$$

Total time derivative of the mixture kinetic energy

$$\frac{D}{dt}K(\omega,\mathbf{v}_s,\mathbf{v}_w)=\int_\omega (1-n)\rho_s\mathbf{v}_s\cdot\boldsymbol{\gamma}_s dx+\int_\omega n\rho_w\mathbf{v}_w\cdot\boldsymbol{\gamma}_w dx$$

$\boldsymbol{\gamma}_s=\frac{d^s}{dt}\mathbf{v}_s$ **: solid acceleration**	$\boldsymbol{\gamma}_w=\frac{d^w}{dt}\mathbf{v}_w$ **: fluid acceleration**

Virtual inertia

In terms of $(\hat{\mathbf{v}}_s, \hat{\mathbf{v}}_w)$

$$A(\omega, \hat{\mathbf{v}}_s, \hat{\mathbf{v}}_w) = \int_\omega \underbrace{(1-n)\rho_s \hat{\mathbf{v}}_s \cdot \boldsymbol{\gamma}_s}_{\text{Solids}} \, dx + \int_\omega \underbrace{n\rho_w \hat{\mathbf{v}}_w \cdot \boldsymbol{\gamma}_w}_{\text{Fluid}} \, dx$$

In terms of $(\hat{\mathbf{v}}_s, \hat{\mathbf{q}})$

$$A(\omega, \hat{\mathbf{v}}_s, \hat{\mathbf{q}}) = \underbrace{\int_\omega \Big[\underbrace{(1-n)\rho_s \cdot \boldsymbol{\gamma}_s + n\rho_w \boldsymbol{\gamma}_w}_{\text{Barycentric acceleration}} \Big] \cdot \hat{\mathbf{v}}_s dx}_{\text{Mixture}} + \underbrace{\int_\omega \rho_w \boldsymbol{\gamma}_w \cdot \hat{\mathbf{q}} dx}_{\text{Pore fluid}}$$

However, the current modelling often use a simplified description (more for numerical reasons that for physical evidences) $\boldsymbol{\gamma}_w \approx \boldsymbol{\gamma}_s$

Therefore

$$A(\omega, \hat{\mathbf{v}}_s, \hat{\mathbf{q}}) = \underbrace{\int_\omega \rho \boldsymbol{\gamma}_s \cdot \hat{\mathbf{v}}_s dx}_{\text{Mixture}} + \underbrace{\int_\omega \rho_w \boldsymbol{\gamma}_s \cdot \hat{\mathbf{q}} dx}_{\text{Pore fluid}}$$

Internal virtual power

Given: the set of virtual velocities $H(\omega) = \left\{ (\hat{\mathbf{v}}_s, \hat{\mathbf{v}}_w) \text{ k.a} \right\}$

Assumption:
- First gradient theory

$$\forall \omega,\ \forall (\hat{\mathbf{v}}_s, \hat{\mathbf{v}}_w) \in H(\omega)$$

$$P^{\text{int}}(\omega, \hat{\mathbf{v}}_s, \hat{\mathbf{v}}_w) = -\int_\omega \underbrace{\mathbf{r}_s \cdot \hat{\mathbf{v}}_s + \mathbf{r}_w \cdot \hat{\mathbf{v}}_w}_{\text{order zero}} + \underbrace{\mathbf{T}_s : \nabla \hat{\mathbf{v}}_s + \mathbf{T}_w : \nabla \hat{\mathbf{v}}_w}_{\text{order one}} \, dx$$

Assumption:
- Material indifference

$$P^{\text{int}}(\omega^t, t) = 0 \text{ for any rigid translation} \quad \Leftrightarrow \quad \mathbf{r}_s + \mathbf{r}_w = 0$$

$$P^{\text{int}}(\omega^t, t) = 0 \text{ for any rigid rotation} \quad \Leftrightarrow \quad \left(\mathbf{T}_s + \mathbf{T}_w \right)_{skew} = 0$$

$$P^{\text{int}}(\omega, \hat{\mathbf{v}}_s, \hat{\mathbf{v}}_w) = -\int_\omega \underbrace{\mathbf{r}_w \cdot (\hat{\mathbf{v}}_w - \hat{\mathbf{v}}_s)}_{\text{order zero}} + \underbrace{\boldsymbol{\sigma} : \mathbf{D}(\hat{\mathbf{v}}_s) + \mathbf{T}_w : \nabla(\hat{\mathbf{v}}_w - \hat{\mathbf{v}}_s)}_{\text{order one}} \, dx$$

$\mathbf{D}(\hat{\mathbf{v}}_s) = \left(\nabla \hat{\mathbf{v}}_s \right)_{sym}$	**Virtual strain rate**
$\boldsymbol{\sigma} = \mathbf{T}_s + \mathbf{T}_w$	**Total stress**

Internal virtual power

New choice for the set of virtual velocities $H(\omega)=\left\{(\hat{\mathbf{v}}_s,\hat{\mathbf{q}})\ \text{k.a}\right\}$

Assumption:
- Inviscid fluid (at the macro-scale) $\mathbf{T}_w=-np\mathbf{I}$

$$P^{\text{int}}(\omega,\hat{\mathbf{v}}_s,\hat{\mathbf{q}})=-\int_\omega \mathbf{f}_w\cdot\mathbf{q}+\boldsymbol{\sigma}:\mathbf{D}(\hat{\mathbf{v}}_s)-p\nabla\cdot\hat{\mathbf{q}}dx \qquad \forall\omega,\ \forall(\hat{\mathbf{v}}_s,\hat{\mathbf{q}})\in H(\omega)$$

$\mathbf{f}_w$ **Vector of solid/fluid interaction**

Significance can best be assessed by inserting the fluid mass balance equation

$$P^{\text{int}}(\omega,\hat{\mathbf{v}}_s,\hat{\mathbf{q}})=-\int_\omega \underbrace{\mathbf{f}_w\cdot\mathbf{q}}_{\substack{\text{solid/fluid}\\\text{interaction}\\\text{term}}}+\underbrace{\underbrace{(\boldsymbol{\sigma}+p\mathbf{I})}_{\substack{\text{effective}\\\text{stress}}}:\mathbf{D}(\hat{\mathbf{v}}_s)}_{\substack{\text{solid matrix}\\\text{term}}}+\underbrace{\frac{np}{\rho_w}\frac{d^w\rho_w}{dt}}_{\substack{\text{pore-fluid}\\\text{term}}}dx$$

External loading virtual power

In term of $(\hat{\mathbf{v}}_s, \hat{\mathbf{v}}_w)$

$$P^{\text{ext}}(\omega, \hat{\mathbf{v}}_s, \hat{\mathbf{v}}_w) = \underbrace{\int_\omega \underbrace{(1-n)\rho_s \mathbf{g}\cdot\hat{\mathbf{v}}_s}_{\text{solids}} + \underbrace{n\rho_w \mathbf{g}\cdot\hat{\mathbf{v}}_w}_{\text{fluid}}\, dx}_{\text{bulk loading}} + \underbrace{\int_{\partial\omega} \underbrace{\mathbf{t}_s\cdot\hat{\mathbf{v}}_s}_{\text{solids}} + \underbrace{\mathbf{t}_w\cdot\hat{\mathbf{v}}_w}_{\text{fluid}}\, da}_{\text{boundary loading}}$$

With the new choice of virtual velocities $(\hat{\mathbf{v}}_s, \hat{\mathbf{q}})$

Noticing that, for an inviscid fluid in a porous medium $\mathbf{t}_w = -np_{ext}\mathbf{n}$

$$P^{\text{ext}}(\omega, \hat{\mathbf{v}}_s, \hat{\mathbf{q}}) = \underbrace{\int_\omega \underbrace{\rho \mathbf{g}\cdot\hat{\mathbf{v}}_s}_{\text{mixture}} + \underbrace{\rho_w \mathbf{g}\cdot\hat{\mathbf{q}}}_{\text{pore-fluid}}\, dx}_{\text{bulk loading}} + \underbrace{\int_{\partial\omega} \underbrace{\mathbf{t}\cdot\hat{\mathbf{v}}_s}_{\text{mixture}} - \underbrace{p_{ext}\hat{\mathbf{q}}\cdot\mathbf{n}}_{\text{pore-fluid}}\, da}_{\text{boundary loading}}$$

$\rho = (1-n)\rho_s + n\rho_w$ **Mixture density**

$\mathbf{t}$ **Total traction vector on boundary**

VPP (Virtual Power Principle) : dynamics

$$\forall \omega,\ \forall(\hat{\mathbf{v}}_s,\hat{\mathbf{q}}) \in H(\omega) \qquad P^{\text{int}}(\omega,\hat{\mathbf{v}}_s,\hat{\mathbf{q}}) + P^{\text{ext}}(\omega,\hat{\mathbf{v}}_s,\hat{\mathbf{q}}) - A(\omega,\hat{\mathbf{v}}_s,\hat{\mathbf{q}}) = 0$$

Movement equations

Dynamics VPP $\Leftrightarrow$

Mixture

$$\begin{cases} \nabla \cdot \boldsymbol{\sigma} + \rho(\mathbf{g} - \boldsymbol{\gamma}_s) = 0 \text{ in } \omega & \text{(movement eq.)} \\ \boldsymbol{\sigma}_{skew} = 0 \text{ in } \omega & \\ \boldsymbol{\sigma} \cdot \mathbf{n} = \mathbf{t} \text{ on } \partial\omega & \text{(boundary condition)} \end{cases}$$

Pore-fluid

$$\begin{cases} -\nabla p + \rho_w(\mathbf{g} - \boldsymbol{\gamma}_s) = \mathbf{f}_w \text{ in } \omega & \text{(movement eq.)} \\ p = p_{ext} \text{ on } \partial\omega & \text{(boundary condition)} \end{cases}$$

Kinetic energy theorem

$\forall \omega, \ (\mathbf{v}_s, \mathbf{q})$ **: actual velocities**

$$\frac{D}{dt} K(\omega, \mathbf{v}_s, \mathbf{q}) = P^{\mathrm{int}}(\omega, \mathbf{v}_s, \mathbf{q}) + P^{\mathrm{ext}}(\omega, \mathbf{v}_s, \mathbf{q})$$

VPP (Virtual Power Principle) : quasi-statics

$$\forall \omega,\ \forall(\hat{\mathbf{v}}_s, \hat{\mathbf{q}}) \in H(\omega) \qquad P^{\mathrm{int}}(\omega, \hat{\mathbf{v}}_s, \hat{\mathbf{q}}) + P^{\mathrm{ext}}(\omega, \hat{\mathbf{v}}_s, \hat{\mathbf{q}}) = 0$$

Equilibrium equations

Quasi-static VPP $\Leftrightarrow$

Mixture

$$\begin{cases} \nabla \cdot \boldsymbol{\sigma} + \rho \mathbf{g} = 0 \text{ in } \omega & \text{(equilibrium eq.)} \\ \boldsymbol{\sigma}_{skew} = 0 \text{ in } \omega & \\ \boldsymbol{\sigma} \cdot \mathbf{n} = \mathbf{t} \text{ on } \partial\omega & \text{(boundary condition)} \end{cases}$$

Pore-fluid

$$\begin{cases} -\nabla p + \rho_w \mathbf{g} = \mathbf{f}_w \text{ in } \omega & \text{(equilibrium eq.)} \\ p = p_{ext} \text{ on } \partial\omega & \text{(boundary condition)} \end{cases}$$

Archimède's theorem vs. Terzaghi's principle

Assume hydrostatic conditions: $\mathbf{f}_w = 0$

Inserting the fluid equilibrium eq. into the mixture equilibrium eq. yields:

Solid matrix equilibrium equation

$$\begin{cases} \nabla \cdot \boldsymbol{\sigma}' + \rho' \mathbf{g} = 0 \text{ in } \omega & \text{(equilibrium eq.)} \\ \boldsymbol{\sigma}'_{skew} = 0 \text{ in } \omega & \\ \boldsymbol{\sigma}' \cdot \mathbf{n} = \mathbf{t} + p_{ext} \mathbf{n} \text{ on } \partial\omega & \text{(boundary condition)} \end{cases}$$

$$\rho' = \rho - \rho_w = (1-n)(\rho_s - \rho_w)$$

Buyoant mixture density (Archimede, 250 av. J.-C.)

$$\boldsymbol{\sigma}' = \boldsymbol{\sigma} + p\mathbf{I}$$

Effective stress (Terzaghi, 1925)

Balance of equations and unknowns

Unknowns		Equations		Dimension
n	Porosity	$\frac{d^s n}{dt} = (1-n)\nabla \cdot \mathbf{v}_s$	Solid mass balance eq.	1
p	Pore pressure	$n\frac{d^s \rho_w}{dt} + \rho_w \nabla \cdot \mathbf{v}_s + \nabla \cdot \left(\rho_w \mathbf{q}\right) = 0$	Fluid mass balance eq.	1
ρ_w	Fluid density	?	Fluid behaviour	1
$\mathbf{v}_s$	Solid matrix velocity	$\nabla \cdot \boldsymbol{\sigma} + \rho \mathbf{g} = 0$	Mixture equilibrium eq.	3
$\mathbf{q}$	Pore-fluid velocity	$-\nabla p + \rho_w \mathbf{g} = \mathbf{f}_w$	Pore-fluid equilibrium eq.	3
$\mathbf{f}_w$	Solid/fluid interaction	?	Solid/fluid behaviour	3
$\mathbf{D}$	Strain rate	$\mathbf{D} = \left(\nabla \mathbf{v}_s\right)_{sym}$	Geometric relationship	6
$\boldsymbol{\sigma}'$	Effective stress	?	Solid matrix behaviour	6
$\boldsymbol{\sigma}$	Total stress	$\boldsymbol{\sigma} = \boldsymbol{\sigma}' - p\mathbf{I}$	Terzaghi's principle	6

Balance of energy

Global internal energy: $$E^{\text{int}}(\omega) = \int_{\omega} \rho e dx$$

Global heat power: $$P^{heat}(\omega) = -\int_{\partial\omega} \mathbf{q}_{\theta} \cdot \mathbf{n} da$$

$\mathbf{q}_{\theta}$ **: Heat flux vector**

$$\rho e = \underbrace{(1-n)\rho_s e_s}_{\text{solids}} + \underbrace{n\rho_w e_w}_{\text{fluid}} + \underbrace{\rho e^{mix}}_{\text{coupling term}}$$ **: Mixture internal energy**

Balance of energy

$$\frac{D}{dt} E^{\text{int}}(\omega) + \frac{D}{dt} K(\omega) = P^{heat}(\omega) + P^{ext}(\omega)$$

$$\underset{\text{kinetic energy theorem}}{\Leftrightarrow}$$

$$\frac{D}{dt} E^{\text{int}}(\omega) = P^{heat}(\omega) - P^{\text{int}}(\omega)$$

Energy equation

$$\frac{D}{dt}E^{\text{int}}(\omega) = P^{heat}(\omega) - P^{\text{int}}(\omega)$$

$\Leftrightarrow$
localization theorem

Assumption: $e^{mix} = 0$
(no internal energy coupling term)

Mixture energy equation

$$(1-n)\rho_s \frac{d^s}{dt} e_s + n\rho_w \frac{d^w}{dt} e_w + \nabla \cdot \mathbf{q}_\theta = \mathbf{f}_w \cdot \mathbf{q} + \boldsymbol{\sigma} : \mathbf{D} - p\nabla \cdot \mathbf{q}$$

$\Leftrightarrow$
Inserting the fluid mass balance equation

$$\underbrace{\left[(1-n)\rho_s \frac{d^s}{dt} e_s - \boldsymbol{\sigma}' : \mathbf{D}\right]}_{\text{solids}} + n \underbrace{\left[\rho_w \frac{d^w}{dt} e_w - \frac{p}{\rho_w} \frac{d^w \rho_w}{dt}\right]}_{\text{pore fluid}} + \underbrace{\nabla \cdot \mathbf{q}_\theta}_{\text{heat}} = \underbrace{\mathbf{f}_w \cdot \mathbf{q}}_{\text{solid/fluid interaction}}$$

Imbalance of entropy

Global entropy: $$S(\omega) = \int_{\omega} \rho s dx$$

$$\rho s = \underbrace{(1-n)\rho_s s_s}_{\text{solids}} + \underbrace{n\rho_w s_w}_{\text{fluid}} + \underbrace{\rho s^{mix}}_{\text{coupling term}}$$ **: Mixture entropy**

Assumption: thermal equilibrium of each phase, having therefore the same absolute temperature

Imbalance of entropy

$$\frac{D}{dt} S(\omega) \geq -\int_{\partial\omega} \frac{1}{T} \mathbf{q}_\theta \mathbf{n} da$$

T **Absolute temperature**

Dissipations

Assumption: $s^{mix} = 0$
(no internal entropy coupling term)

Volume intrinsic dissipation

$$\Phi_m \underset{def}{=} T\left[(1-n)\rho_s \frac{d^s}{dt} s_s + n\rho_w \frac{d^w}{dt} s_w\right] + \nabla \cdot \mathbf{q}_\theta$$

Volume heat dissipation

$$\Phi_\theta \underset{def}{=} -\frac{1}{T}\mathbf{q}_\theta \cdot \nabla T$$

$$\frac{D}{dt} S(\omega) \geq -\int_{\partial\omega} \frac{1}{T}\mathbf{q}_\theta \mathbf{n} da \quad \underset{\text{localization theorem}}{\Leftrightarrow} \quad \Phi_m + \Phi_\theta \geq 0$$

Dissipations

$\Psi_s = e - s_s T$: **Solid matrix free energy** $\Psi_w = e_w - s_w T$: **Fluid matrix free energy**

Inserting Φ_m i, as well as Ψ_s and Ψ_w in the energy equation yields

$$\Phi_m = \underbrace{\mathbf{f}_w \cdot \mathbf{q}}_{\text{solid/fluid interaction}} + \underbrace{\left[\boldsymbol{\sigma}' : \mathbf{D} - (1-n)\rho_s s_s \frac{d^s}{dt} T - (1-n)\rho_s \frac{d^s}{dt} \Psi_s \right]}_{\text{solid matrix}}$$

$$+ n \underbrace{\left[\frac{p}{\rho_w} \frac{d^w \rho_w}{dt} - \rho_w s_w \frac{d^w}{dt} T - \rho_w \frac{d^w}{dt} \Psi_w \right]}_{\text{pore fluid}}$$

State variables

State variables

T **Température** ρ_w **Fluid density** $\boldsymbol{\varepsilon} = \left(\nabla \mathbf{u}_s\right)_{sym}$ **Elastic small strain**

Assumptions

$$\Psi_w \equiv \Psi_w(T, \rho_w) \qquad \Psi_s \equiv \Psi_s(T, \boldsymbol{\varepsilon})$$

(Matrix elasticity, fluid compressibility and thermal effects)

$$\mathbf{D}(\mathbf{v}_s) = \left(\nabla \mathbf{v}_s\right)_{sym} = \left(\nabla\left[\frac{d^s}{dt}\mathbf{u}_s\right]\right)_{sym} = \frac{d^s}{dt}\left(\nabla \mathbf{u}_s\right)_{sym} = \frac{d^s}{dt}\boldsymbol{\varepsilon}$$

(Small strains)

Therefore

$$\Phi_m = \underbrace{\mathbf{f}_w \cdot \mathbf{q}}_{\text{solid/fluid interaction}} + \underbrace{\left[\left(\boldsymbol{\sigma}' - (1-n)\rho_s \frac{\partial \Psi_s}{\partial \boldsymbol{\varepsilon}}\right) : \mathbf{D} + (1-n)\rho_s\left[-s_s - \frac{\partial \Psi_s}{\partial T}\right]\frac{d^s}{dt}T\right]}_{\text{solid matrix}}$$

$$+ \underbrace{n\left[\left(\frac{p}{\rho_w} - \rho_w \frac{\partial \Psi_w}{\partial \rho_w}\right)\frac{d^w \rho_w}{dt} + \rho_w\left(-s_w - \frac{\partial \Psi_w}{\partial T}\right)\frac{d^w}{dt}T\right]}_{\text{pore fluid}}$$

State laws

By usual reasoning, the state laws are as follows

Solid matrix elasticity	$\boldsymbol{\sigma}' = (1-n)\rho_s \dfrac{\partial \Psi_s}{\partial \boldsymbol{\varepsilon}}$	$s_s = -\dfrac{\partial \Psi_s}{\partial T}$	Solid entropy
Fluid compressibility	$p = (\rho_w)^2 \dfrac{\partial \Psi_w}{\partial \rho_w}$	$s_w = -\dfrac{\partial \Psi_w}{\partial T}$	Fluid entropy

Rows: Solid matrix (first row); Pore fluid (second row).

Columns: Mechanics (first two columns); Thermics (last two columns).

The intrinsic dissipation reduces to the solid/fluid interaction

$$\Phi_m = \mathbf{f}_w \cdot \mathbf{q}$$

Energy equation and dissipations

Finally, the energy equation - which is not yet the heat equation - reads

$$T\left[(1-n)\rho_s \frac{d^s}{dt} s_s + n\rho_w \frac{d^w}{dt} s_w\right] + \nabla \cdot \mathbf{q}_\theta = \mathbf{f}_w \cdot \mathbf{q}$$

The dissipations are

$$\Phi_m = \mathbf{f}_w \cdot \mathbf{q} \qquad \Phi_\theta \underset{def}{=} -\frac{1}{T} \mathbf{q}_\theta \cdot \nabla T$$

A sufficient - but not necessary condition to fulfill the imbalance entropy is

$$\Phi_m \geq 0 \qquad \Phi_\theta \geq 0$$

Balance of equations and unknowns: poro-mechanics

Unknowns		Equations	Dimension	
n	**Porosity**	$\frac{d^s n}{dt} = (1-n)\nabla \cdot \mathbf{v}_s$	**Solid mass balance eq.**	**1**
p	**Pore pressure**	$n\frac{d^s \rho_w}{dt} + \rho_w \nabla \cdot \mathbf{v}_s + \nabla \cdot \left(\rho_w \mathbf{q}\right) = 0$	**Fluid mass balance eq.**	**1**
ρ_w	**Fluid density**	$p = \left(\rho_w\right)^2 \frac{\partial \Psi_w}{\partial \rho_w}$	**Fluid behaviour**	**1**
$\mathbf{u}_s$	**Solid matrix displacement**	$\nabla \cdot \boldsymbol{\sigma} + \rho \mathbf{g} = 0$	**Mixture equilibrium eq.**	**3**
$\mathbf{q}$	**Pore-fluid velocity**	$-\nabla p + \rho_w \mathbf{g} = \mathbf{f}_w$	**Pore-fluid equilibrium eq.**	**3**
$\mathbf{f}_w$	**Solid/fluid interaction**	$\mathbf{f}_w \cdot \mathbf{q} \geq 0$	**Solid/fluid dissipation**	**3**
$\boldsymbol{\varepsilon}$	**Small strain**	$\boldsymbol{\varepsilon} = \left(\nabla \mathbf{u}_s\right)_{sym}$	**Geometric relationship**	**6**
$\boldsymbol{\sigma}'$	**Effective stress**	$\boldsymbol{\sigma}' = (1-n)\rho_s \frac{\partial \Psi_s}{\partial \boldsymbol{\varepsilon}}$	**Solid matrix behaviour**	**6**
$\boldsymbol{\sigma}$	**Total stress**	$\boldsymbol{\sigma} = \boldsymbol{\sigma}' - p\mathbf{I}$	**Terzaghi’s principle**	**6**

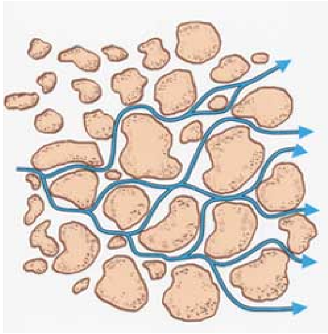
Balance of equations and unknowns: thermics

Unknowns		Equations		Dimension
s_s	Solid entropy	$s_s = -\frac{\partial \Psi_s}{\partial T}$	Solid matrix behaviour.	1
s_w	Fluid entropy	$s_w = -\frac{\partial \Psi_w}{\partial T}$	Fluid matrix behaviour.	1
T	Absolute temperature		Energy equation	1

$$T\left[(1-n)\rho_s \frac{d^s}{dt} s_s + n\rho_w \frac{d^w}{dt} s_w\right] + \nabla \cdot \mathbf{q}_\theta = \mathbf{f}_w \cdot \mathbf{q}$$

$\mathbf{q}_\theta$	Heat vector	$-\frac{1}{T}\mathbf{q}_\theta \cdot \nabla T \geq 0$	Thermal dissipation	1

Milieux poreux
Poro-Mécanique
Couplages hydro-mécanique



Thermal diffusion, mass diffusion
Fourier's and Darcy's laws
Heat and seepage equations

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Balance of equations and unknowns: deformations

Unknowns		Equations		Dimension
n	**Porosity**	$\frac{d^s n}{dt} = (1-n)\nabla \cdot \mathbf{v}_s$	**Solid mass balance eq.**	**1**
$\mathbf{u}_s$	**Solid matrix displacement**	$\nabla \cdot \boldsymbol{\sigma} + \rho \mathbf{g} = 0$	**Mixture equilibrium eq.**	**3**
$\boldsymbol{\varepsilon}$	**Small strain**	$\boldsymbol{\varepsilon} = \left(\nabla \mathbf{u}_s\right)_{sym}$	**Geometric relationship**	**6**
$\boldsymbol{\sigma}'$	**Effective stress**	$\boldsymbol{\sigma}' = (1-n)\rho_s \frac{\partial \Psi_s}{\partial \boldsymbol{\varepsilon}}$	**Solid matrix behaviour**	**6**
$\boldsymbol{\sigma}$	**Total stress**	$\boldsymbol{\sigma} = \boldsymbol{\sigma}' - p\mathbf{I}$	**Terzaghi's principle**	**6**

Effective stress

The total stress is defined by the static equilibrium equation

$$\nabla \cdot \boldsymbol{\sigma} = 0 \qquad \left(\operatorname{tr} \boldsymbol{\sigma} < 0 \;\Leftrightarrow\; \text{compression} \right)$$

The effective stress is defined as follows (Terzaghi, 1925)

$$\boldsymbol{\sigma}' = \boldsymbol{\sigma} + p\mathbf{I}$$

The effective stress differs than the total stress only on the isotropic part

$$\tfrac{1}{3} \operatorname{tr} \boldsymbol{\sigma}' = \tfrac{1}{3} \operatorname{tr} \boldsymbol{\sigma} + p$$

$$\boldsymbol{\sigma}'^{d} = \boldsymbol{\sigma}^{d}$$

The behaviour law of the solid matrix involves the effective stress, for example, isotropic linear elasticity in small strains

$$\boldsymbol{\sigma}' = 2G\boldsymbol{\varepsilon} + \operatorname{tr} \boldsymbol{\varepsilon} \left(\chi - \frac{2G}{3} \right) \mathbf{I} \qquad G = \frac{E}{2(1+\nu)} \qquad \chi = \frac{E}{3(1-2\nu)}$$

(shear modulus) (bulk modulus)

Balance of equations and unknowns: thermics

Unknowns		Equations		Dimension
s_s	**Solid entropy**	$s_s = -\dfrac{\partial \Psi_s}{\partial T}$	**Solid matrix behaviour.**	**1**
s_w	**Fluid entropy**	$s_w = -\dfrac{\partial \Psi_w}{\partial T}$	**Fluid matrix behaviour.**	**1**
T	**Absolute temperature**		**Energy equation**	**1**

$$T\left[(1-n)\rho_s \frac{d^s}{dt} s_s + n\rho_w \frac{d^w}{dt} s_w\right] + \nabla \cdot \mathbf{q}_\theta = \mathbf{f}_w \cdot \mathbf{q}$$

$\mathbf{q}_\theta$	**Heat vector**	$-\dfrac{1}{T}\mathbf{q}_\theta \cdot \nabla T \geq 0$	**Thermal dissipation**	**1**

Specific heat od solids and fluid

Solid matrix

$$\begin{cases} s_s = -\dfrac{\partial \Psi_s}{\partial T} \\ \Psi_s \equiv \Psi_s(T, \boldsymbol{\varepsilon}) \end{cases} \Rightarrow \quad T \frac{d^s}{dt} s_s = c_s \frac{d^s}{dt} T - T \left(\frac{\partial \Psi_s}{\partial T \partial \boldsymbol{\varepsilon}} \right) \frac{d^s}{dt} \boldsymbol{\varepsilon}$$

$$c_s = -T \frac{\partial^2 \Psi_s}{\partial T^2}$$ **Specific heat of the solid matrix**

Pore fluid

$$\begin{cases} s_w = -\dfrac{\partial \Psi_w}{\partial T} \\ \Psi_s \equiv \Psi_s(T, \rho_w) \end{cases} \Rightarrow \quad T \frac{d^w}{dt} s_w = c_w \frac{d^w}{dt} T - T \left(\frac{\partial \Psi_s}{\partial T \partial \rho_w} \right) \frac{d^w}{dt} \rho_w$$

$$c_w = -T \frac{\partial^2 \Psi_w}{\partial T^2}$$ **Specific heat of the pore fluid**

Specific heat of the porous medium

Solid matrix **Pore fluid**

$$\begin{cases} s_s = -\dfrac{\partial \Psi_s}{\partial T} & s_w = -\dfrac{\partial \Psi_w}{\partial T} \\ \Psi_s \equiv \Psi_s(T,\boldsymbol{\varepsilon}) & \Psi_s \equiv \Psi_s(T,\rho_w) \end{cases} \Rightarrow \quad T\left[(1-n)\rho_s \frac{d^s}{dt} s_s + n\rho_w \frac{d^w}{dt} s_w\right] = \rho c \frac{d^s}{dt} T + \rho_w c_w \nabla T \cdot \mathbf{q} - T r_\Psi$$

$$\rho c = \underbrace{(1-n)\rho_s c_s}_{\text{solids}} + \underbrace{n\rho_w c_w}_{\text{fluid}}$$

Specific heat of the mixture

$$r_\Psi = (1-n)\rho_s \underbrace{\left(\frac{\partial \Psi_s}{\partial T \partial \boldsymbol{\varepsilon}}\right)}_{\text{solid dilatation}} \frac{d^s}{dt}\boldsymbol{\varepsilon} + n\rho_w \underbrace{\left(\frac{\partial \Psi_s}{\partial T \partial \rho_w}\right)}_{\text{fluid dilatation}} \frac{d^w}{dt}\rho_w$$

Volume power due to dilatation

Heat equation in the porous medium

$$\underbrace{\rho c \frac{d^s}{dt} T + \nabla \cdot \mathbf{q}_\theta}_{\text{Heat diffusion in the porous medium}} = \underbrace{\underbrace{\mathbf{f}_w \cdot \mathbf{q}}_{\text{Intrinsic dissipation (elastic solid matrix)}} + \underbrace{Tr_\Psi}_{\text{Power due to dilatation}}}_{\text{Often neglected}} \underbrace{- \rho_w c_w \nabla T \cdot \mathbf{q}}_{\text{Heat transport by seepage (advection)}}$$

Fourier's law

$$\Phi_\theta \geq 0 \quad \Leftarrow \quad \begin{cases} \mathbf{q}_\theta = -\dfrac{\partial}{\partial \nabla T} \Omega_\theta (\nabla T, \text{state variables}) \\ \Omega_\theta = \frac{1}{2} (\nabla T) \cdot \boldsymbol{\kappa} \cdot (\nabla T) \\ \boldsymbol{\kappa} \quad \text{Symmetric definite positive order two tensor} \end{cases}$$

This is the Fourier's law $\mathbf{q}_\theta = -\boldsymbol{\kappa} \cdot \nabla T$

Here, we have

$$\text{state variables} = (T, \rho_w, \boldsymbol{\varepsilon})$$

$\boldsymbol{\kappa} = \boldsymbol{\kappa}(\text{state variables})$ **Thermal conductivity of the porous medium**

Balance of equations and unknowns: seepage

Unknowns		Equations		Dimension
p	**Pore pressure**	$n\frac{d^s\rho_w}{dt}+\rho_w\nabla\cdot\mathbf{v}_s+\nabla\cdot\left(\rho_w\mathbf{q}\right)=0$	**Fluid mass balance eq.**	**1**
ρ_w	**Fluid density**	$p=\left(\rho_w\right)^2\frac{\partial\Psi_w}{\partial\rho_w}$	**Fluid behaviour**	**1**
$\mathbf{q}$	**Pore-fluid velocity**	$-\nabla p+\rho_w\mathbf{g}=\mathbf{f}_w$	**Pore-fluid equilibrium eq.**	**3**
$\mathbf{f}_w$	**Solid/fluid interaction**	$\mathbf{f}_w\cdot\mathbf{q}\geq 0$	**Solid/fluid dissipation**	**3**

Water state law

The water state law can be found in many books

For current applications with FEM codes, the following satet law is often used

$$\rho_w = \rho_w^{ref} \exp\left(\frac{p}{\chi_w}\right)$$

$\chi_w \approx 2$ GPa **Bulk water modulus**

Seepage diffusion law

$$\mathbf{f}_w \cdot \mathbf{q} \geq 0 \quad \Leftarrow \quad \begin{cases} \mathbf{q} = \dfrac{\partial}{\partial \mathbf{f}_w} \Omega_{fs}(\mathbf{f}_w, \text{state variables}) \\ \Omega_{fs} = \frac{1}{2} \mathbf{f}_w \cdot \boldsymbol{\kappa}_{fs} \cdot \mathbf{f}_w \\ \boldsymbol{\kappa}_{fs} \quad \text{Symmetric definite positive order two tensor} \end{cases}$$

This is a diffusion law $\mathbf{q} = \boldsymbol{\kappa}_{fs} \cdot \mathbf{f}_w$

Here, we have

$$\text{state variables} = (T, \rho_w, \boldsymbol{\varepsilon})$$

$\boldsymbol{\kappa}_{fs} = \boldsymbol{\kappa}_{fs}(\text{state variables})$ **Hydraulic conductivity of the porous medium**

Darcy's law (mechanics-like form)

Actually, the hydraulic conductivity may be written as

$$\boldsymbol{\kappa}_{fs} = \frac{1}{\rho_w \eta_w(T)} \boldsymbol{\Lambda}_s(T, \boldsymbol{\varepsilon})$$

$\boldsymbol{\Lambda}_s$ (m^2)	**Geometric permeability of the solid matrix**
η_s $(\mathrm{m}^2/\mathrm{s})$	**Kinematic fluid viscosity**

Inserting the equilibrium eq. of the pore fluid yields

Darcy's law $$\mathbf{q} = \frac{1}{\rho_w \eta_w} \boldsymbol{\Lambda}_s \cdot \left[-\nabla p + \rho_w \mathbf{g} \right]$$

Darcy's law (engineering-like form)

The hydraulic conductivity may also be written as

$$\boldsymbol{\kappa}_{fs} = \frac{1}{\gamma_w}\mathbf{K} \qquad \textbf{where} \qquad \gamma_w = \rho_w g$$

$\mathbf{K}$ (m/s)	**Hydraulic permeability of the porous medium**
g (m/s^2)	**Gravitational constant**

The hydraulic head is defined as follows

$$H = \frac{p - p_{atm}}{\gamma_w} + z \text{ (m)}$$

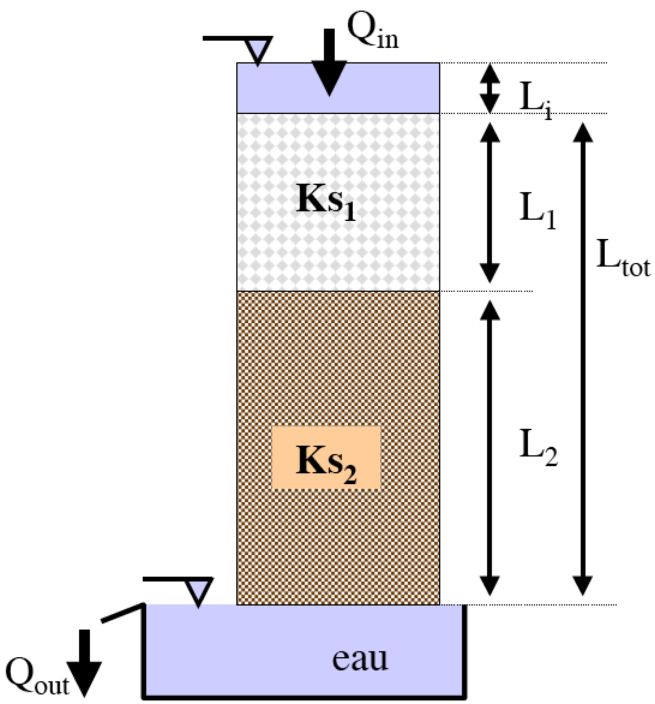
Type of soil	k (m/s)
gravel	$10^{-3} - 10^{-1}$
sand	$10^{-6} - 10^{-3}$
silt	$10^{-8} - 10^{-6}$
clay	$10^{-10} - 10^{-8}$

Inserting the equilibrium eq. of the pore fluid yields

Darcy's law $\qquad \mathbf{q} = -\mathbf{K} \cdot \nabla H$

Where the (hidden) assumptions are: 1) incompressible fluid, 2) $p_{atm} = 0$ **(reference pressure)**

Milieu stratifié



$$K_{s\,eff} = \frac{L_1 + L_2}{\dfrac{L_1}{K_{S1}} + \dfrac{L_2}{K_{S2}}}$$

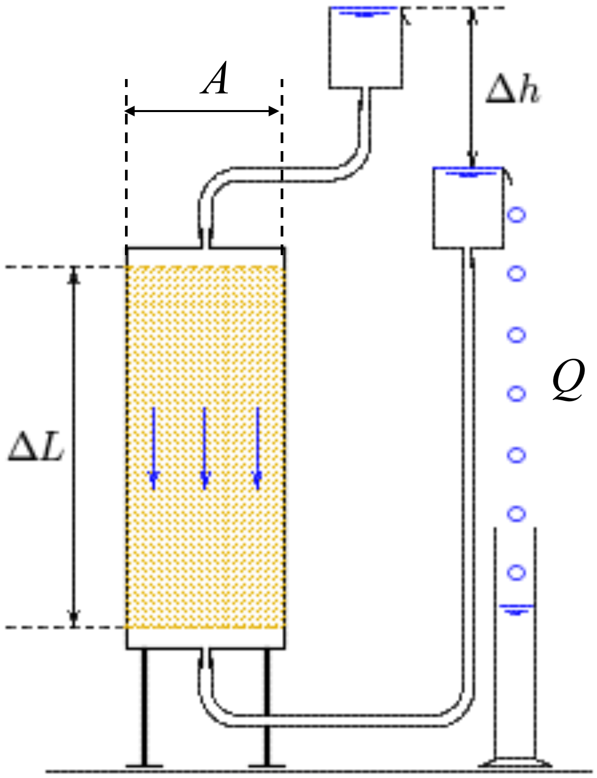
$$K_{eff,z} = \frac{\sum_j L_j}{\sum_j \dfrac{L_j}{K_j}}$$

perpendiculairement aux strates: moyenne harmonique

$$K_{eff,x} = \frac{\sum_j K_j L_j}{\sum_j L_j}$$

parallèlement aux strates: moyenne géométrique

Testing: constant head permeability test



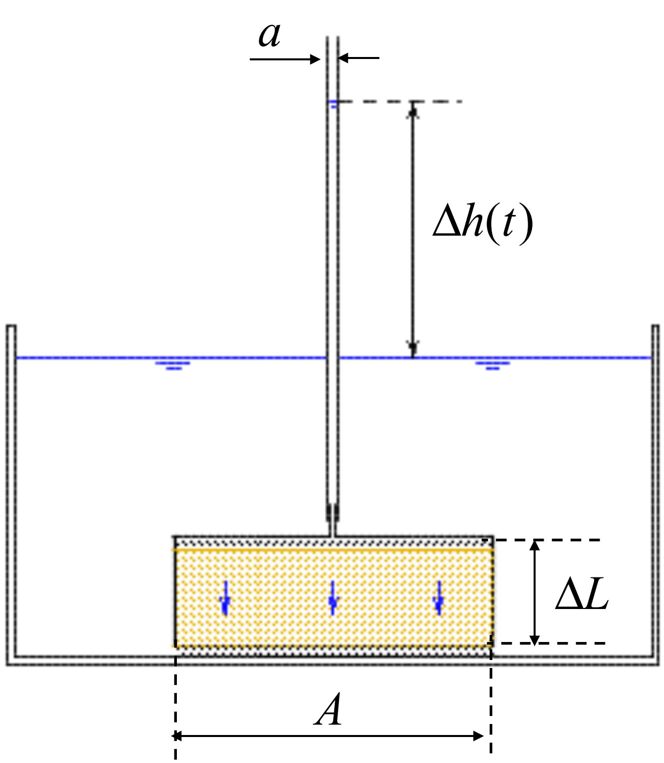
Δh : constant head drop (m)

Q : total discharge (m^3/s)

A : area of soil sample (m^2)

ΔL : length of soil sample (m)

Testing: falling head test



$\Delta h(t)$: variable head drop (m)

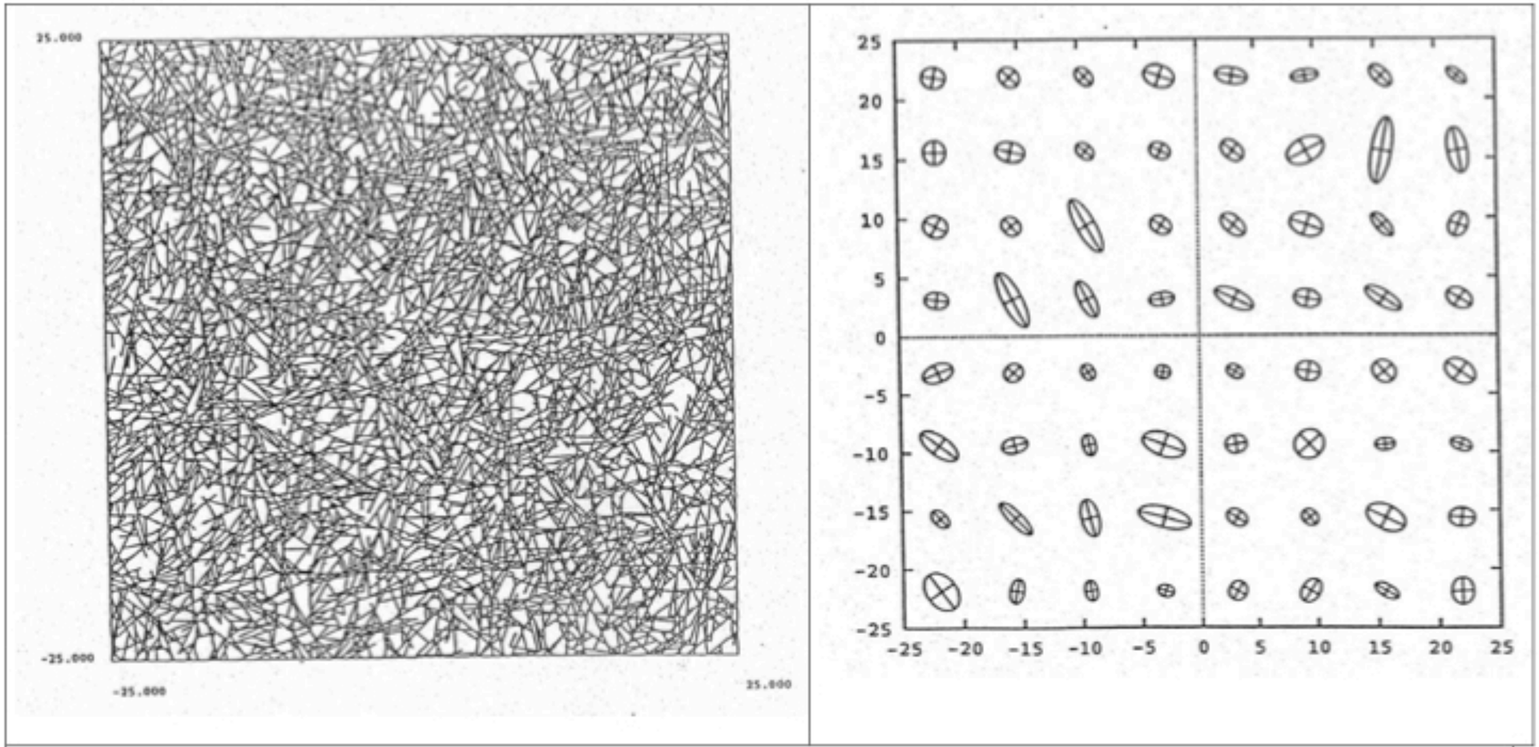
$Q(t)$: total discharge (m^3/s)

a : area of tube above soil (m^2)

A : area of soil sample (m^2)

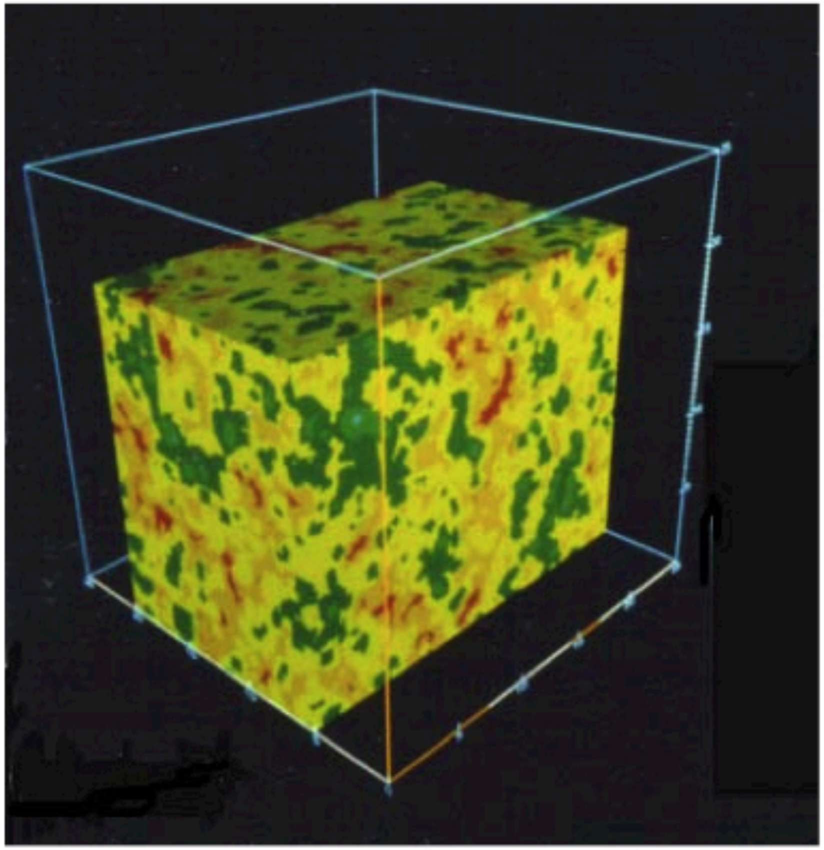
ΔL : length of soil sample (m)

Perméabilité anisotrope



Réseau plan de fractures (environ 7000 fractures) et les perméabilités tensorielles équivalentes du domaine divisé en 64 sous-blocs (les Kij sont représentées par les ellipses d'anisotropie). [source : R.A. et al. 1994]

Perméabilité micro



PERMEABILITE D'UN MASSIF POREUX ALEATOIRE GENERE NUMERIQUEMENT EN 3D
(CHAMP ALEATOIRE AUTOCORRELE A STRUCTURE ISOTROPE)

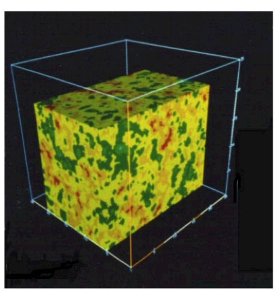
Résultats théorique et numérique connus

Solide
rigide et impermeable
+
Ecoulement de Navier-Stokes
dans les pores

$$\tilde{\nabla} \cdot \tilde{\mathbf{v}}_w = 0$$

$$\rho_w \left(\frac{\partial \tilde{\mathbf{v}}_w}{\partial t} + \tilde{\nabla} \tilde{\mathbf{v}}_w \cdot \tilde{\mathbf{v}}_w \right) = \tilde{\nabla} \tilde{\boldsymbol{\sigma}}$$

$$\tilde{\boldsymbol{\sigma}} = -\tilde{p}\mathbf{I} + \rho_w \eta_w \left(\tilde{\nabla} \tilde{\mathbf{v}}_w \right)_{sym}$$



PERMEABILITE D'UN MASSIF POREUX ALEATOIRE GENERE NUMERIQUEMENT EN 3D
(CHAMP ALEATOIRE AUTOCORRELE A STRUCTURE ISOTROPE)

$$\underbrace{\tilde{l}}_{\text{pore scale}} \ll \underbrace{\mathrm{L}}_{\text{REV scale}}$$

Homogeneisation
(changement d'échelle
Micro→macro)

$$\varepsilon = \frac{\tilde{l}}{L} \ll 1$$

Matrice solide rigide
+
Ecoulement de Darcy

$$\nabla \cdot \mathbf{q} = 0$$

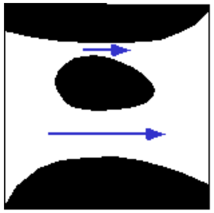
$$\mathbf{q} = -\frac{1}{\rho_w \eta_w} \boldsymbol{\Lambda}_s \cdot \nabla p$$

avec

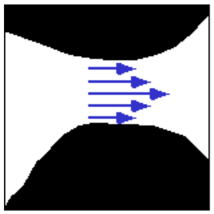
$$\mathbf{q} \underset{def}{=} \left\langle \chi_{pore} \tilde{\mathbf{v}}_w \right\rangle$$

p **: terme du 1er ordre en** ε

Origines microscopique de la dissipation

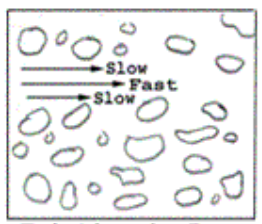


different pore sizes

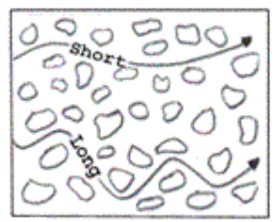


different velocities in a pore

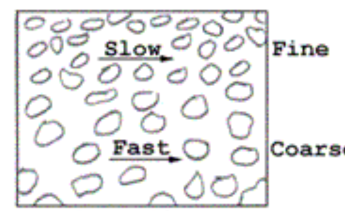
different pathways around the grains



Friction in Pore



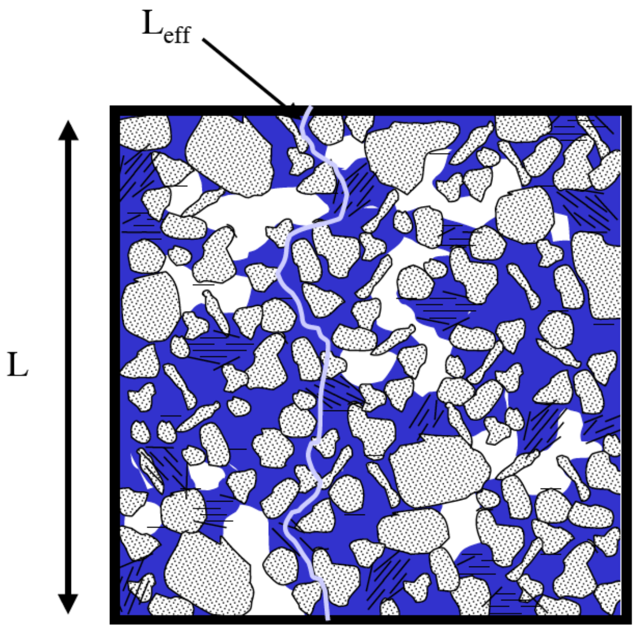
Path Length

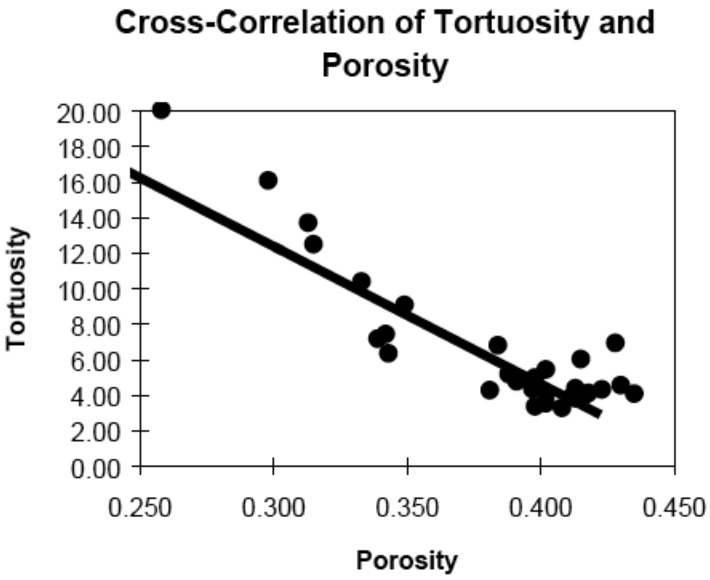


Pore Size

Stéphane Bonelli

Tortuosity





$$\tau \approx 35.6 - 77.3n$$

Tortuosity

$$\tau = \left(\frac{L_{eff}}{L}\right)^2$$

Rough approximation

$$\tau \approx \frac{25}{12} \approx 2$$

The porous medium viewed as a bundle of tubes

Hagen-Poiseuille's law

$$\left\langle \tilde{v}_w \right\rangle_{tubes} = \frac{R^2}{8\rho_w \eta_w} \frac{\Delta p}{L_{eff}}$$

Assumption

$$\left\langle \tilde{v}_w \right\rangle_{pore} = \frac{L}{L_{eff}} \left\langle \tilde{v}_w \right\rangle_{tubes}$$

Darcy's law

$$q = \frac{\lambda}{\rho_w \eta_w} \frac{\Delta p}{L}$$

$$q = n\left\langle \tilde{v}_w \right\rangle_{pore} = n\frac{L}{L_{eff}}\left\langle \tilde{v}_w \right\rangle_{tubes} = \frac{nR^2}{8\rho_w \eta_w \tau}\left(\frac{\Delta p}{L}\right)$$

$$\lambda = \frac{nR^2}{8\tau}$$

Hydraulic radius

Tube

$$R_h = \frac{R}{2}$$

Porous medium

$$R_h = \frac{n}{a_V}$$

$$R = \frac{2n}{a_V}$$

$$\lambda = \frac{n^3}{2\tau (a_V)^2}$$

The Kozeny-Karman relationship for granular materials

General relationship

$$\lambda = \frac{n^3}{2\tau(a_V)^2}$$

n: porosity

a_V: volume specific pore surface

τ: tortuosity

The Kozeny-Karman relationship for granular materials

$$\left.\begin{aligned} \tau &\approx \frac{25}{12} \\ a_V &= (1-n)\frac{6}{D_{grain}} \end{aligned}\right\} \quad \lambda = \frac{n^3 D_{grain}^2}{150(1-n)^2} \quad \Rightarrow \quad \lambda = \lambda_{ref}\left(\frac{n}{n_{ref}}\right)^3\left(\frac{1-n_{ref}}{1-n}\right)^2$$

Geometric permeabilities for fine materials

General relationship (assuming pore surface≈solid surface)

$$\lambda = \frac{n^3 d_{eq}^2}{2\tau(1-n)^2}$$

n: porosity

$a_{W_{solids}}$: mass specific solid surface

τ: tortuosity

$d_{eq} = (\rho_s a_{W_{solids}})^{-1}$: equivalent grain size

$$\Rightarrow \quad \lambda = \lambda_{ref} \left(\frac{n}{n_{ref}} \right)^3 \left(\frac{1-n_{ref}}{1-n} \right)^2$$

More accurate descriptions can be found in petrophysics, relating permeabilities and porosities to others physical quantities (like the cation exchange capacity, or the electrical conductivities)

In brief: Deformations

$$\nabla \cdot \boldsymbol{\sigma} + \rho \mathbf{g} = 0$$ (Total equilibrium eq.)

$$\boldsymbol{\sigma}' = 2G\boldsymbol{\varepsilon} + \mathrm{tr}\,\boldsymbol{\varepsilon}\left(\chi - \frac{2G}{3}\right)\mathbf{I}$$ (Pore fluid state law)

$$\boldsymbol{\sigma} = \boldsymbol{\sigma}' - p\mathbf{I}$$ (Terzaghi's principle)

Constitutive laws

Material informations:

χ	**Bulk solid matrix modulus**
G	**Shear solid matrix modulus**

Coupling:

p **Pore-pressure**

T **Temperature (For conciseness, strain due to dilatation is not explicited)**

In brief: Seepage equation

$$n\frac{d^s\rho_w}{dt}+\rho_w\nabla\cdot\mathbf{v}_s+\nabla\cdot\left(\rho_w\mathbf{q}\right)=0$$

(Pore fluid mass balance)

Constitutive laws:

$$\rho_w=\rho_w^{ref}\exp\left(\frac{p}{\chi_w}\right)$$

(Pore fluid state law)

$$\mathbf{q}=\frac{1}{\rho_w\eta_w}\mathbf{\Lambda}_s\cdot\left[-\nabla p+\rho_w\mathbf{g}\right]$$

(Darcy' s law)

Material informations:

χ_w	**Bulk water modulus**
$\rho_w\eta_w$	**Water viscosity**
$\mathbf{\Lambda}$	**Geometric permeability**

Coupling: $(n,\mathbf{v}_s)$ **Solid matrix deformation on the mass balance** $(\nabla\cdot\mathbf{v}_s)$ **and the permeability** $\mathbf{\Lambda}$

T **Temperature on** ρ_w,η_w,χ_w

In brief: Heat equation

$$\rho c \frac{d^s}{dt} T + \nabla \cdot \mathbf{q}_\theta + \rho_w c_w \nabla T \cdot \mathbf{q} = 0$$ (Energy balance)

$$\mathbf{q}_\theta = -\boldsymbol{\kappa} \cdot \nabla T$$ (Fourier's law) } **Constitutive law**

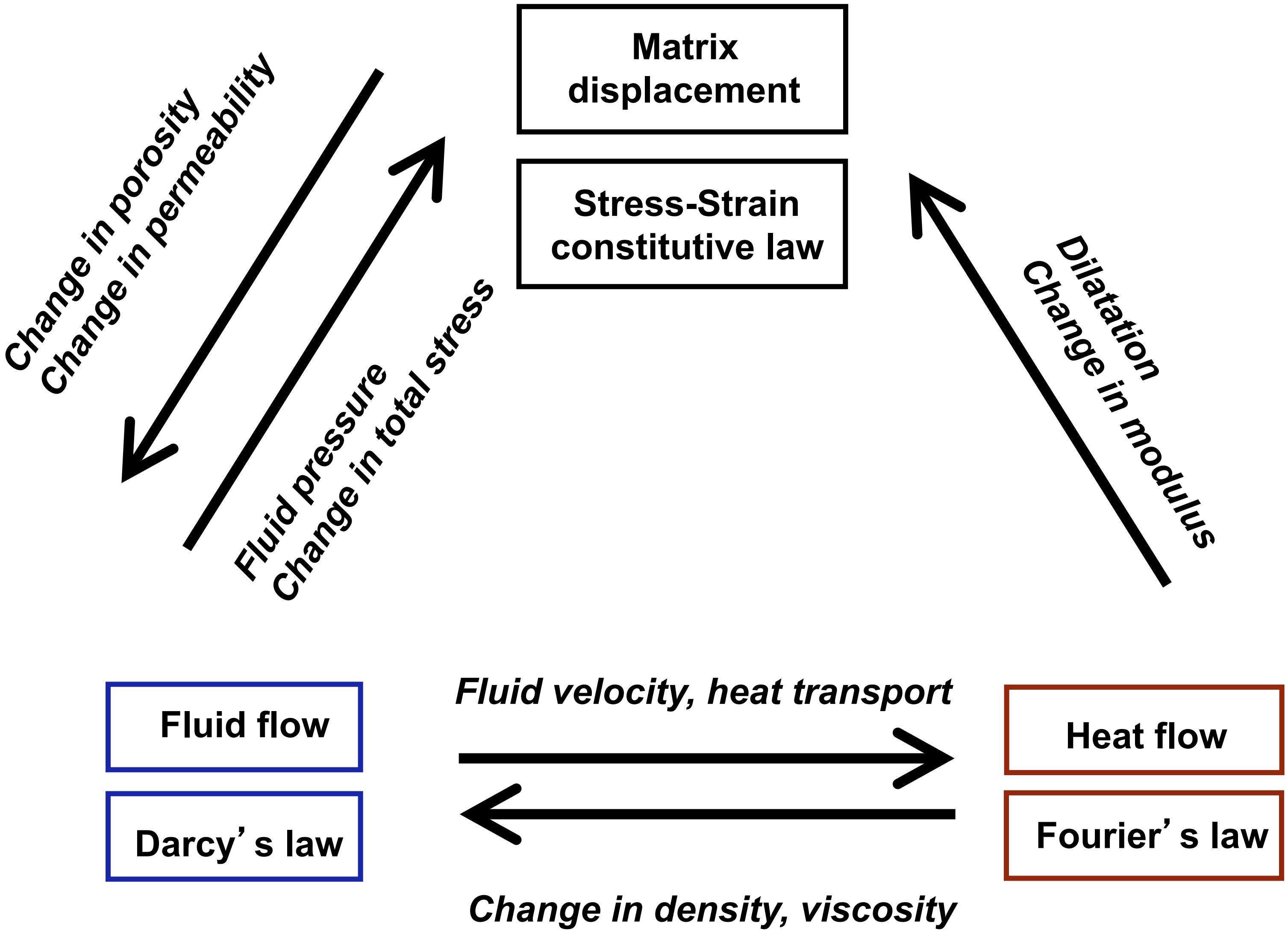
Material informations:

$$\rho c = \underbrace{(1-n)\rho_s c_s}_{\text{solids}} + \underbrace{n\rho_w c_w}_{\text{fluid}}$$ **Specific heat of the mixture**

$$\boldsymbol{\kappa} = \boldsymbol{\kappa}(\text{state variables})$$ **Thermal conductivity of the porous medium**

Coupling: $\mathbf{q}$ **heat transport by seepage flow (advection)**

THM coupling



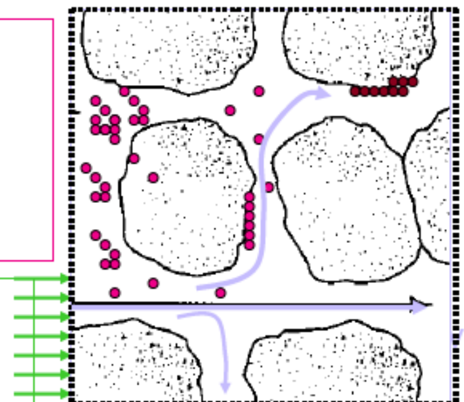
Couplages THMC: transport de particules

transport et transfert des polluants

bilan de masse: $\mathrm{div}\,(\underline{f} + \underline{f} + \underline{f}) + \partial_t m = 0$

Diffusion moléculaire

$\underline{f} = -\mathbf{D}(c)\ \underline{\mathrm{grad}}\ c$



sorption, atténuation, transformation

$\partial_t m$

Convection

$\underline{f} = \underline{V}_r c$

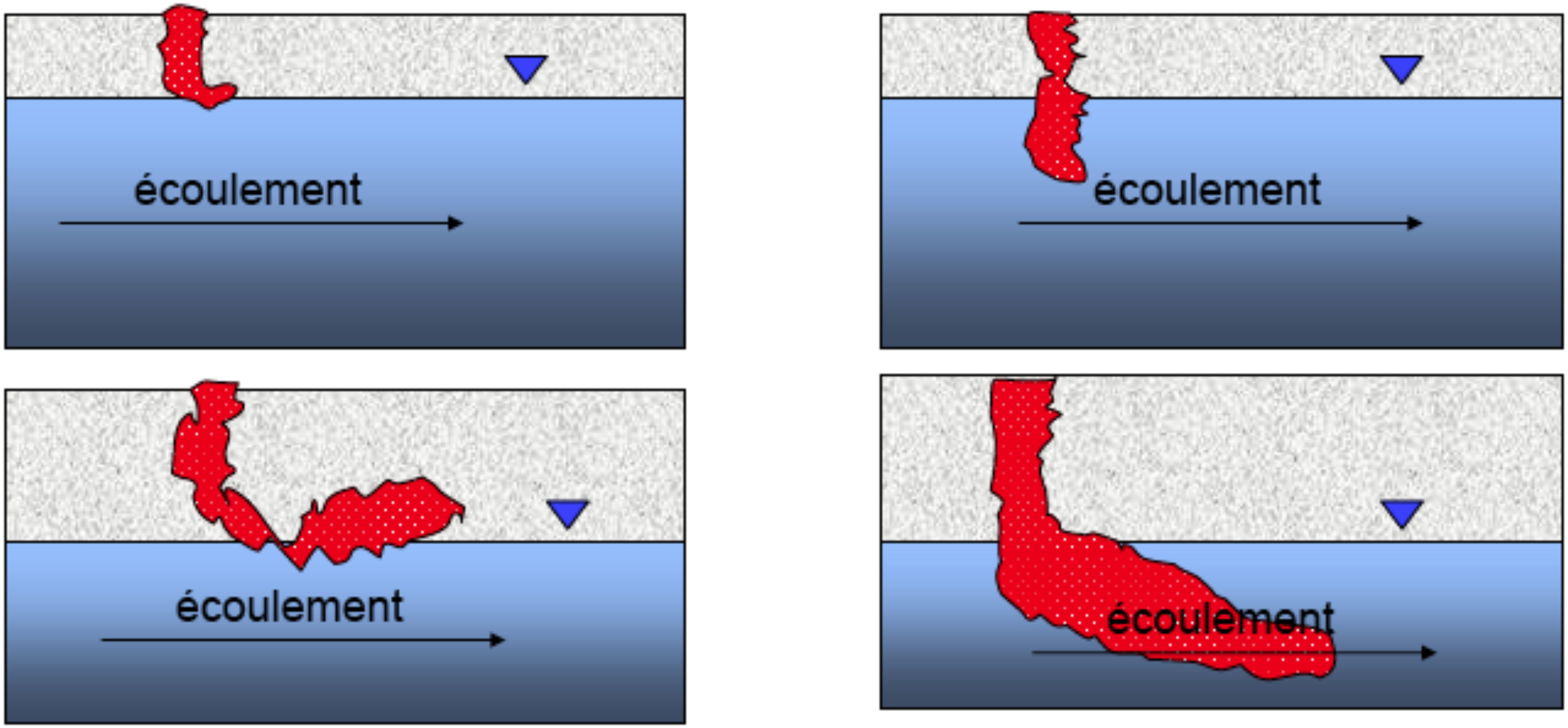
$\underline{V}_r$: loi de Darcy

Dispersion

$\underline{f} = \mathbf{D}_m(\underline{V})\ \underline{\mathrm{grad}}\ c$

42

Couplages THMC: pollutions de nappes

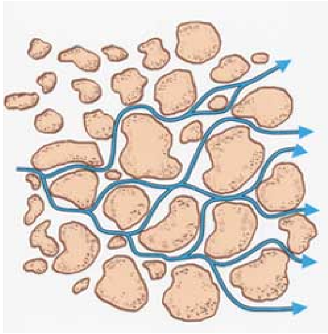


LNAPL
(hydrocarbures)

DNAPLS
(solvants chlorés)

38

Milieux poreux
Poro-Mécanique
Couplages hydro-mécanique



Elastoplasticité
Mohr-Coulomb
Cam-Clay

Stéphane Bonelli
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Notations

Consider any order two-tensor $(\mathbf{a},\mathbf{c},\mathbf{b})$

Identity order-two tensor: $\mathbf{I} \underset{def}{/} \left\{\forall \mathbf{a},\ \mathbf{I}\cdot\mathbf{a} = \mathbf{a}\cdot\mathbf{I}\right\}$

Trace operator: $\mathrm{tr}\,\mathbf{a} \underset{def}{=} \mathbf{I}:\mathbf{a} = \mathbf{a}:\mathbf{I}$

Tensorial products

$$\times \underset{def}{/} \left\{\forall(\mathbf{a},\mathbf{c},\mathbf{b}) \text{ order two tensors, } \left[\mathbf{a}\times\mathbf{b}\right]:\mathbf{c} = \left(\mathbf{b}:\mathbf{c}\right)\mathbf{a} = \mathbf{c}:\left[\mathbf{b}\times\mathbf{a}\right]\right\}$$

$$\otimes \underset{def}{/} \left\{\forall(\mathbf{a},\mathbf{c},\mathbf{b}) \text{ order two tensors, } \left[\mathbf{a}\otimes\mathbf{b}\right]:\mathbf{c} = \mathbf{a}\cdot\mathbf{c}\cdot\mathbf{b} = \mathbf{c}:\left[\mathbf{a}^T\otimes\mathbf{b}^T\right]\right\}$$

Decomposition into spheric and deviatoric parts

$$\mathbf{a} = \underbrace{a_m\mathbf{I}}_{\text{spheric part}} + \underbrace{\mathbf{a}^d}_{\text{deviatoric part}}$$

$$a_m\mathbf{I} = \underbrace{\left[\tfrac{1}{3}\mathbf{I}\times\mathbf{I}\right]}_{\text{spheric projector}}:\mathbf{a} = \left(\tfrac{1}{3}\mathrm{tr}\,\mathbf{a}\right)\mathbf{I}$$

$$\mathbf{a}^d = \underbrace{\left[\mathbf{I}\otimes\mathbf{I} - \tfrac{1}{3}\mathbf{I}\times\mathbf{I}\right]}_{\text{deviatoric projector}}:\mathbf{a} = \mathbf{a} - \left(\tfrac{1}{3}\mathrm{tr}\,\mathbf{a}\right)\mathbf{I}$$

Invariants of order-two tensors

Eigenvalues of any symmetric order two tensor $\mathbf{a}$

$$\det\left(\mathbf{a}-\lambda\mathbf{I}\right)=0 \quad\Leftrightarrow\quad \lambda=a_m+\lambda^d,\quad \det\left(\mathbf{a}^d-\lambda^d\mathbf{I}\right)=0$$

$$\det\left(\mathbf{a}^d-\lambda^d\mathbf{I}\right)=0 \quad\Leftrightarrow\quad \left(\lambda^d\right)^3-\tfrac{1}{2}(\mathbf{a}^d:\mathbf{a}^d)\lambda^d-\det(\mathbf{a}^d)=0$$

Trigo tools: $\cos 3\theta = 4\cos^3\theta - 3\cos\theta \qquad \sin 3\omega = 3\sin\omega - 4\sin^3\omega$

$$\lambda_i^d \underset{i=1,2,3}{=} \sqrt{\tfrac{2}{3}\mathbf{a}^d:\mathbf{a}^d}\cos\left[\theta+\tfrac{2\pi}{3}(1-i)\right] \underset{i=1,2,3}{=} \sqrt{\tfrac{2}{3}\mathbf{a}^d:\mathbf{a}^d}\sin\left[\omega+\tfrac{2\pi}{3}(2-i)\right]$$

$$\cos 3\theta \underset{def\ \theta}{=} \frac{27\det\mathbf{a}^d}{2\left(\tfrac{3}{2}\mathbf{a}^d:\mathbf{a}^d\right)^{3/2}} \qquad \sin 3\omega \underset{def\ \omega}{=} -\frac{27\det\mathbf{a}^d}{2\left(\tfrac{3}{2}\mathbf{a}^d:\mathbf{a}^d\right)^{3/2}}$$

$$0\le\theta\le\tfrac{\pi}{3} \qquad \omega=\frac{\pi}{6}-\theta \qquad -\tfrac{\pi}{6}\le\omega\le\tfrac{\pi}{6}$$

Principal stress invariants

Mean stress: $\sigma_m = \frac{1}{3} \operatorname{tr} \boldsymbol{\sigma}$

Traction $\sigma_m > 0$ **Compression** $\sigma_m < 0$

Von-Mises equivalent stress (1913): $\sigma_{eq} = \sqrt{\frac{3}{2} \boldsymbol{\sigma}^d : \boldsymbol{\sigma}^d}$

Shear stress intensity

Lode's angle (1925): $\cos 3\theta_\sigma = \dfrac{27 \det \boldsymbol{\sigma}^d}{2\sigma_{eq}^{3/2}}$ **or** $\sin 3\omega_\sigma = -\dfrac{27 \det \boldsymbol{\sigma}^d}{2\sigma_{eq}^{3/2}}$

Shear type (pure shear, simple shear, extension, …)

Principal stresses as a function of principal stress invariants

$$\sigma_I > \sigma_{II} > \sigma_{III}$$

$$\begin{cases} \sigma_I = \sigma_m + \frac{2}{3}\sigma_{eq}\cos\theta_\sigma \\ \sigma_{II} = \sigma_m + \frac{2}{3}\sigma_{eq}\cos(\theta_\sigma - \frac{2\pi}{3}) \\ \sigma_{III} = \sigma_m + \frac{2}{3}\sigma_{eq}\cos(\theta_\sigma + \frac{2\pi}{3}) \end{cases}$$

or

$$\begin{cases} \sigma_I = \sigma_m + \frac{2}{3}\sigma_{eq}\sin(\omega_\sigma + \frac{2\pi}{3}) \\ \sigma_{II} = \sigma_m + \frac{2}{3}\sigma_{eq}\sin\omega_\sigma \\ \sigma_{III} = \sigma_m + \frac{2}{3}\sigma_{eq}\sin(\omega_\sigma - \frac{2\pi}{3}) \end{cases}$$

$$0 \le \theta \le \frac{\pi}{3}$$

$$\omega = \theta - \frac{\pi}{6}$$

$$-\frac{\pi}{6} \le \omega \le \frac{\pi}{6}$$

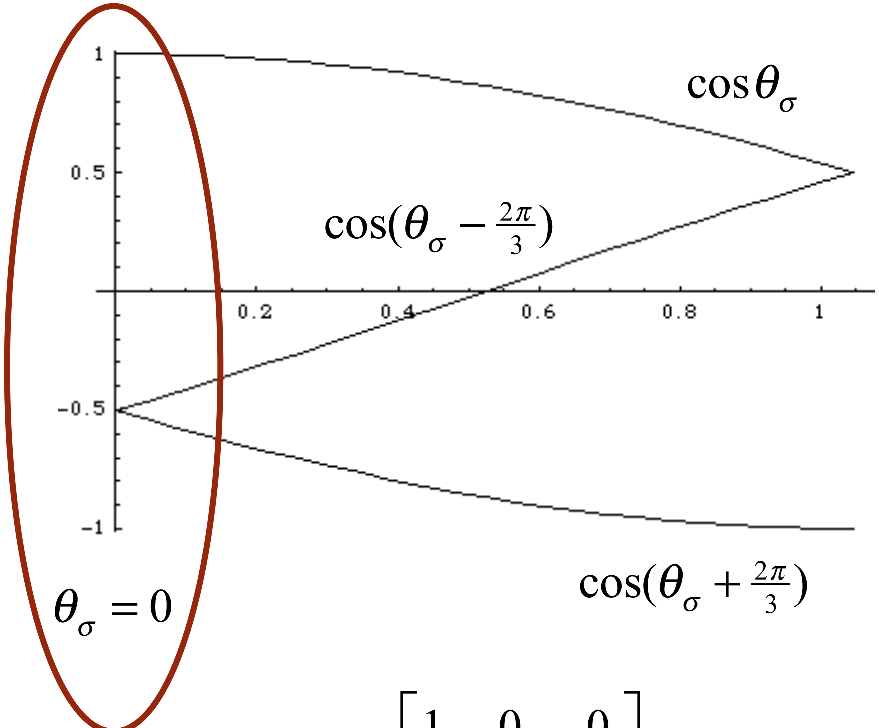
Principal stress invariants as a function of principal stresses

Mean stress: $\sigma_m = \frac{1}{3}(\sigma_I + \sigma_{II} + \sigma_{III})$

Von-Mises equivalent stress: $\sigma_{eq} = \sqrt{\frac{1}{2}\left[(\sigma_I - \sigma_{II})^2 + (\sigma_{II} - \sigma_{III})^2 + (\sigma_{III} - \sigma_I)^2\right]}$

Lode's angle: $\cos 3\theta_\sigma = \dfrac{9(2\sigma_{III} - \sigma_I - \sigma_{II})(2\sigma_{II} - \sigma_I - \sigma_{III})(2\sigma_I - \sigma_{II} - \sigma_{III})}{2(\sigma_I^2 + \sigma_{II}^2 + \sigma_{III}^2 - \sigma_I\sigma_{II} - \sigma_{II}\sigma_{III} - \sigma_I\sigma_{III})^{3/2}}$

Shear type vs. Lode's angle: extension



$$\theta_\sigma = 0$$

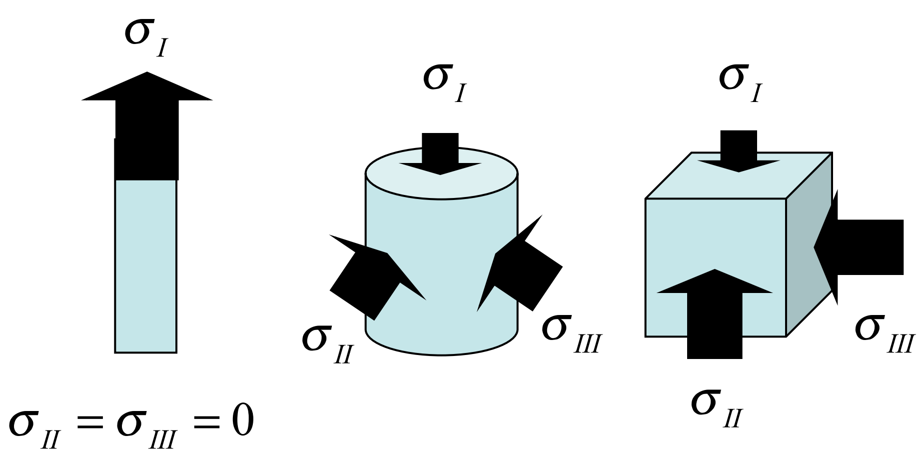
$$\boldsymbol{\sigma} = \begin{bmatrix} \sigma_m + \sigma_I^d & 0 & 0 \\ 0 & \sigma_m - \frac{1}{2}\sigma_I^d & 0 \\ 0 & 0 & \sigma_m - \frac{1}{2}\sigma_I^d \end{bmatrix}$$

$$\sigma_I > \sigma_{II} = \sigma_{III}$$

Extension along σ_I

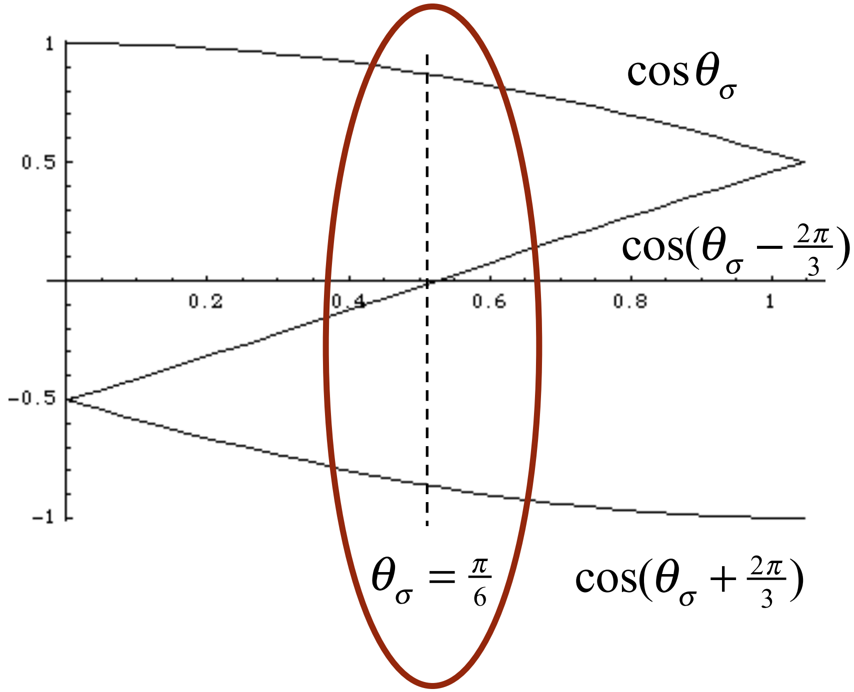
$$\boldsymbol{\sigma}^d = \tfrac{2}{3}\sigma_{eq} \begin{bmatrix} 1 & 0 & 0 \\ 0 & -\frac{1}{2} & 0 \\ 0 & 0 & -\frac{1}{2} \end{bmatrix}$$

$$\begin{cases} \sigma_{II}^d = \sigma_{III}^d = -\frac{1}{2}\sigma_I^d \\ \sigma_I^d > 0 \end{cases} \Rightarrow \begin{cases} \sigma_{eq} = \frac{3}{2}\sigma_I^d \\ \cos 3\theta_\sigma = 1 \end{cases}$$



Examples

Shear type vs. Lode's angle: pure shear



$$\theta_\sigma = \frac{\pi}{6}$$

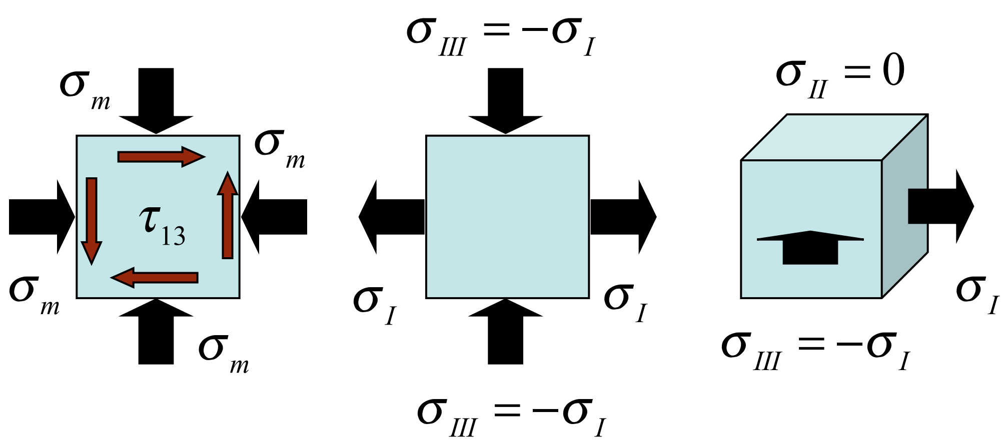
$$\boldsymbol{\sigma} = \begin{bmatrix} \sigma_m + |\sigma_I^d| & 0 & 0 \\ 0 & \sigma_m & 0 \\ 0 & 0 & \sigma_m - |\sigma_I^d| \end{bmatrix}$$

$$\sigma_I > \sigma_{II} = \sigma_m > \sigma_{III}$$

Shear orthogonal to σ_2

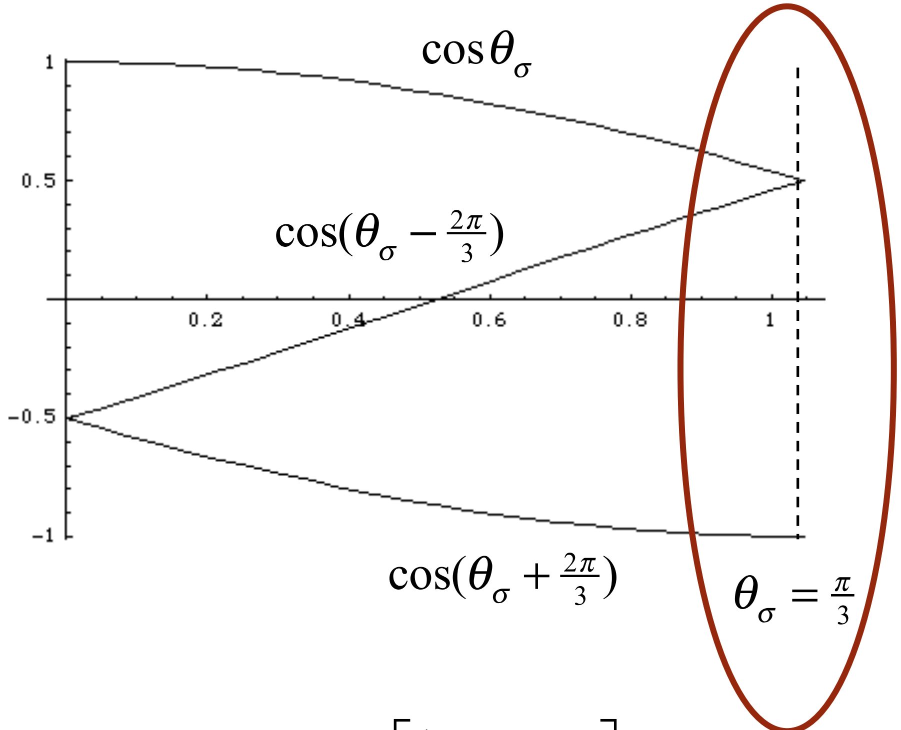
$$\boldsymbol{\sigma}^d = \tfrac{2}{3}\sigma_{eq} \begin{bmatrix} \frac{\sqrt{3}}{2} & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & -\frac{\sqrt{3}}{2} \end{bmatrix}$$

$$\begin{cases} \sigma_I^d = -\sigma_{III}^d \\ \sigma_I^d = 0 \end{cases} \Rightarrow \begin{cases} \sigma_{eq} = \sqrt{3}|\sigma_I^d| \\ \cos 3\theta_\sigma = 0 \end{cases}$$



Examples

Shear type vs. Lode's angle: compression



$$\theta_\sigma = \frac{\pi}{3}$$

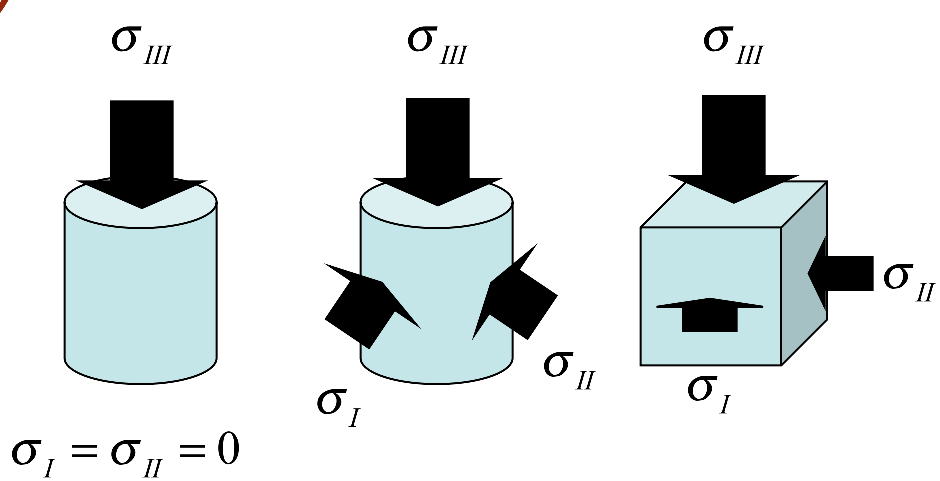
$$\boldsymbol{\sigma} = \begin{bmatrix} \sigma_m + \frac{1}{2}\sigma^d_{III} & 0 & 0 \\ 0 & \sigma_m + \frac{1}{2}\sigma^d_{III} & 0 \\ 0 & 0 & \sigma_m - \sigma^d_{III} \end{bmatrix}$$

$$\sigma_I > \sigma_{II} = \sigma_{III}$$

Compression along σ_{III}

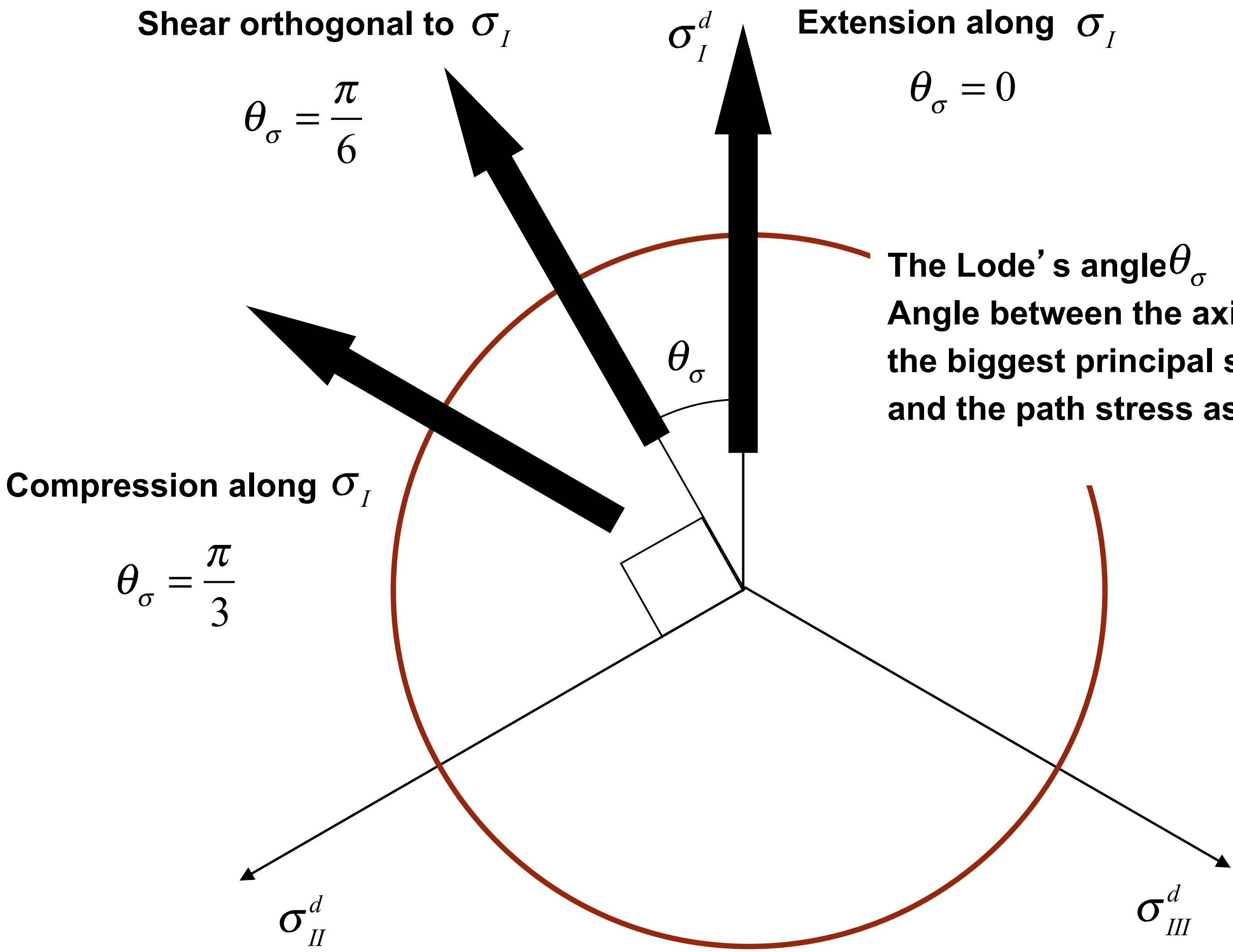
$$\boldsymbol{\sigma}^d = \tfrac{2}{3}\sigma_{eq} \begin{bmatrix} \frac{1}{2} & 0 & 0 \\ 0 & \frac{1}{2} & 0 \\ 0 & 0 & -1 \end{bmatrix}$$

$$\begin{cases} \sigma^d_I = \sigma^d_{II} = -\frac{1}{2}\sigma^d_{III} \\ \sigma^d_{III} < 0 \end{cases} \Rightarrow \begin{cases} \sigma_{eq} = -\frac{3}{2}\sigma^d_{III} \\ \cos 3\theta_\sigma = -1 \end{cases}$$



Examples

Shear type vs. Lode's angle



The Lode's angle θ_σ gives
Angle between the axis of
the biggest principal stress σ_I
and the path stress as defined by the vector

$$\begin{Bmatrix} \sigma_{II} \\ \sigma_{II} \\ \sigma_{III} \end{Bmatrix}$$

Principal strain-rate invariants

Volume strain rate: $\dot{\varepsilon}_v = \mathrm{tr}\,\dot{\boldsymbol{\varepsilon}}$

Dilatancy $\dot{\varepsilon}_v > 0$ **Contractancy** $\dot{\varepsilon}_v < 0$

Equivalent strain rate $\dot{\varepsilon}_{eq} = \sqrt{\frac{2}{3}\dot{\boldsymbol{\varepsilon}}^d : \dot{\boldsymbol{\varepsilon}}^d}$

Shear strain rate intensity

Power equivalence

$\boldsymbol{\sigma}' : \dot{\boldsymbol{\varepsilon}} = \sigma_{eq}\dot{\varepsilon}_{eq} + \sigma'_m \dot{\varepsilon}_v$ **if** $\boldsymbol{\sigma}'$ **and** $\dot{\boldsymbol{\varepsilon}}$ **have the same eigen vectors**

Elastoplastic models for porous and for granular materials

Assumptions: small strains, isotropic material
(for conciseness only, thing are much pore complicated for large strains and/or anisotropy

Strain decomposition

$$\boldsymbol{\varepsilon} = \underbrace{\boldsymbol{\varepsilon}^e}_{\text{reversible strain}} + \underbrace{\boldsymbol{\varepsilon}^p}_{\text{irreversible strain}}$$

Elasticity

$$\dot{\boldsymbol{\sigma}}' = \underset{\sim}{\mathbf{D}}^e(\boldsymbol{\sigma}') : \dot{\boldsymbol{\varepsilon}}^e$$

Plasticity

Yield locus

$$f(\boldsymbol{\sigma}', R) \leq 0$$

Flow rule (non associated, not standard, not generalized)

- Plastic strains

$$\dot{\boldsymbol{\varepsilon}}^p = \dot{\lambda}\mathbf{Q}(\boldsymbol{\sigma}', R)$$

- Isotropic hardening

$$\dot{R} = -\dot{\lambda} H(R)$$

- Consistency condition

$$\dot{\lambda} \geq 0, \ \dot{\lambda} f(\boldsymbol{\sigma}', R) = 0$$

Elastic models for porous and for granular materials

Isotropic non-linear elasticity

$$\dot{\boldsymbol{\sigma}}' = \underset{\sim}{\mathbf{D}}^e(\boldsymbol{\sigma}') : \dot{\boldsymbol{\varepsilon}}^e$$

-usually written in incremental form
-not always thermodynamics consistent
-always non-linear
-usually depend on the mean effective stress $\sigma'_m = \frac{1}{3}\operatorname{tr}\boldsymbol{\sigma}'$

$$\underset{\sim}{\mathbf{D}}^e(\boldsymbol{\sigma}') = 2\,\underbrace{G(\sigma'_m)}_{\text{shear modulus}}\,\underbrace{\left[\mathbf{I}\otimes\mathbf{I} - \tfrac{1}{3}\mathbf{I}\times\mathbf{I}\right]}_{\text{deviatoric projector}} + 3\,\underbrace{\chi(\sigma'_m)}_{\text{bulk modulus}}\,\underbrace{\tfrac{1}{3}\mathbf{I}\times\mathbf{I}}_{\text{spheric projector}}$$

$G(\sigma'_m) = \chi(\sigma'_m)\dfrac{3(1-2\nu)}{2(1+\nu)}$: **shear modulus**

$\chi(\sigma'_m)$: **bulk modulus**

ν : **constant Poisson's coefficient**

Elastic models for porous and for granular materials

Granular materials

$$\chi(\sigma'_m) = \begin{cases} \chi^e \left(\dfrac{-\sigma'_m}{p_{ref}} \right)^n & \text{if } \sigma'_m < 0 \text{ (for compression only)} \\ \text{not defined if } \sigma'_m > 0 \text{ (traction not allowed)} & \end{cases}$$

where

χ^e **: reference bulk modulus (kPa)**

p_{ref} **: reference stress (kPa)**

n **: exponent** $(0 < n < 1,$ usual value n=0.6)

Elastic models for porous and for granular materials

Clays materials

$$\chi(\sigma'_m) = \begin{cases} \dfrac{1+e}{\kappa^e}(-\sigma'_m) \text{ if } \sigma'_m < 0 \text{ (for compression only)} \\ \text{not defined if } \sigma'_m > 0 \text{ (traction not allowed)} \end{cases}$$

where

κ^e **: elastic index (dimensionless)**

e **: initial void ratio, usually considered as constant and equal the initial void ratio in small strains** e^0

Questions

The void ratio is another engineering quantity widely used in porous mechanics. Void ratio is defined as follows:

$$e \underset{def}{=} \frac{V_{pore}}{V_{solids}}$$

The solids material is rigid.

1. Express the void ratio as a function of the porosity.

2. Express the volume strain as a function of the porosity.

3. Express the volume strain as a function of the void ratio.

4. Express the volume strain rate as a function of the porosity.

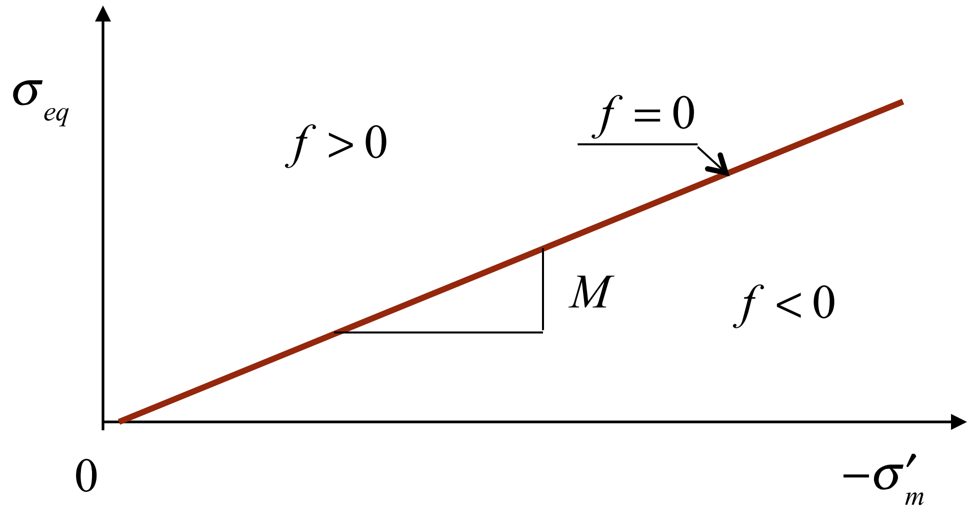
5. Express the volume strain rate as a function of the void ratio.

6. Express the mean effective stress as a function of the void ratio for a clay matrix and a clay elastic model.

Drucker-Prager failure criterion for cohesionless media (1952)

Yield locus

$$f(\boldsymbol{\sigma}') = \sigma_{eq} + M\sigma'_m$$



Evolution rule (not associated)

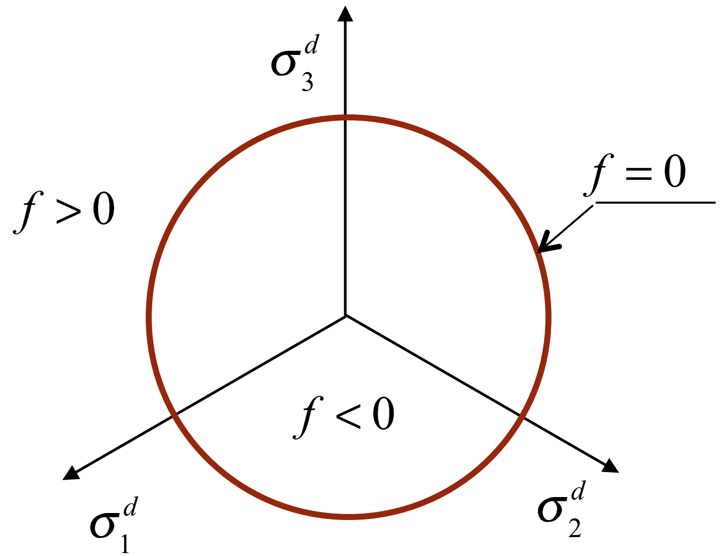
$$\dot{\boldsymbol{\varepsilon}}^p = \dot{\lambda}\left[\frac{M_\Psi}{3}\mathbf{I} + \frac{3}{2\sigma_{eq}}\boldsymbol{\sigma}^d\right]$$

Two material constants

$$M = \left.\frac{\sigma_{eq}}{-\sigma'_m}\right|_{failure} \qquad M_\Psi = \left.\frac{\dot{\varepsilon}_v^p}{\dot{\varepsilon}_{eq}^p}\right|_{failure}$$

$$\boldsymbol{\sigma} : \dot{\boldsymbol{\varepsilon}}^p = \dot{\lambda}\sigma'_m(M_\Psi - M)$$

$$\begin{cases} \boldsymbol{\sigma} : \dot{\boldsymbol{\varepsilon}}^p > 0 \\ \boldsymbol{\sigma}' < 0 \end{cases} \Leftrightarrow \quad 0 \le M_\Psi \le M$$



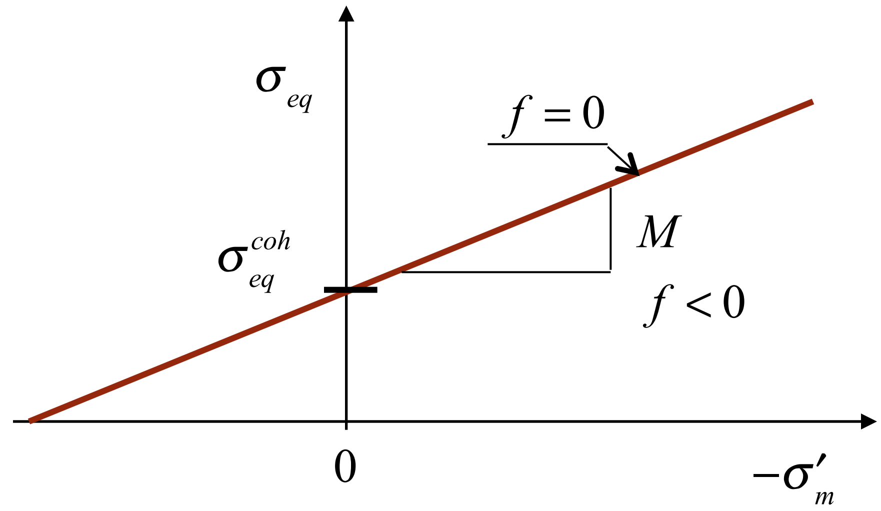
Drucker-Prager failure criterion (with cohesion)

Yield locus

$$f(\boldsymbol{\sigma}') = \sigma_{eq} + M\sigma'_m - \sigma_{eq}^{coh}$$

Evolution rule (not associated)

$$\dot{\boldsymbol{\varepsilon}}^p = \dot{\lambda}\left[\frac{M_\Psi}{3}\mathbf{I} + \frac{3}{2\sigma_{eq}}\boldsymbol{\sigma}^d\right]$$

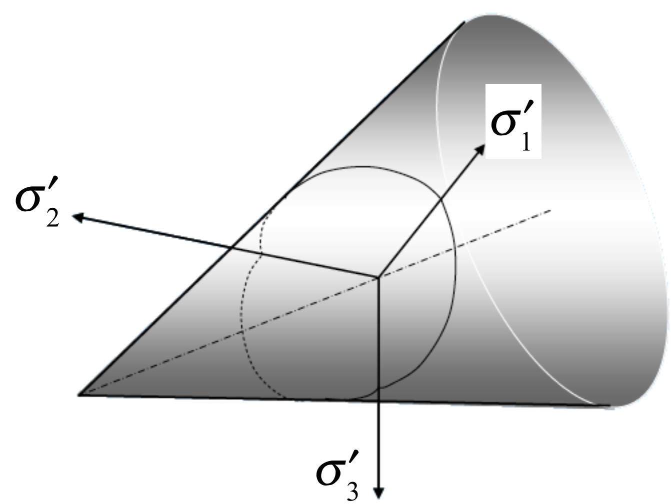


Three material constants

$$M = \left.\frac{\sigma_{eq}}{-\sigma'_m}\right|_{failure}$$

$$M_\Psi = \left.\frac{\dot{\varepsilon}_v^p}{\dot{\varepsilon}_{eq}^p}\right|_{failure}$$

$$\sigma_{eq}^{coh}$$



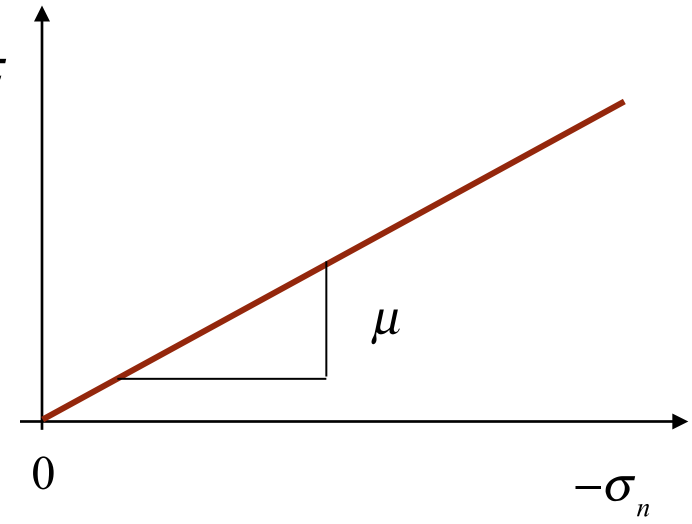
Coulomb friction criterion (1773)

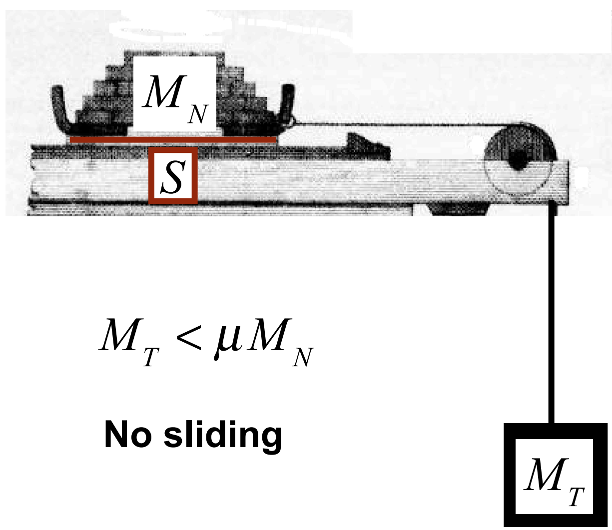
Yield locus

$$\tau + \mu \sigma_n \leq 0$$

μ : friction coefficient

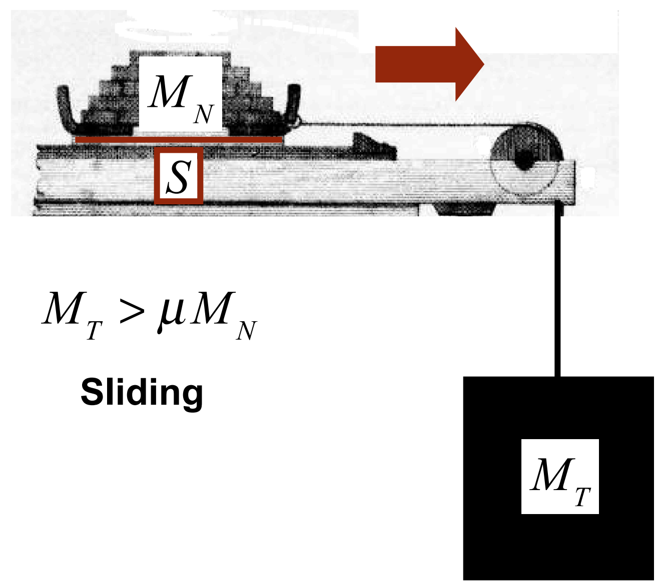
$$\tau = \frac{M_T g}{S} \qquad \sigma_n = \frac{M_N g}{S}$$





$M_T < \mu M_N$

No sliding



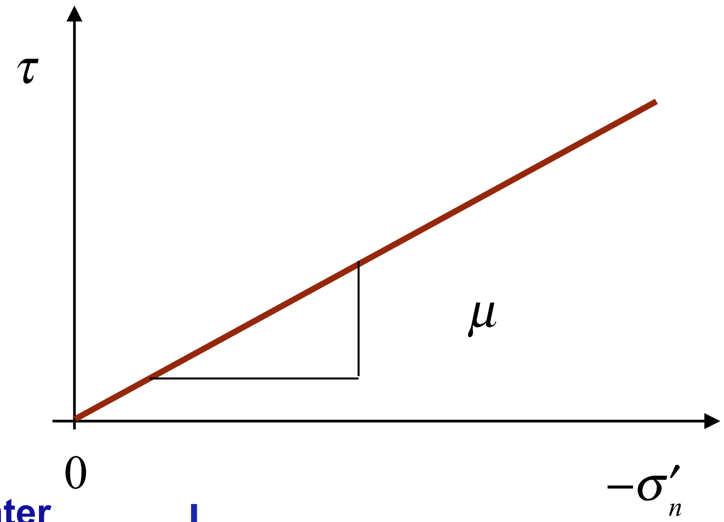
$M_T > \mu M_N$

Sliding

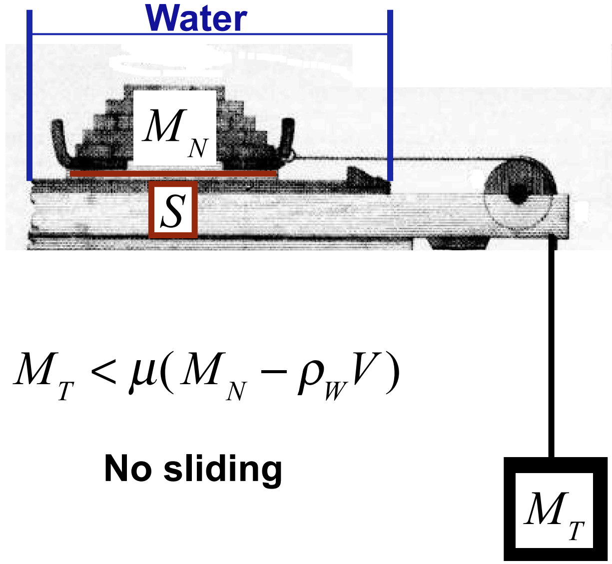
Coulomb friction criterion (1773) accounting for Archimèdes principle (-260)

Yield locus

$$\tau + \mu\sigma'_n \le 0$$

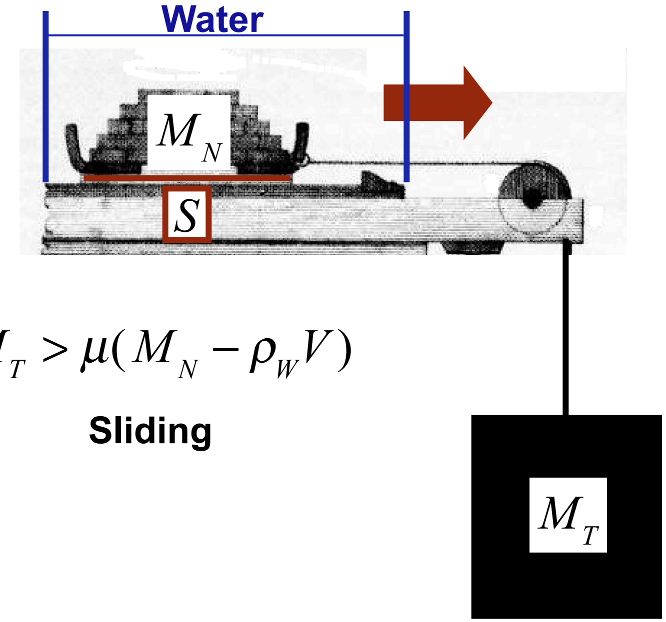


$$\tau = \frac{M_T g}{S} \qquad \sigma'_n = \frac{(M_N - \rho_W V)g}{S}$$



$$M_T < \mu(M_N - \rho_W V)$$

No sliding



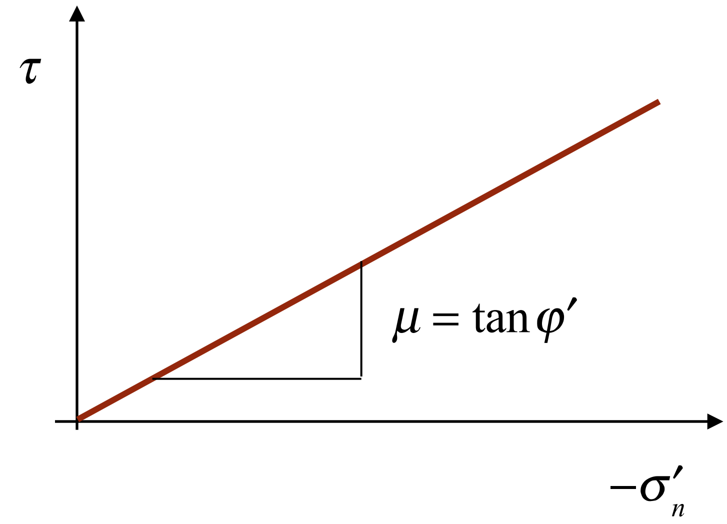
$$M_T > \mu(M_N - \rho_W V)$$

Sliding

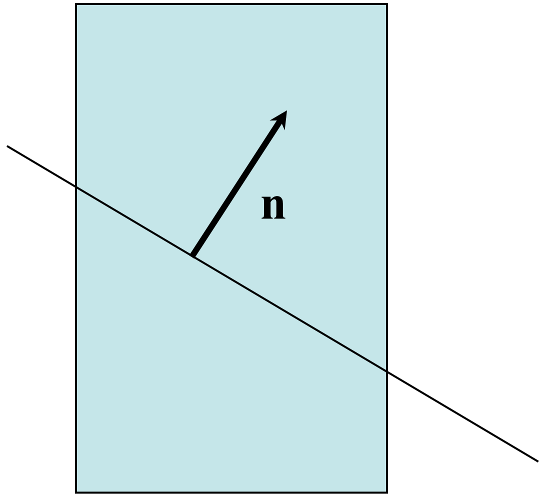
Mohr(1900)-Coulomb (1773) failure criterion

Yield locus

$$\forall \mathbf{n}, \begin{cases} \tau + \sigma' \tan \varphi'_n \leq 0 \\ \tau = \left[\mathbf{I} - \mathbf{n} \otimes \mathbf{n} \right] \cdot \boldsymbol{\sigma}' \\ \sigma' = \left[\mathbf{n} \otimes \mathbf{n} \right] \cdot \boldsymbol{\sigma}' \end{cases}$$



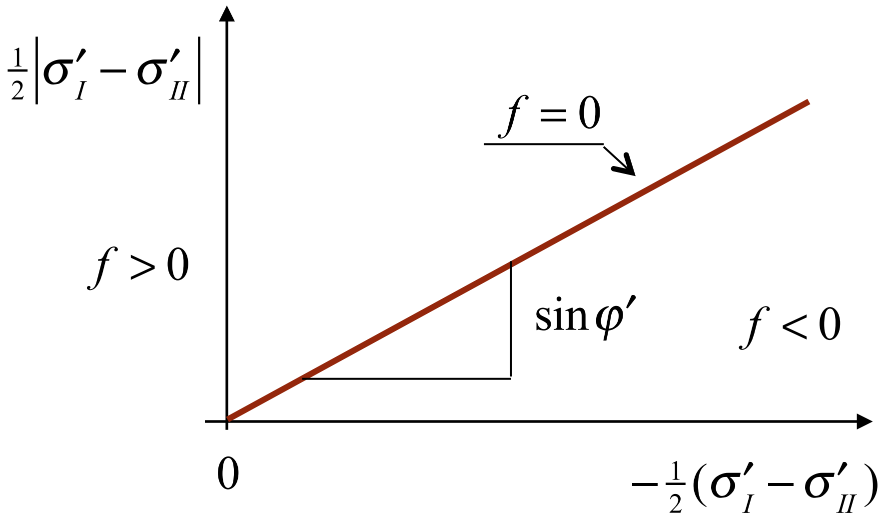
φ' : internal friction angle



Mohr-Coulomb failure criterion (cohesionless)

Yield locus

$$f(\boldsymbol{\sigma}') = \tfrac{1}{2}(\sigma'_I - \sigma'_{II}) + \tfrac{1}{2}(\sigma'_I + \sigma'_{II})\sin\varphi'$$

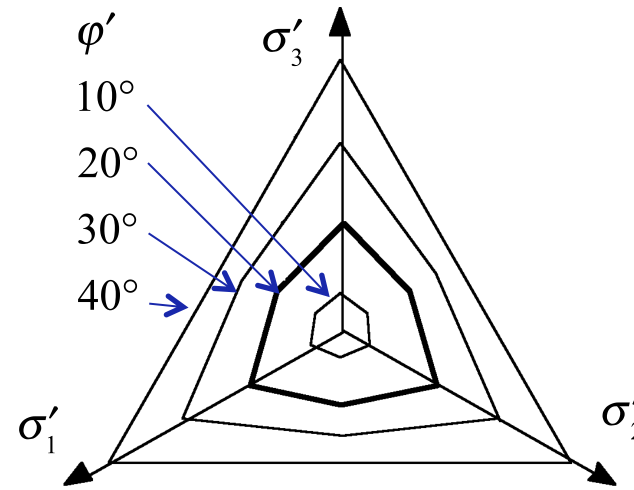


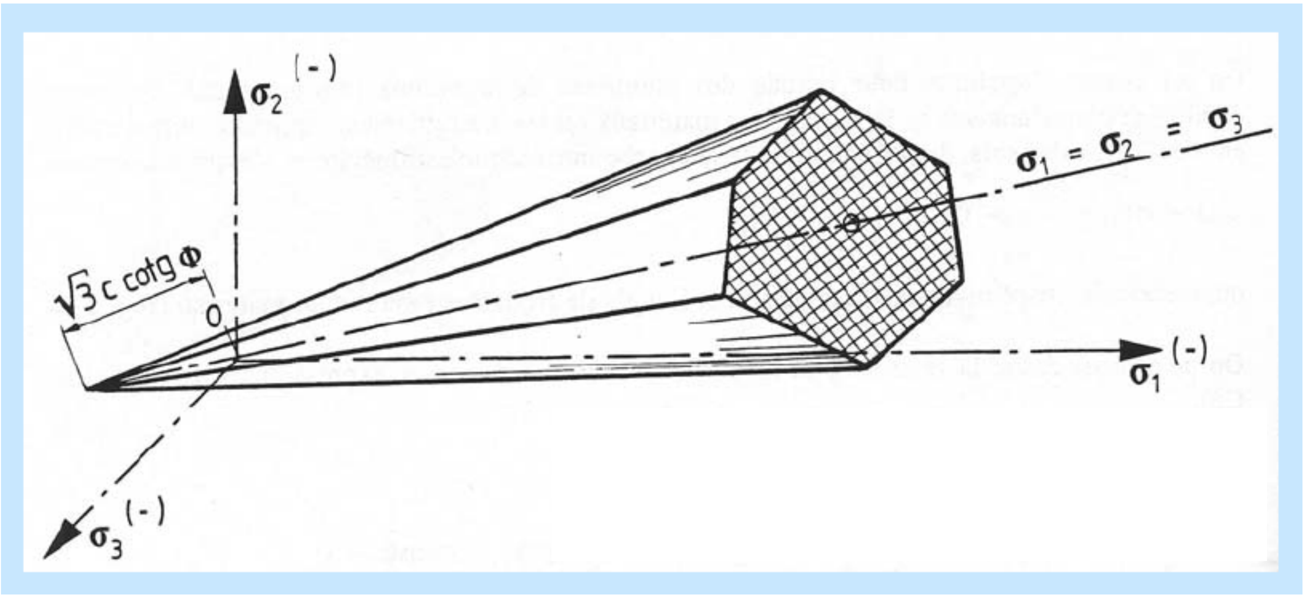
$$f(\boldsymbol{\sigma}') \le 0 \qquad \Leftrightarrow \qquad \frac{\sigma'_I}{\sigma'_{III}} \le \frac{1+\sin\varphi'}{1-\sin\varphi'} = \tan^2\left(\frac{\pi}{4} + \frac{\varphi'}{2}\right)$$

Mohr-Coulomb failure criterion (with cohesion)

Yield locus

$$\begin{cases} f_1(\boldsymbol{\sigma}') = \frac{1}{2}\left|\sigma'_2 - \sigma'_3\right| + \frac{1}{2}(\sigma'_2 + \sigma'_3)\sin\varphi' - c'\cos\varphi' \\ f_2(\boldsymbol{\sigma}') = \frac{1}{2}\left|\sigma'_1 - \sigma'_3\right| + \frac{1}{2}(\sigma'_1 + \sigma'_3)\sin\varphi' - c'\cos\varphi' \\ f_3(\boldsymbol{\sigma}') = \frac{1}{2}\left|\sigma'_1 - \sigma'_2\right| + \frac{1}{2}(\sigma'_1 + \sigma'_2)\sin\varphi' - c'\cos\varphi' \end{cases}$$





Mohr-Coulomb failure criterion

Yield locus

$$f(\boldsymbol{\sigma}') = \sigma_{eq} h(\omega_\sigma) + M\sigma'_m$$

$$M = \frac{6\sin\varphi'}{3-\sin\varphi'} \qquad h(\omega_\sigma) = \frac{2}{3-\sin\varphi'}(\sqrt{3}\cos\omega_\sigma + \sin\varphi' \sin\omega_\sigma)$$

Extension	Pure shear	Compression
$\theta_\sigma = 0$	$\theta_\sigma = \frac{\pi}{6}$	$\theta_\sigma = \frac{\pi}{3}$
$\frac{\sigma_{eq}}{-\sigma'_m} \leq \frac{6\sin\varphi'}{3+\sin\varphi'}$	$\frac{\sigma_{eq}}{-\sigma'_m} \leq \sqrt{3}\sin\varphi'$	$\frac{\sigma_{eq}}{-\sigma'_m} \leq \frac{6\sin\varphi'}{3-\sin\varphi'}$

Mohr-Coulomb failure criterion

Pure shear orthogonal to

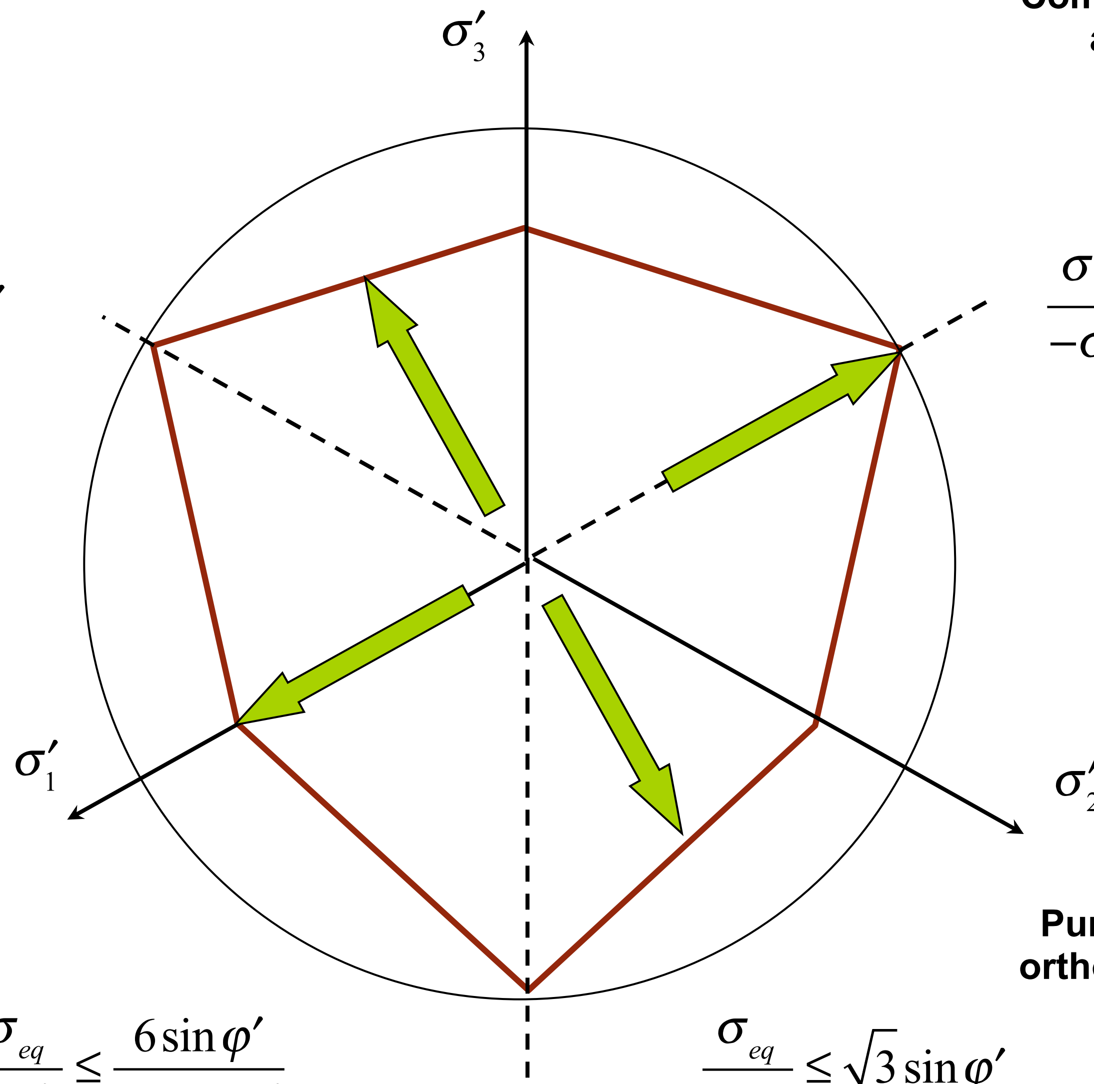
σ'_1

$$\frac{\sigma_{eq}}{-\sigma'_m} \leq \sqrt{3}\sin\varphi'$$

Compression along

σ'_1

$$\frac{\sigma_{eq}}{-\sigma'_m} \leq \frac{6\sin\varphi'}{3-\sin\varphi'}$$



Extension along

σ'_1

$$\frac{\sigma_{eq}}{-\sigma'_m} \leq \frac{6\sin\varphi'}{3+\sin\varphi'}$$

Pure shear orthogonal to

σ'_1

$$\frac{\sigma_{eq}}{-\sigma'_m} \leq \sqrt{3}\sin\varphi'$$

Regularized mohr-Coulomb like failure criteria

Yield locus:

$$f(\boldsymbol{\sigma}') = \sigma_{eq} h(\theta_\sigma) + M\sigma'_m$$

Matsuoka-Nakaï (1974)

$$h(\theta_\sigma) = \left(\frac{1+\gamma\eta\cos 3\theta_\sigma}{1-\gamma} \right)^{1/2} \qquad \eta = \frac{\sigma_{eq}}{-M\sigma'_m} \qquad \gamma = \frac{9-\sin^2\varphi'}{9(3+\sin^2\varphi')}$$

Lade-Duncan (1975)

$$h(\theta_\sigma) = \left(\frac{1+\gamma\eta\cos 3\theta_\sigma}{1-\gamma} \right)^{1/2} \qquad \eta = \frac{\sigma_{eq}}{-M\sigma'_m} \qquad \gamma = \frac{4}{9}$$

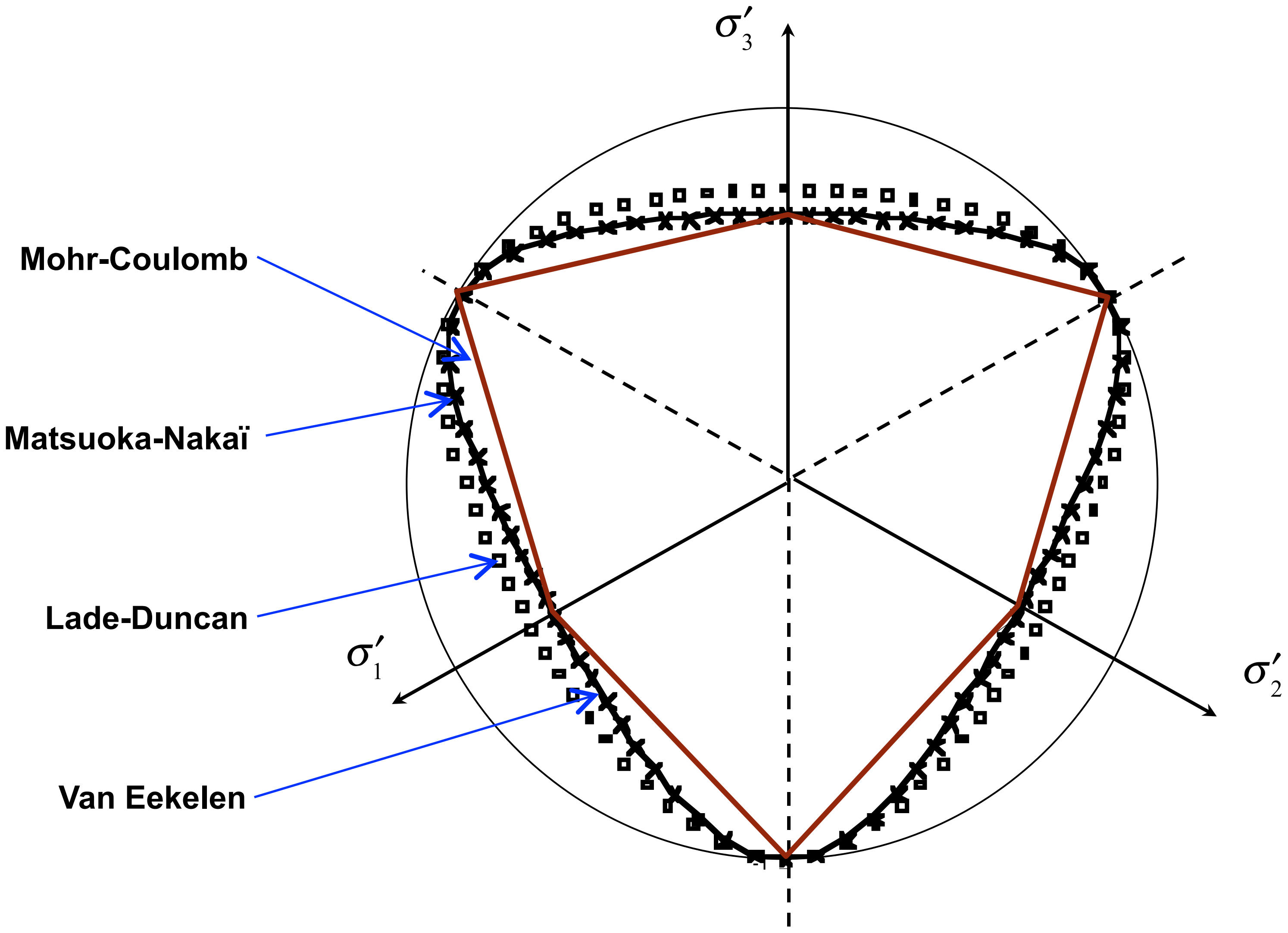
Van Eekelen (1980)

$$h(\theta_\sigma) = \left(\frac{1+\gamma\cos 3\theta_\sigma}{1-\gamma} \right)^{k} \qquad k = 0.229 \qquad \gamma = \frac{1-r}{1+r} \qquad r = \left(\frac{3-\sin\varphi'}{3+\sin\varphi'} \right)^{1/k}$$

Seule valeur possible avec Abaqus: $k = 1$ **(attention à la perte de convexité !!)**

Valeur optimum assurant la convexité: $k = 0.229$

Regularized mohr-Coulomb like failure criteria



Cambridge elastoplastic models: Cam-Clay

The Cam-Clay model for isotropic media in small strains is usually described as follows

Strain decomposition $\boldsymbol{\varepsilon} = \boldsymbol{\varepsilon}^e + \boldsymbol{\varepsilon}^p$

Isotropic non-linear elasticity

$$\chi(\sigma'_m) = \frac{1+e}{\kappa^e}(-\sigma'_m) \qquad G(\sigma'_m) = \chi(\sigma'_m)\frac{3(1-2\nu)}{2(1+\nu)}$$

Plasticity

Yield locus

$$f(\boldsymbol{\sigma}', \sigma_c) = \sigma_{eq} + M\sigma'_m \ln\left(\frac{\sigma_c}{-\sigma'_m}\right)$$

Flow rule

- Associated plastic potential

$$\mathbf{Q}(\boldsymbol{\sigma}', \sigma_c) = \frac{\partial f}{\partial \boldsymbol{\sigma}'}(\boldsymbol{\sigma}', \sigma_c)$$

- Non associated isotropic hardening

$$\dot{\sigma}_c = \sigma_c^0 \exp(-\beta\varepsilon_v^p) \qquad \varepsilon_v^p = \mathrm{tr}\,\boldsymbol{\varepsilon}^p$$

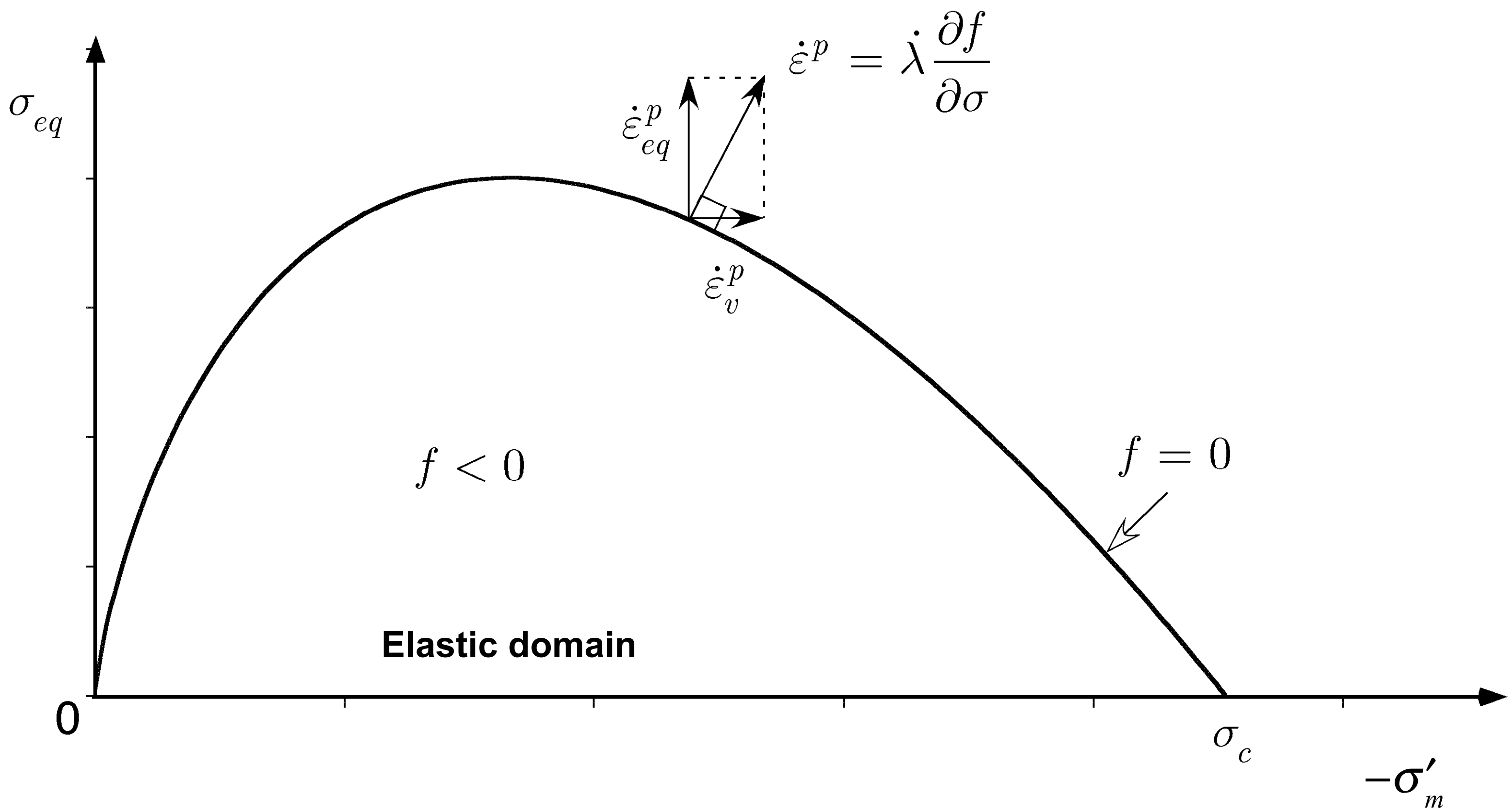
Material constants

$$\kappa^e,\ \nu,\ M,\ \beta$$

Initial state

$$\sigma_c^0,\ e^0$$

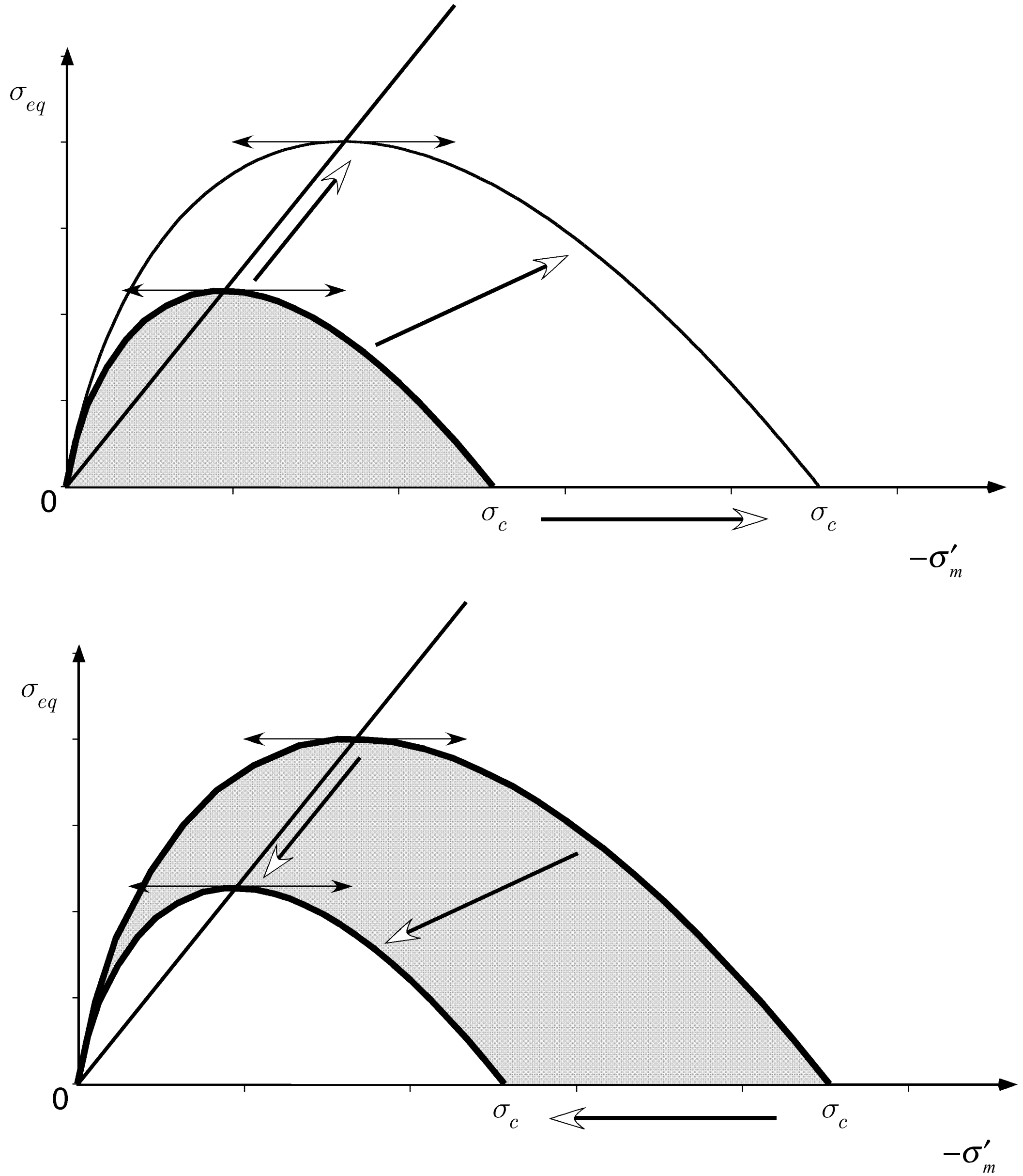
Cambridge elastoplastic models: Cam-Clay



The isotropic hardening variable σ_c is the consolidation stress.

The material has some memory of the greatest consolidation stress undergone in the course of its history.

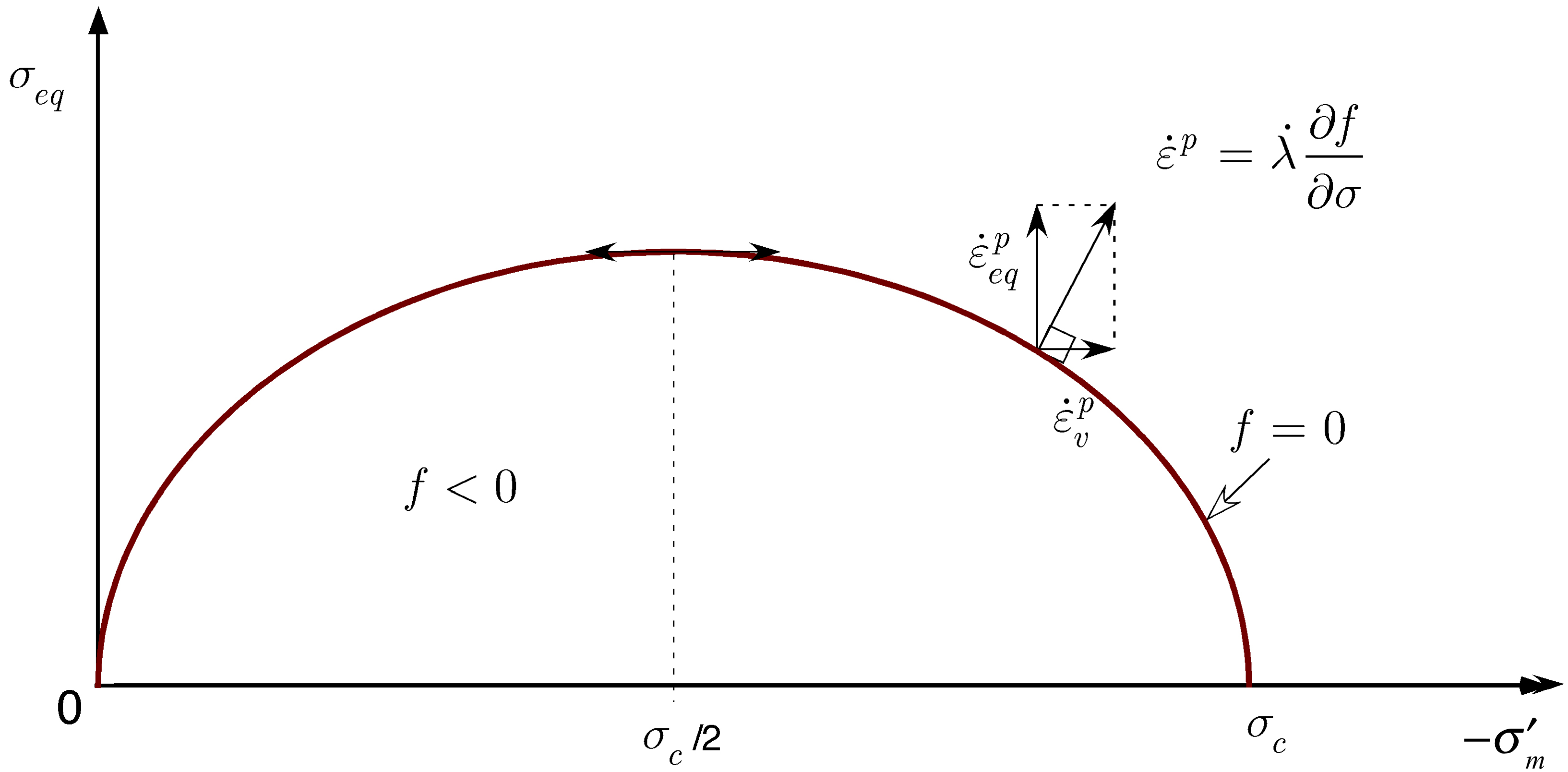
Cambridge elastoplastic models: Cam-Clay



Cambridge elastoplastic models: modified Cam-Clay

The modified Cam-Clay model for isotropic media in small strains is the same as the Cam-Clay model, with the following yield locus

$$f(\boldsymbol{\sigma}',\sigma_c) = \sigma_{eq}^2 + M\sigma'_m(\sigma'_m + \sigma_c)$$

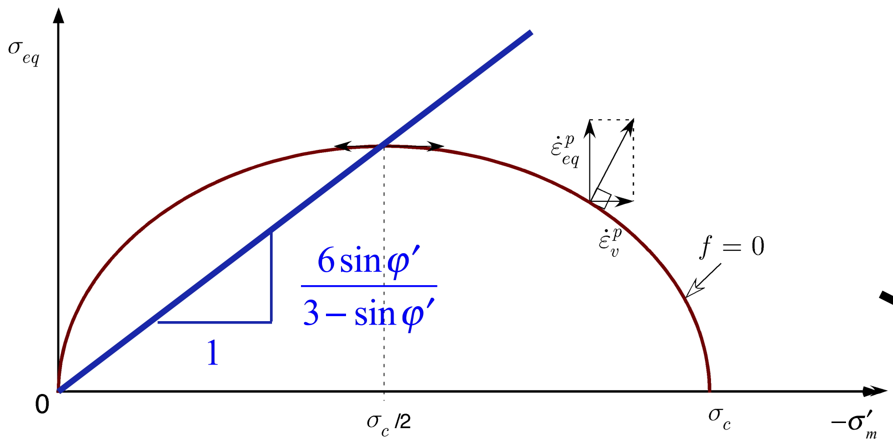


Equivalent expression: $f(\boldsymbol{\sigma}',\sigma_c) = \sigma_{eq} + M\sigma'_m\sqrt{\dfrac{\sigma_c}{-\sigma'_m} - 1}$

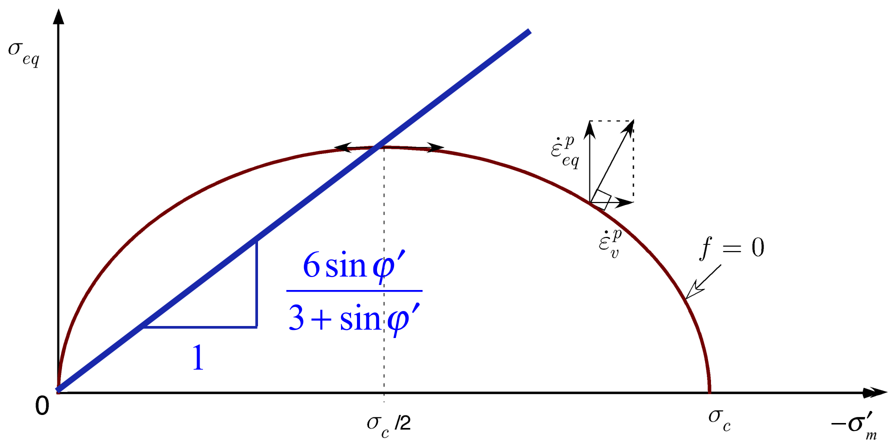
Modified Cam-Clay accouting for the third stress invariant

Yield locus

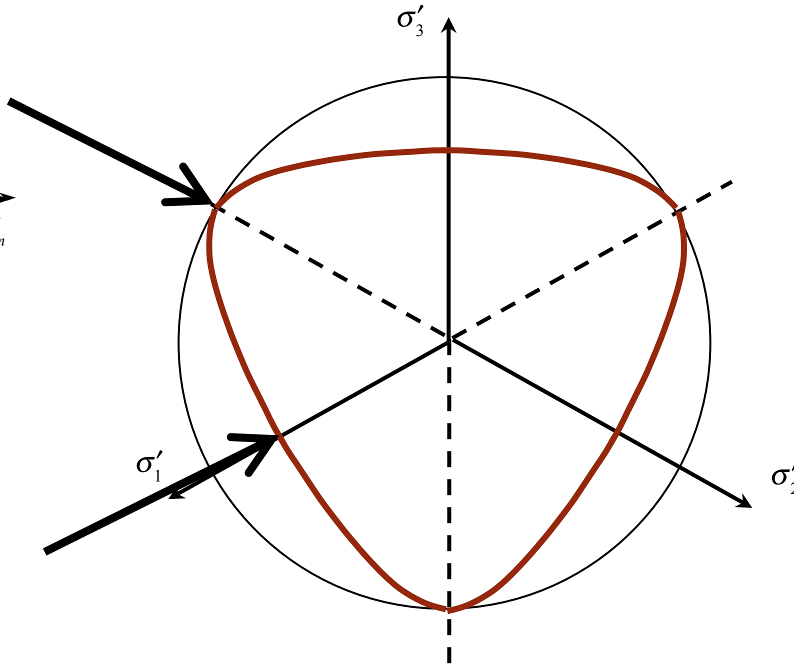
$$f(\boldsymbol{\sigma}',\sigma_c)=\left[\sigma_{eq}h(\theta_\sigma)\right]^2+M\sigma'_m(\sigma'_m+\sigma_c)$$



Compression



Extension



Questions

Modèle de Cam-Clay

1. **Le point d'intersection de la surface de charge du modèle de Cam-Clay et de la droite définie par**

$$\sigma_{eq} + M\sigma'_m = 0$$

est un point particulier. Le positionner sur le graphique.

2. **Expliciter la vitesse de déformation plastique en fonction de** $\sigma_{eq}, \sigma'_m \text{ et } \dot{\lambda}$

(σ_c **ne doit pas apparaître.)**

3. **Expliciter la condition cinématique du modèle de Cam-Clay, en fonction de** $\sigma_{eq} \text{ et } \sigma'_m$

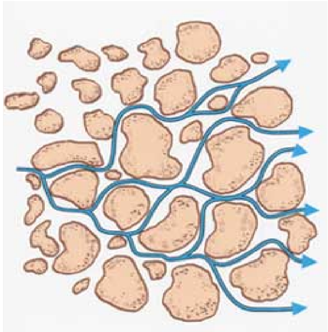
Cette condition relie $\dot{\varepsilon}_v^p$ **et** $\dot{\varepsilon}_{eq}^p$

4. **Quelle inéquation doit vérifier** (σ'_m, σ_{eq}) **pour que l'on ait une évolution avec dilatance plastique ?**

5. **Quelle inéquation doit vérifier** (σ'_m, σ_{eq}) **pour que l'on ait une évolution avec contractance plastique ?**

6. **Comment évolue la variable d'écrouissage isotrope en dilatance plastique ? Et en contractance plastique ?**

Milieux poreux
Poro-Mécanique
Couplages hydro-mécanique



Stresses in soils
Cases study

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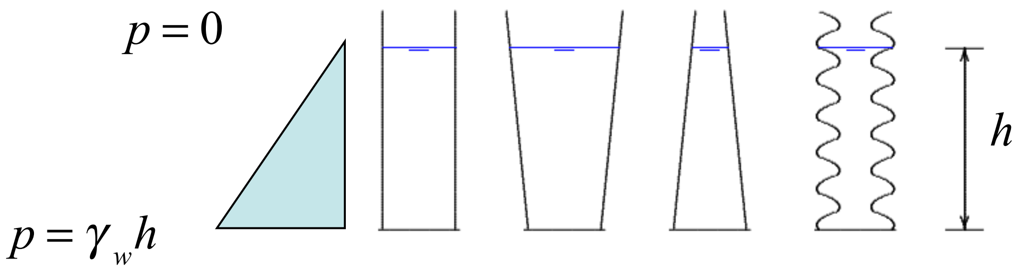
Pore pressures and hydraulic head

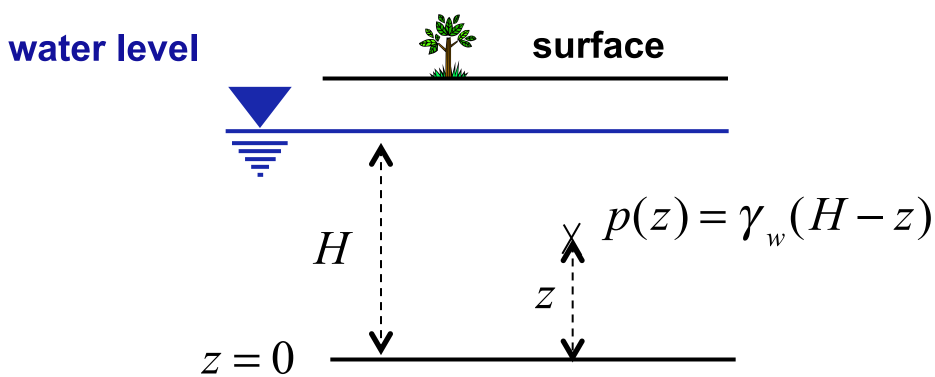


$\gamma_w \approx 10\ \mathrm{kN/m^3}$

Hydrostatic water pressure depends upon depth only $\left(p > 0 \text{ in the water}\right)$

The hydraulic head is defined as follows

$$H = \frac{p - p_{atm}}{\gamma_w} + z \qquad \gamma_w = \rho_w g$$



Stéphane Bonelli

Effective stress

The total stress is defined by the static equilibrium equation

$$\nabla \cdot \boldsymbol{\sigma} = 0 \qquad \left(\operatorname{tr} \boldsymbol{\sigma} < 0 \;\Leftrightarrow\; \text{compression}\right)$$

The effective stress is defined as follows (Terzaghi, 1925)

$$\boldsymbol{\sigma}' = \boldsymbol{\sigma} + p\mathbf{I}$$

The effective stress differs than the total stress only on the isotropic part

$$\tfrac{1}{3}\operatorname{tr} \boldsymbol{\sigma}' = \tfrac{1}{3}\operatorname{tr} \boldsymbol{\sigma} + p$$

$$\boldsymbol{\sigma}'^{d} = \boldsymbol{\sigma}^{d}$$

The behaviour law of the solid matrix involves the effective stress, for example, isotropic linear elasticity in small strains

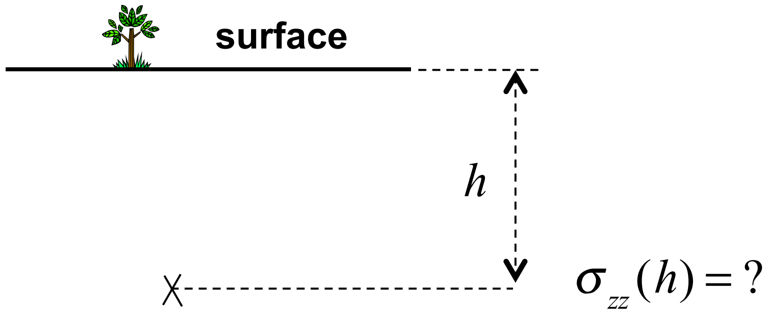
$$\boldsymbol{\sigma}' = 2G\boldsymbol{\varepsilon} + \operatorname{tr} \boldsymbol{\varepsilon}\left(\chi - \frac{2G}{3}\right)\mathbf{I}$$

$$G = \frac{E}{2(1+\nu)} \qquad \chi = \frac{E}{3(1-2\nu)}$$

(shear modulus) (bulk modulus)

Question (1/4): total vertical stress in a dry soi

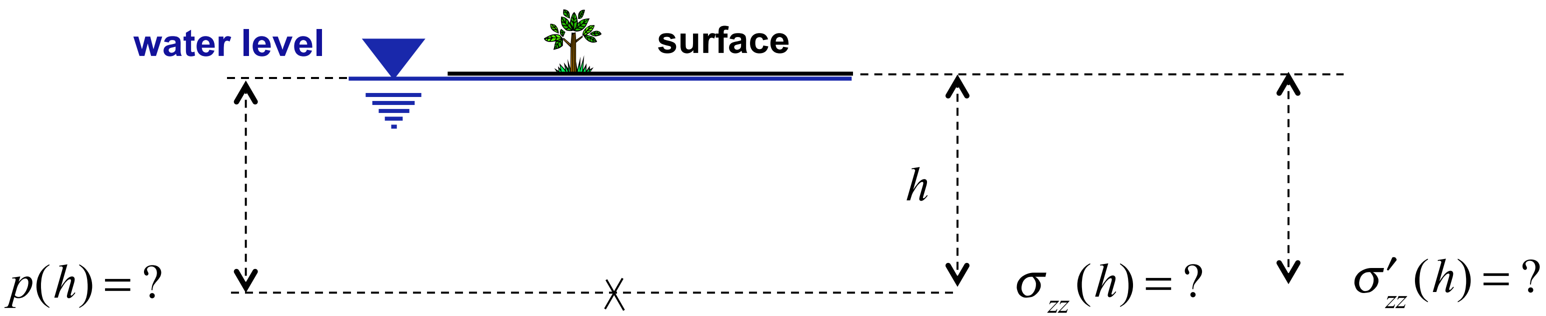
1. Assume a dry and homogeneous semi-infinite soil.
Express the total vertical stress.



Assume a porosity n=0.3, what is the total vertical stress for a 10 m depth (in kPa) ?

Question (2/4): total vertical stress in a water-saturated soil

2. Assume a water-saturated and homogeneous semi-infinite soil.
Express the total vertical stres and the pore-pressure.
Infer the vertical effective stress.



Assume a porosity n=0.3, for a 10 m depth what are (in kPa):

- the total vertical stress?

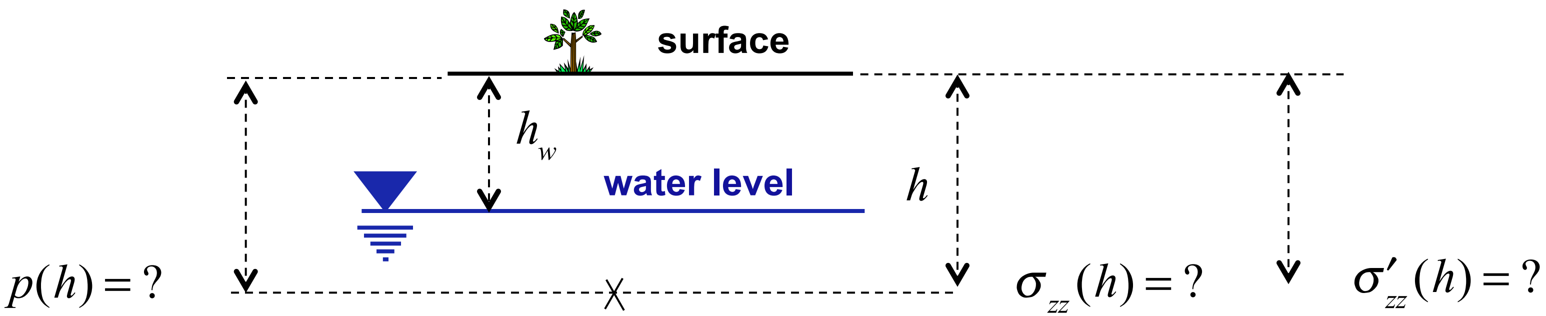
- the pore pressure ?

- the effective stress ?

Stéphane Bonelli

Question (3/4): total vertical stress in a soil

3. Assume a semi-infinite soil with a homogeneous solid matrix.
Assume a hydrostatic water level.
Express the total vertical stres and the pore-pressure.
Infer the vertical effective stress.



Assume a porosity n=0.3,and a water level depth h_w =5 m. For a 10 m depth what are (in kPa):

- the total vertical stress?

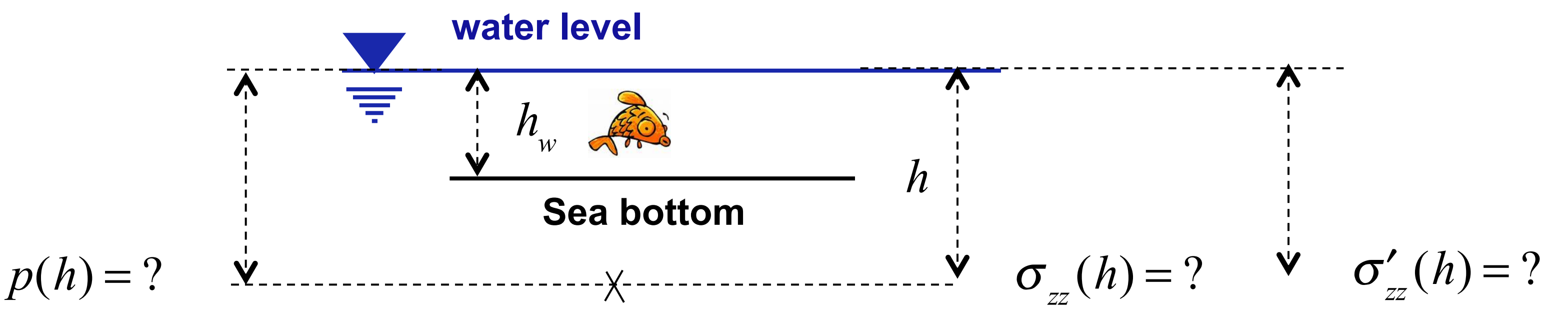
- the pore pressure ?

- the effective stress ?

Stéphane Bonelli

Question (4/4): stresses again

4. Assume a semi-infinite soil with a homogeneous solid matrix.
Assume a hydrostatic water level.
Express the total vertical stres and the pore-pressure.
Infer the vertical effective stress.



Assume a porosity n=0.3,and a water level depth h_w =5 m. For a 10 m depth what are (in kPa):

- the total vertical stress?

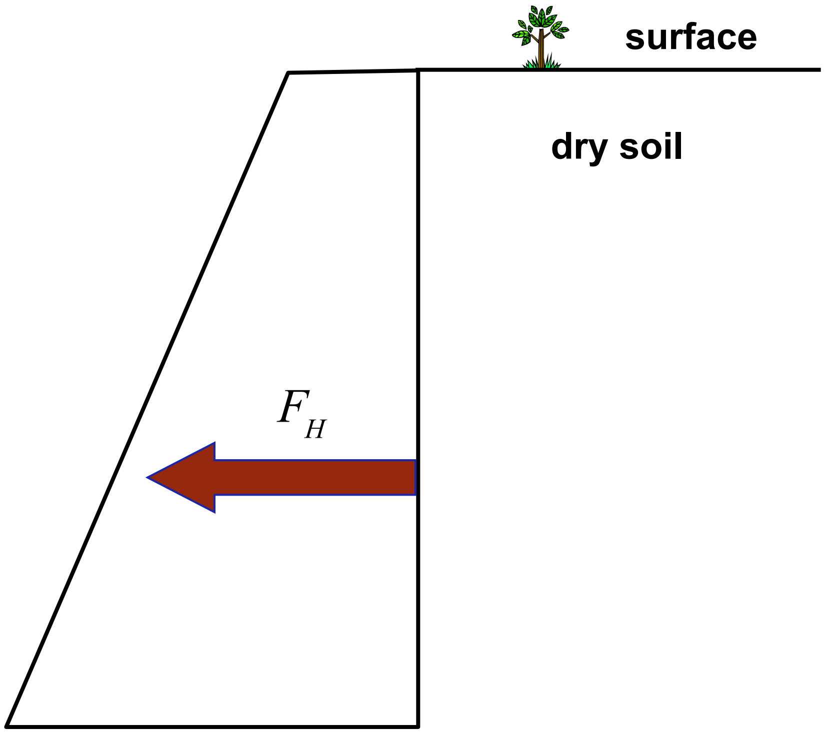
- the pore pressure ?

- the effective stress ?

Question (1/4): the retaining wall

1. Assume a rigid and impervious retaining wall below a semi-infinite dry soil.
   Assume that the soil behaves elastically, with an homogeneous isotropic and linear elastic behaviour law relating the effectives stresses σ' to the strain.
   Assume a perfect wall/soil contact with no interface displacement.
   Express the mean horizontal force exerted by the soil on the wall.

$$F_H = \int_0^H \sigma_{xx}(h)dh$$



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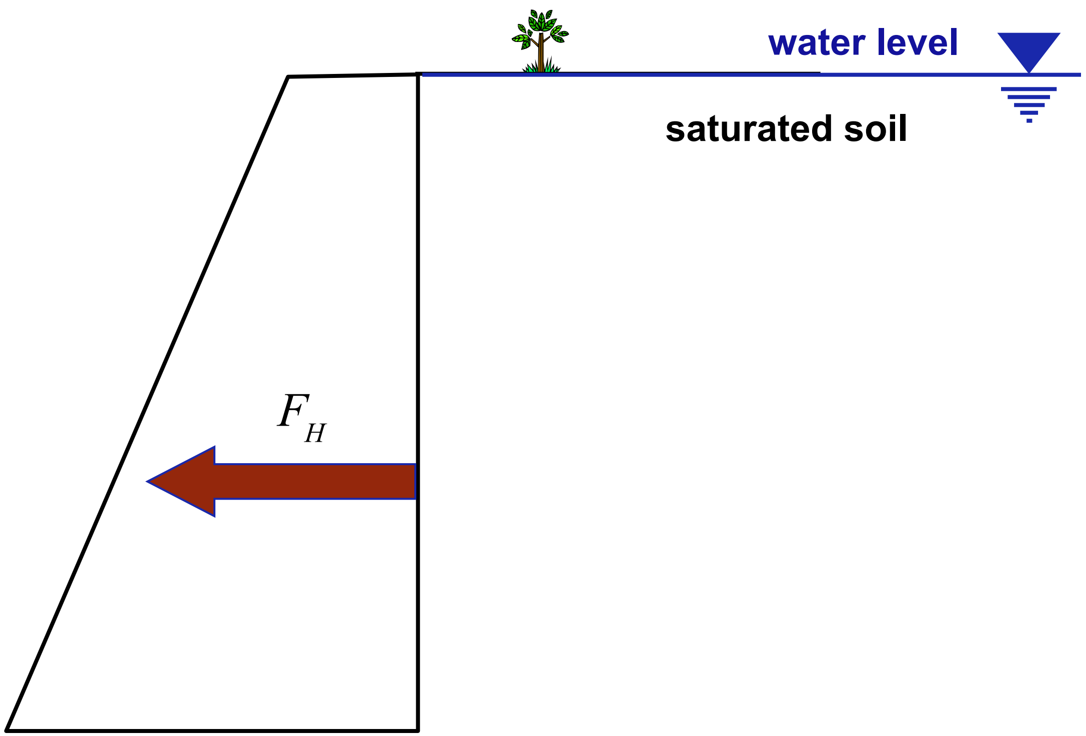
Question (2/4): the retaining wall

2. Now the soil is water-saturated.
Express the mean horizontal force exerted by this soil on the wall.



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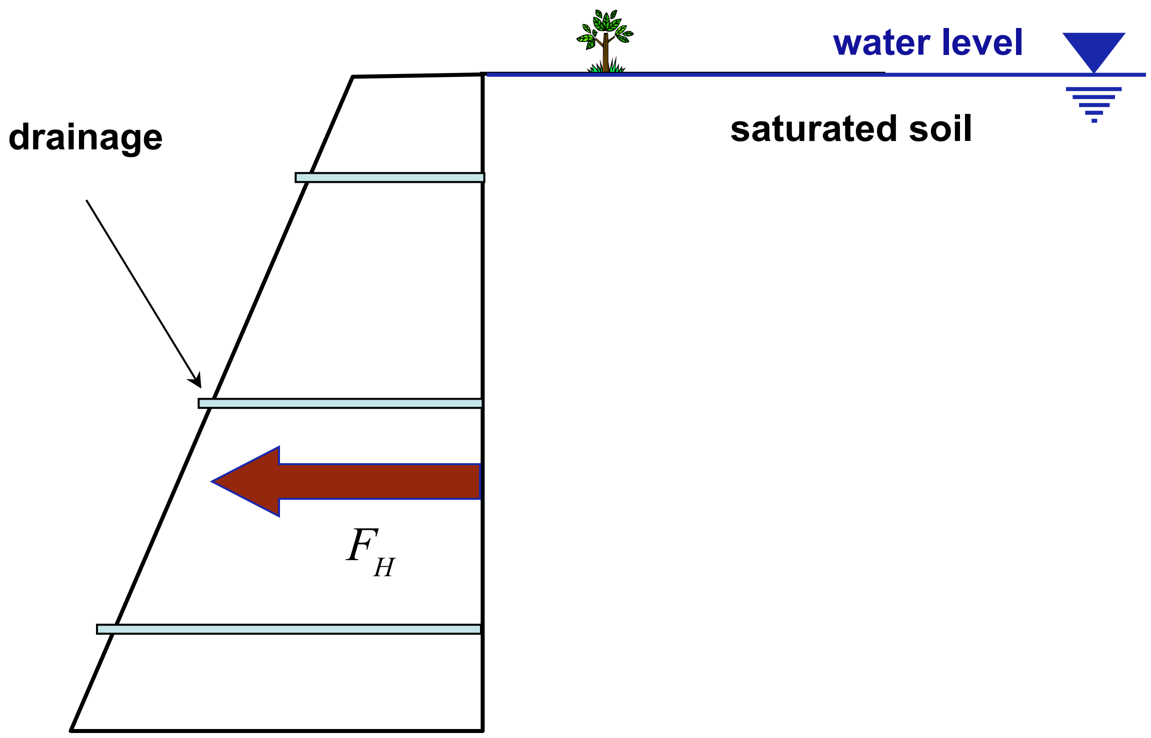
Question (3/4): the retaining wall

3. Now the soil is water-saturated but the wall is drained.
   Express the mean horizontal force exerted by this soil on the wall.
   Conclusion ?



Question (4/4): the retaining wall

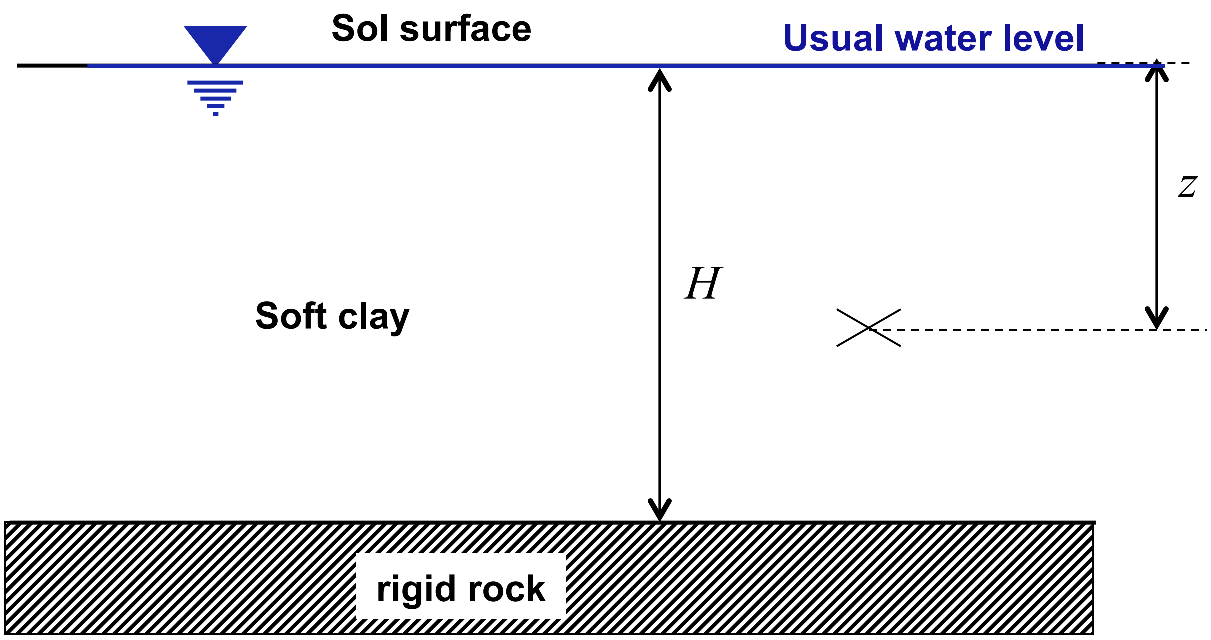
Question (1/3): long subsidence after a construction

1. We consider a soil constitued by a layer of soft clay, lying on a rigid rock.
   The clay is assumed saturated, and have a constant density.

   Express the total vertical stress $\sigma_{zz}^{0}(z)$, the pore pressure $p^{0}(z)$

   and the effective vertical stress $\sigma_{zz}'^{0}(z)$ as a function of depth z

   corresponding to this situation.



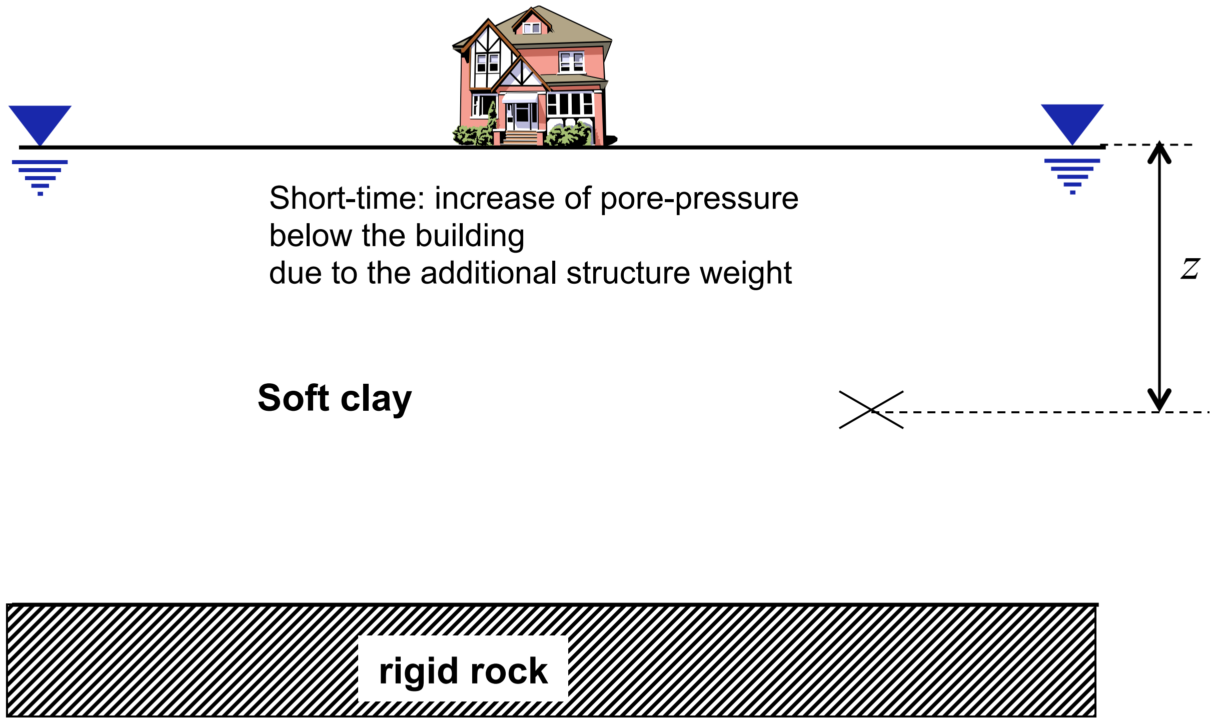
Question (2/3): long subsidence after a construction

2. A building is constructed very quickly. The pore-water is assumed to compensate integraly this excess loading. The clay is assumed saturated, and have a constant density.

   Express the total vertical stress $\sigma^1_{zz}(z)$, the pore pressure $p^1(z)$

   and the effective vertical stress $\sigma'^1_{zz}(z)$ as a function of depth z

   corresponding to this situation, which is the « short-term » situation, just after the construction.



Stéphane Bonelli

Question (3/3): long subsidence after a construction

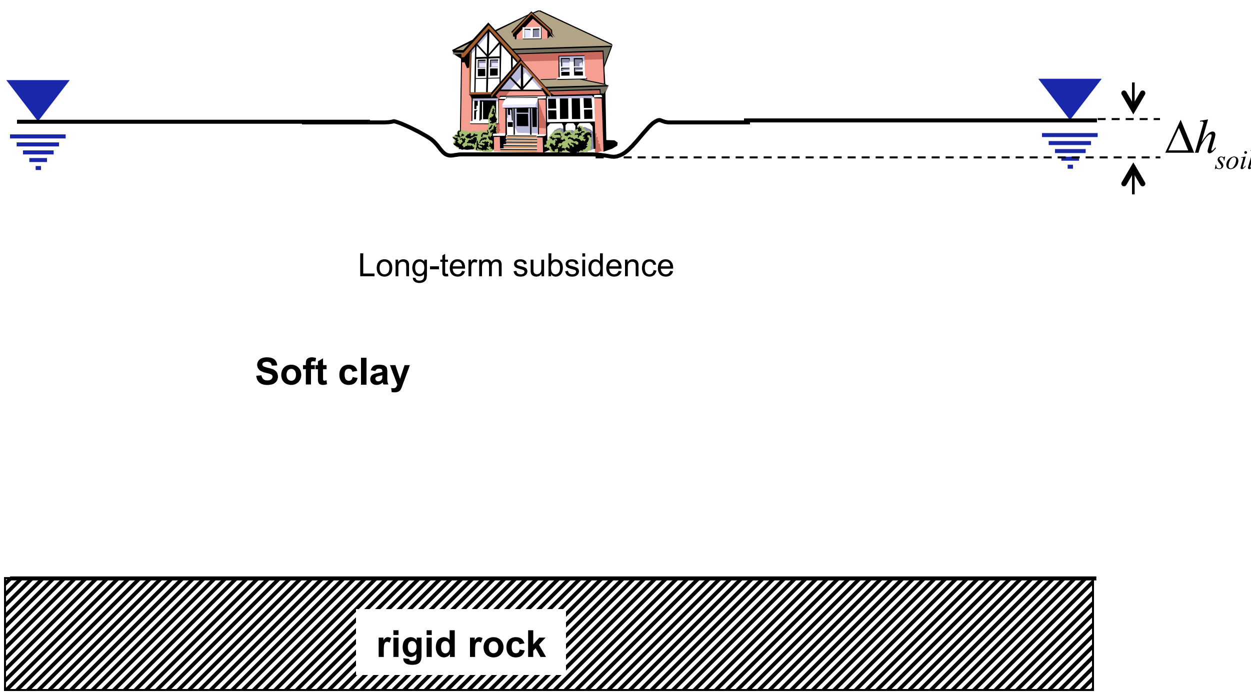
3. **Now we skip to many years afetr the construction. The pore-pressures have dissipated. The clay is assumed saturated, and have a constant density. In addition, this clay behaves elastically as follows**

$$\sigma'_{zz} = E_{\text{œd}}\varepsilon_{zz} \qquad E_{\text{œd}} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)}$$

where ε_{zz} **is the vertical strain, and** $E_{\text{œd}}$ **is the oeodometric modulus.**

Give an estimation of the subsidence Δh_{soil} **, considering that**

$$E = 1\ 000 \text{ kPa},\ \nu = \tfrac{1}{3},\ H = 12 \text{ m}$$



Stéphane Bonelli

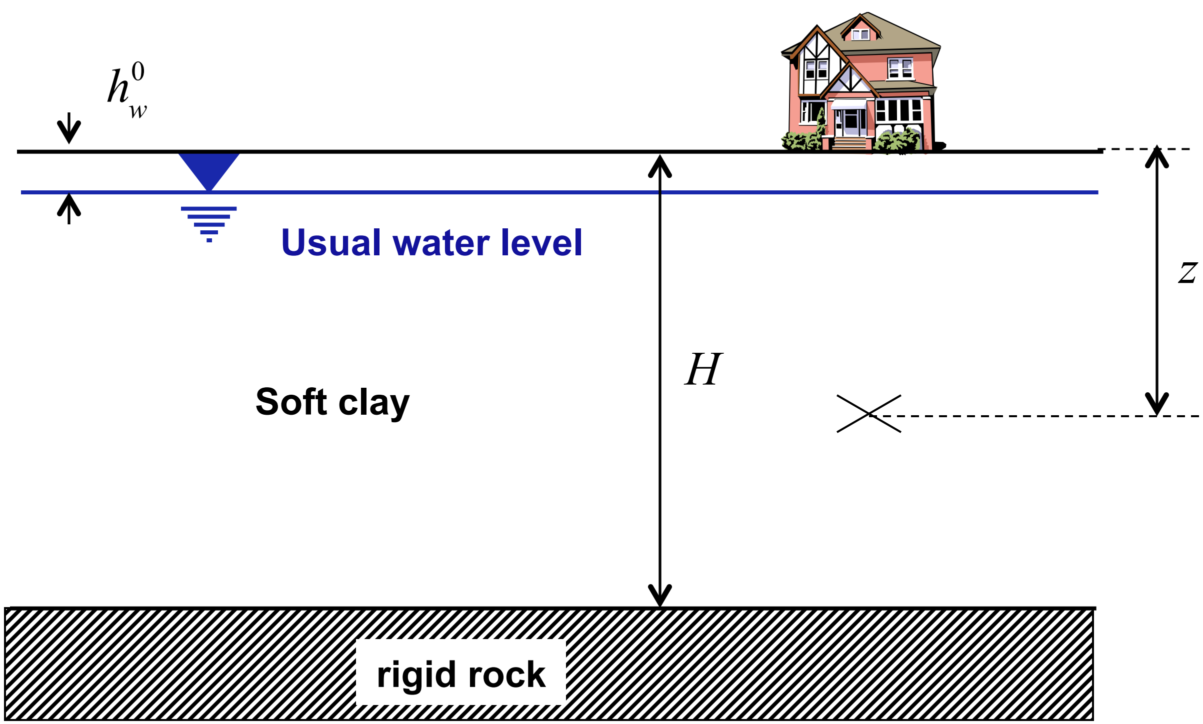
Question (1/4): subsidence by pumping

1. A building lies on a soil.
   This soil is constitued by a layer of soft clay, lying on a rigid rock.
   The clay is assumed saturated, and have a constant density.

   Express the total vertical stress $\sigma_{zz}^{0}(z)$, the pore pressure $p^{0}(z)$

   and the effective vertical stress $\sigma_{zz}'^{0}(z)$ as a function of depth z

   corresponding to this situation.



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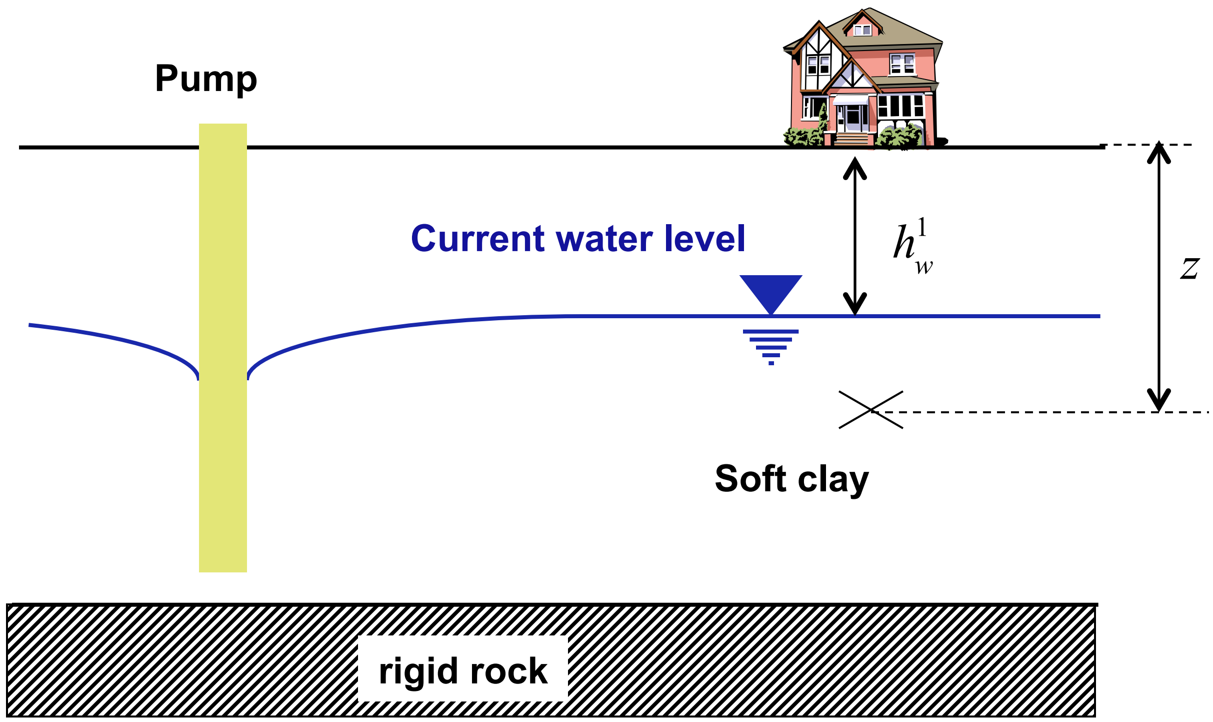
Question (2/4): subsidence by pumping

2. Somebody install a huge pump not too far from the building.
   This water level falls down.
   The clay is assumed saturated, and have a constant density.

   Express the total vertical stress $\sigma_{zz}^{1}(z)$, the pore pressure $p^{1}(z)$ and the effective vertical stress $\sigma_{zz}'^{1}(z)$ as a function of depth z corresponding to this situation.



Question (3/4): subsidence by pumping

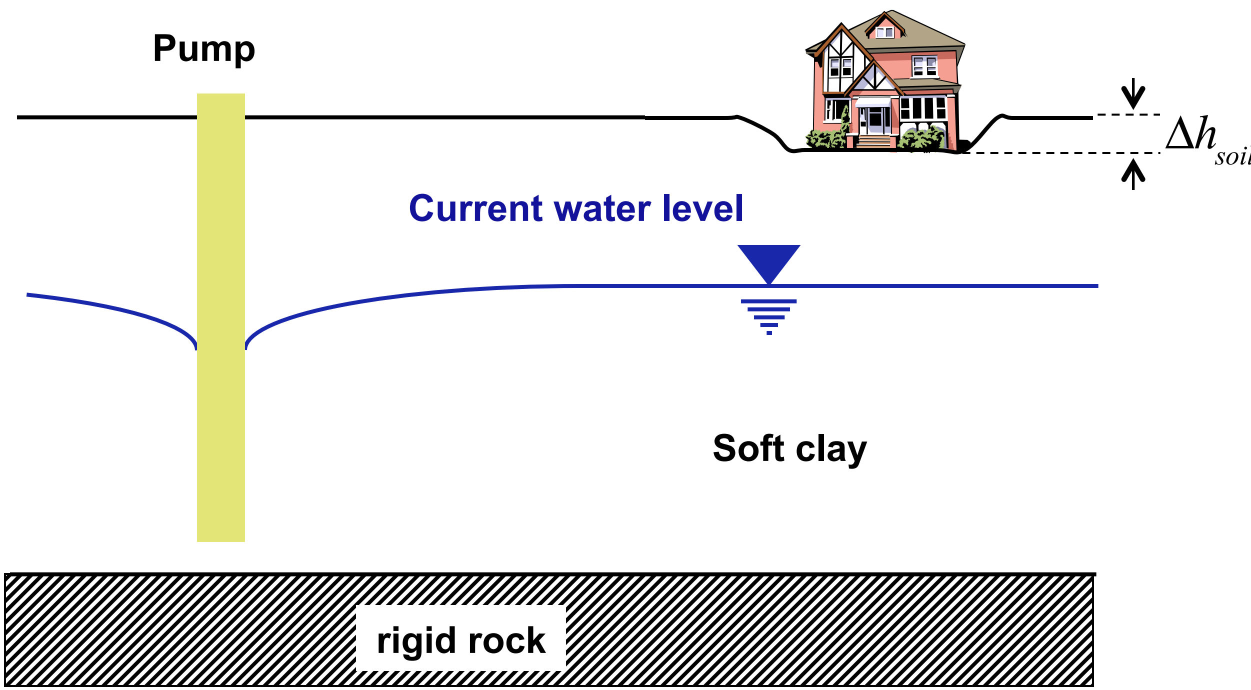
3. Somebody install a huge pump not too far from the building.The water level falls down. The clay is assumed saturated, and have a constant density. In addition, this clay behaves elastically as follows

$$\sigma'_{zz} = E_{œd}\varepsilon_{zz} \qquad E_{œd} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)}$$

where ε_{zz} is the vertical strain, and $E_{œd}$ is the oeodometric modulus.

Give an estimation of the subsidence Δh_{soil} , considering that

$$E = 1\ 000 \text{ kPa},\ \nu = \tfrac{1}{3},\ h_w^0 = 2 \text{ m},\ h_w^1 = 4 \text{ m},\ H = 12 \text{ m}$$



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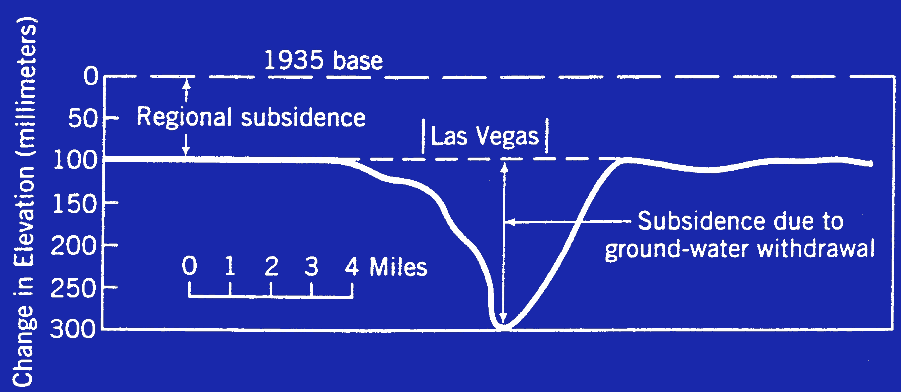
Question (4/4): subsidence by pumping



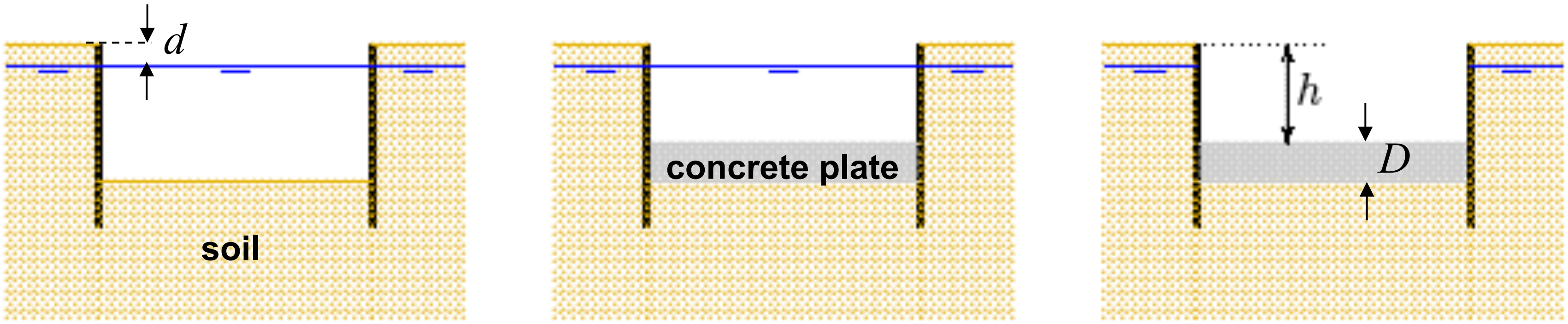
Subsidence profile, Las Vegas Valley, showing differential subsidence due to pumping superposed on regional subsidence (from Malmberg, 1960).

Stéphane Bonelli

Question(1/3): floatation

1. A concrete floor under water.
Examples: foundations of basements, or pavements of the access road of a tunnel.

One of the function of the concrete plate is to give additional weight to the soil, in order to prevent the soil to float.



Excavation of the pit under water, with dredging equipment

Construction of the concrete floor under water

Lowering of water

What is the minimum thickness *D* of the concrete layer ensuring that it will not float itself (and therefore it will be able to provide additional weight to the soil) ?

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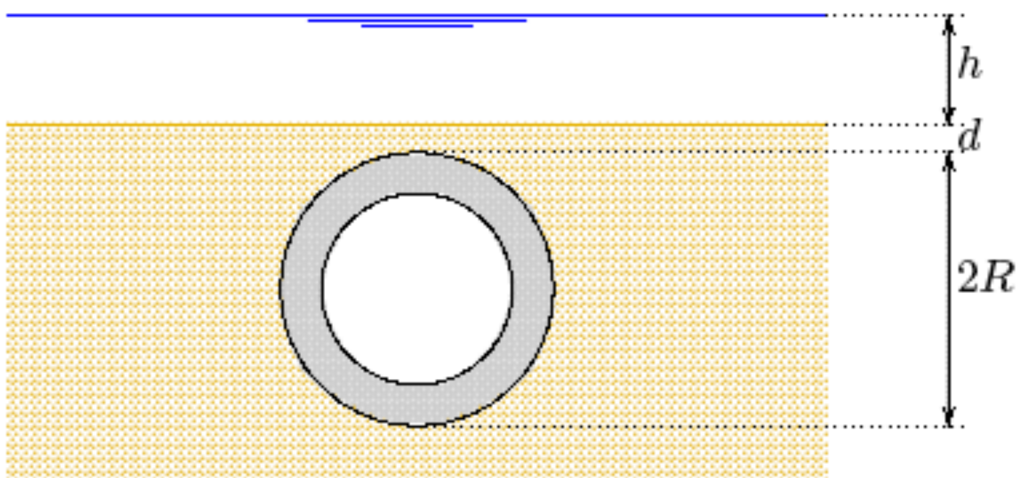
Question(2/3): floatation

2. A pipe or a tunnel under water.

For the risk of floatation, the most dangerous situation will be when the structure is empty.



What is the minimum thickness *d* of the soil above the structure ensuring that the structure it will not float ?

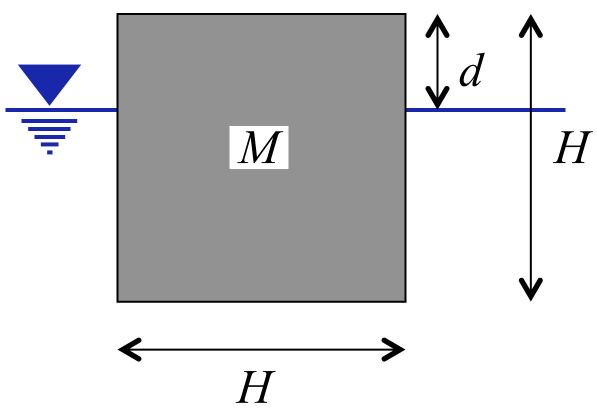
Question(3/3): floatation

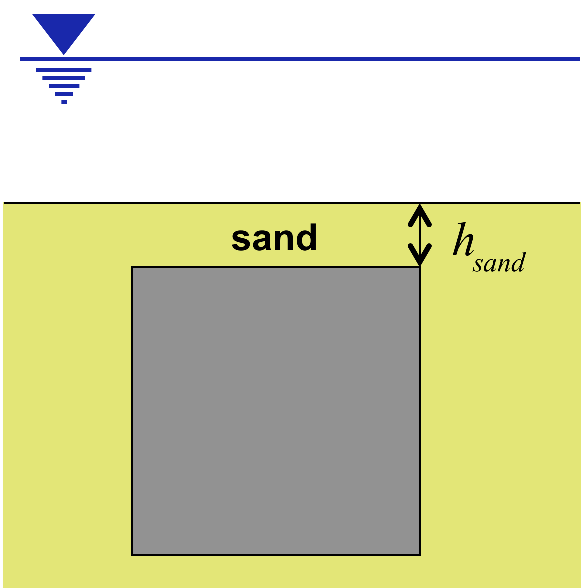
3. Miscealinous.

A tunnel os square cross section H^2 has a weight (above water) *M* per meter length.
The tunnel is beeing floated to its destination.
Calculate the draught *d*.

The tunnel is now sunk into a trench that has been dredged in the sand at the bottom of the river, and then covered with sand of volumic weight g_{sand}.
Determine the minimum cover of sand h_{sand} necessary to prevent floatation of the tunnel.

Numerical values: $H = 8\text{ m}, M = 50\text{ t/mL}, \gamma_{sand} = 20\text{ kN/m}^3$





Stéphane Bonelli

Question(1/3): nappe côtière phréatique

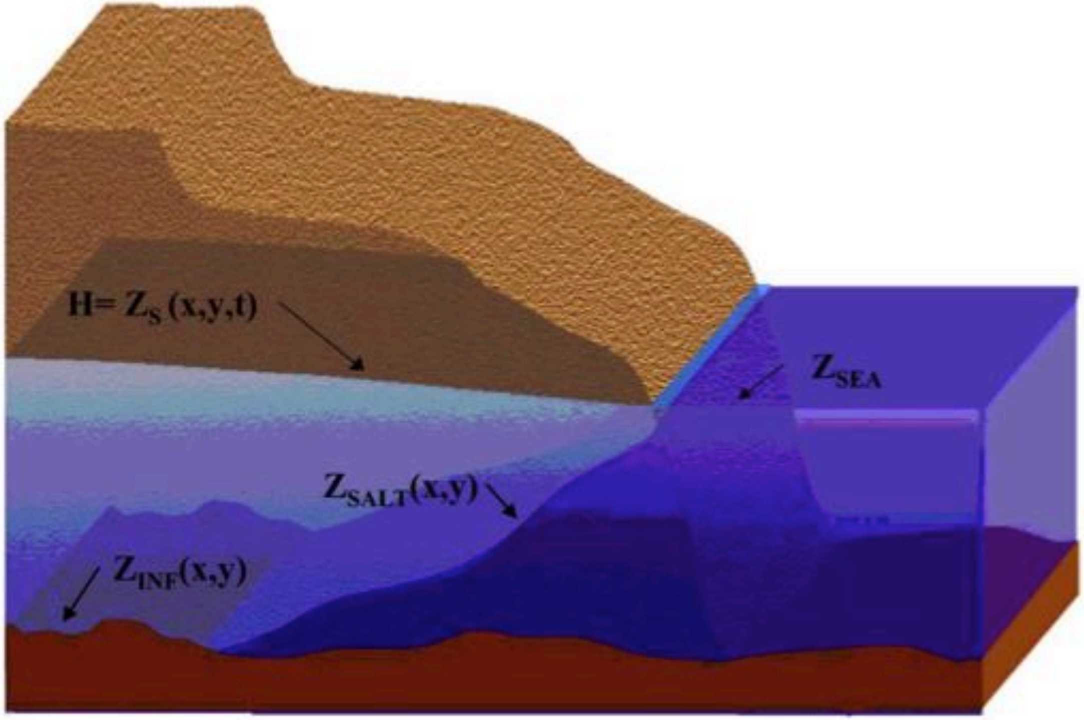


Schéma de l'intrusion d'un coin salé à l'équilibre dans une nappe côtière phréatique, sans recharge ni pompages (vue en perspective, milieux hétérogène sans symétrie plane).

Question(2/3): nappe côtière phréatique

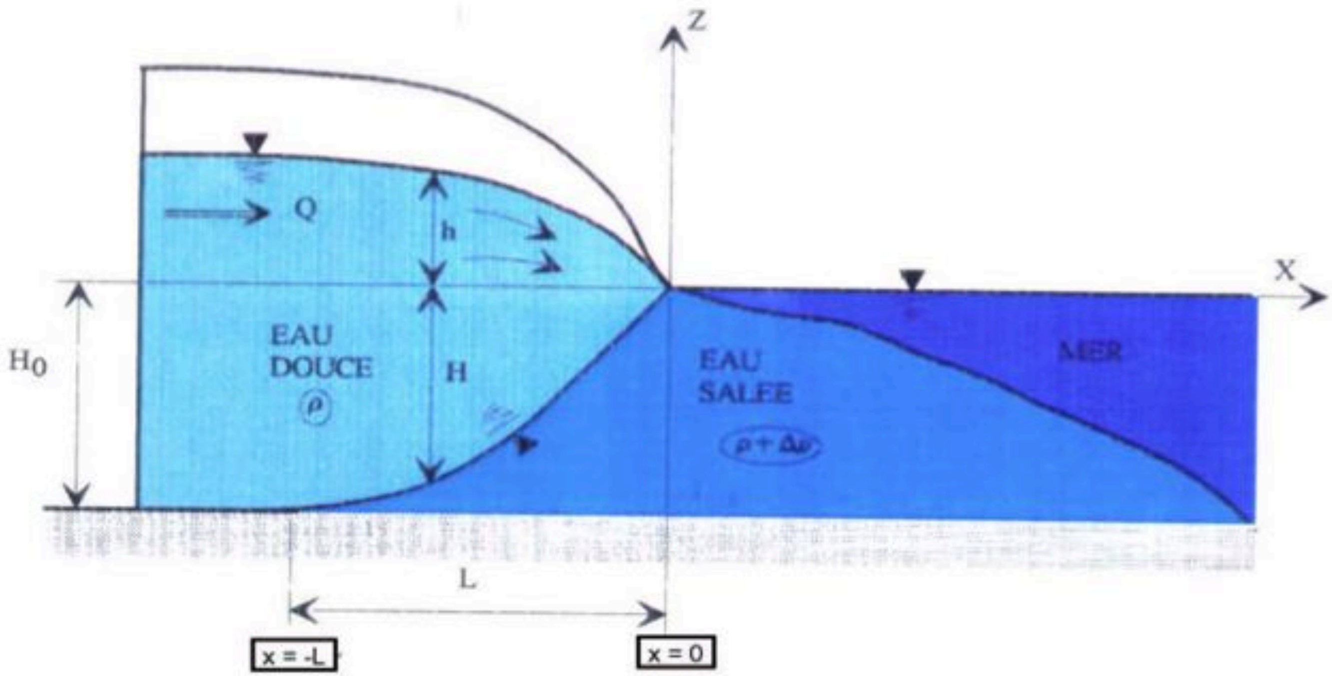
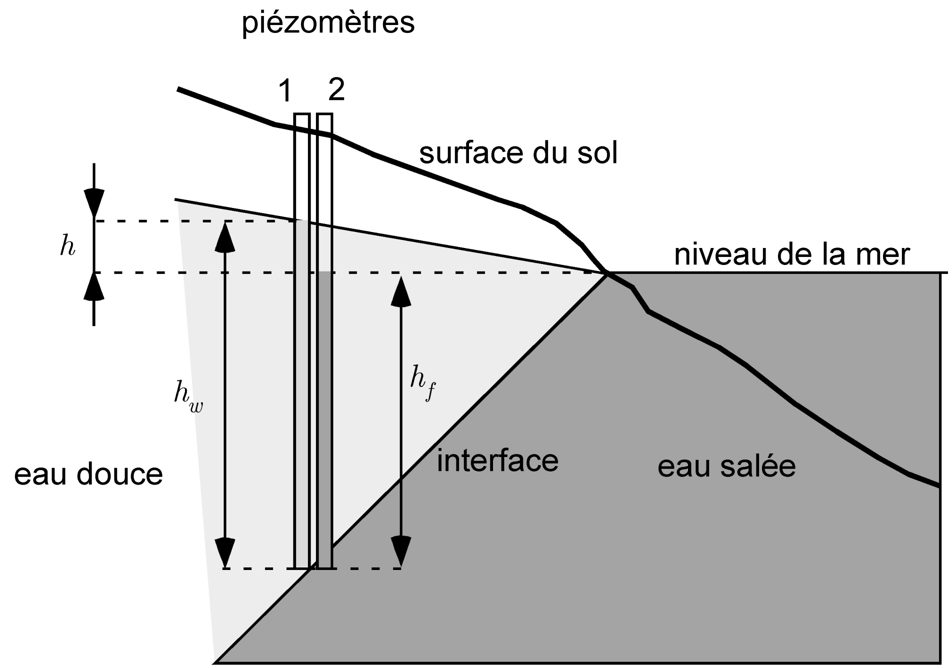


Schéma de l'intrusion d'un coin salé à l'équilibre dans une nappe côtière phréatique, sans recharge ni pompages, en symétrie plane (la coupe est transverse au trait de côte).

Question(3/3): nappe côtière phréatique

Principe de Ghyben-Herzberg

1) On suppose l'équilibre hydrostatique.
Ecrire la relation entre p_w et h_w.
Ecrire la relation entre p_f et h_f.



2) On suppose que la pression est continue à travers l'interface.
Ecrire la relation entre h_w et h_f.

3) On note h l'élévation de la surface libre au-dessus du niveau de la mer.
Exprimer h_f en fonction de h.

ρ_w : masse volumique de l'eau douce

p_w : pression dans l'eau douce

h_w : hauteur de colonne d'eau douce dans le piézo 1

ρ_f : masse volumique de l'eau salée

p_f : pression dans l'eau salée

h_f : hauteur de colonne d'eau salée dans le piézo 2

Application numérique:

$$\rho_w = 1000 \text{ kg/m}^3$$

$$\rho_f = 1025 \text{ kg/m}^3$$

Stéphane Bonelli