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1 **A combined mixing model for high-frequency concentration-discharge relationships**

2

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7

8 **Highlights**

- 9 • A new combined mixing model for concentration-discharge relationships.
- 10 • A formula combining dynamics of the discharge, with dynamics of the ion concentrations.
- 11 • An unusual approach of hydrograph separation.
- 12 • How coupling a time dynamic hydrological model with static $C - Q$ relations.

13

14 **Abstract**

15 Streamflow is the major factor influencing the evolution of solute concentration in river water and
16 different modelling approaches exist to characterize the dependency of ion concentration to
17 discharge: the simplest are based on measurable quantities (stream discharge and stream ion
18 concentration) but do not allow for an explicit, physical, flow-path interpretation; the more complex
19 are based on mixing assumptions with different end-members sources, but require the knowledge of
20 (unmeasurable) flow components. We present here a new concentration-discharge model, which
21 associates a classical concentration-discharge relationship with a classical two-component mixing
22 equation. The originality of our approach lies in the fact that we do not proceed in the usual way to
23 perform the hydrograph separation: we use an *a priori* assumption of the baseflow-quickflow
24 separation to infer the source concentration values, contrarily to the usual (inverse) approach. The
25 other notable originality is that all the parameters of this model depend on the temporal variation of
26 the stream discharge. This combined model was tested on high-frequency ion concentration series
27 from the ORACLE-Orgeval observatory (France). This work demonstrates that high temporal
28 resolution data allows for explicit testing of model performance across different hydrologic scales.
29 Results show that the combined mixing model allows a better estimation of streamflow solute
30 concentration series for most ions tested at inter-annual scale, except for nitrate (which do not
31 exhibit a clear $C - Q$ relationship). Our results also confirm the advantage of coupling a time
32 dynamic hydrological model with static $C - Q$ relations for each of the flow components.

33 Keywords

34 Concentration-discharge relationships; high-frequency measurements; mixing equations.

35

36 1. Introduction

37 Concentration-discharge ($C - Q$) relationships are interesting for a variety of purposes and potential
38 users and because they employ simple approaches to describe complex hydro-chemical interactions,
39 hydrologists and geochemists have been using and exploring them for over 70 years (see Chanut et
40 al., 2002; Durum, 1953; Hem, 1948; Johnson et al., 1969; Kirchner, 2019; Moatar et al., 2017). The
41 established relations can be purely empirical, used to infill lacking concentration data and compute
42 fluxes over long time periods, or to characterize the regime export of chemical components in a
43 number of catchments and empirically derive heuristic patterns on a regional and global scale (e.g.
44 Bieroza et al., 2018; Meybeck and Moatar, 2012; Moatar et al., 2017). In some cases, the relations
45 have been derived from a more complex model, describing a physical representation of water
46 circulation and composition, from various mixing sources, with the objective to derive relevant
47 information on internal functioning (Johnson et al., 1969).

48 To model $C - Q$ relationships, two classical approaches exist:

- 49 • The temporal $C - Q$ patterns relationship (i.e. fit parameters of C vs Q , Musolff et al. (2017))
50 try to explain the processes controlling the mobilization and delivery of chemical elements
51 into streams (i.e. export regimes): dilution, flow-enhancement, or no-variation at all (see e.g.
52 Salmon et al., 2001), as well as biogeochemical transformations in river networks (Minaudo
53 et al., 2019). These models are, first and foremost, based on observations of concentrations
54 relatively stable over time, as electro-conductivity (EC) or chlorides (Durum, 1953; Hem,
55 1948). Often, the main objective is to identify the groundwater contribution to streamflow
56 (Durum, 1953). These models are based on measurable quantities (discharge and in-stream
57 chemical concentrations) (Durum, 1953; Hem, 1948; Tunqui Neira et al., 2020) (see example,
58 Eq. (1) and Eq. (2) in Table 1).
- 59 • The more complex n -component models have in addition the aim of quantifying the sources
60 of the chemical concentrations measured in the river (Barthold et al., 2011). Introduced at
61 the end of the 1960s (Hubert et al., 1969; Johnson et al., 1969; Pinder and Jones, 1969), they
62 are based on the chemical contribution of hydrological sources (i.e. groundwater, runoff,
63 precipitation), assuming each one can be characterized by a constant concentration and
64 assuming the existence of a methodology to identify the relative contribution of each source
65 to streamflow. This mixing approach is based on the mass balance equation (see its simplest

66 expression, Eq. (3), Table 1). Hall (1970) gives an exhaustive presentation of these models,
 67 their different expressions and assumptions. The mixing model was applied at different time
 68 scale from flood and event scale (Hubert et al., 1969; Pinder and Jones, 1969) to annual scale
 69 (Johnson et al., 1969) and for different purposes such as hydrograph separation (Pinder and
 70 Jones, 1969), understanding of flowpaths in catchment (Johnson et al., 1969), or the
 71 dynamics of the components of storm flow (Hubert et al., 1969).

72 Table 1: Classical $C - Q$ relationships used in literature

Approach	Equation	Number of parameters	Formula	Eq. n°
Temporal $C - Q$ patterns	Power-law	2	$C = aQ^b$	Eq. (1)
	Two-sided power-law equation	3	$C^{\frac{1}{n}} = a + bQ^{\frac{1}{n}}$	Eq. (2)
Mixing end-member sources	mixing model (<i>with at least two sources which can be simplified with base flow and quick flow</i>)	at least 2	$C = C_b \frac{Q_b}{Q} + C_q \frac{Q_q}{Q}$	Eq. (3)
<i>With</i> C : total streamflow ion concentration at time t (mgL^{-1}) Q : total streamflow (m^3s^{-1}) at time t a, b, n : parameters Q_b : base flow (m^3s^{-1}) Q_q : quick flow (m^3s^{-1} , $Q_q = Q - Q_b$) C_b : ion concentration of the base flow (mgL^{-1}) C_q : ion concentration of the quick flow (mgL^{-1})				

73
 74 Despite their simplicity, the power-law models can yield excellent fits for some ions, explaining up to
 75 90 % of the variance of the concentrations. For this reason, they are still widely used today (Barco et
 76 al., 2008; Godsey et al., 2009; Moatar and Meybeck, 2007; Probst and Bazerbachi, 1986). However,
 77 several authors have criticized the power-law models, underlining that they lump different hydro-
 78 chemical processes and dynamics, and do not allow for an explicit physical flow-path interpretation
 79 (Moatar et al., 2017; Rose et al., 2018). Godsey et al. (2009) compared different $C - Q$ models, and
 80 concluded that it is difficult to find simple generalizable models that accurately represent the typical
 81 shape of the $C - Q$ relationship, which are internally consistent, and make plausible assumptions
 82 about catchment behavior.

83 The variability of $C - Q$ relationships could be due to a large number of processes, varying in space,
 84 time and with the characteristics of the catchment. The exact mechanisms leading to $C - Q$ relations
 85 remain an open question. Both natural and anthropogenic factors affect the biogeochemical
 86 response of streams, and, while the majority of solutes show identifiable behaviors in individual
 87 catchments, only a minority of behaviors can be generalized (Botter et al., 2019). Some recent

88 studies attempt to differentiate two distinct behaviors: *chemodynamic* and *chemostatic* export.
89 Chemostatic export is defined as relatively small variations in concentrations compared to discharge
90 (Musolff et al., 2015; Thompson et al., 2011). Chemostasis could be associated with constant rate of
91 chemical weathering (Godsey et al., 2009), “a legacy storage” of anthropogenic nutrient which
92 buffer the variability in concentrations (Basu et al., 2010; Clow and Mast, 2010), or with a significant
93 hydraulic residence time compared to weathering kinetics (Ameli et al., 2017; Maher, 2011).
94 Chemostatic export processes could represent the long-term trend of basin chemistry and can
95 therefore be identified as an average of concentrations (Musolff et al., 2015). Chemodynamic
96 patterns are characterized as high variations in concentrations compared to discharge caused by
97 flushing, enrichment behavior, or dilution behavior (Musolff et al., 2015). It can be controlled: (i) by
98 activation of solute sources heterogeneously distributed in space; (ii) by threshold-driven transport
99 of constituents or (iii) high reactivity of constituents (Jones et al., 2017; Musolff et al., 2015; Vaughan
100 et al., 2017; Zhang et al., 2016). We could also note that Zhang et al. (2016) emphasized that
101 temporal patterns $C - Q$ models (including the b parameter of Eq. (1)) may vary over time or with
102 season, and a single b coefficient for the entire period of record is over-simplification and potentially
103 misleading. The other difficulty of a generalization is also the difference in behavior and source of the
104 different ions. Beyond the patterns, changes can occur whether due to weathering, meteorological
105 or anthropic processes, affecting one ion more than another (Knapp et al., 2020; Musolff et al., 2017;
106 Rose et al., 2018; Zhi et al., 2019).

107 To deal with the non-linearity of the $C - Q$ relationship, some authors have proposed to increase the
108 number of hydrological components (e.g. Evans and Davies, 1998; Probst, 1985), make specific
109 catchment calibration (Godsey et al., 2009), consider flow classes, e.g. low and high flows (Meybeck
110 and Moatar, 2012; Moatar et al., 2017) or integrate significant time steps (Kirchner, 2019). To gain
111 insight into the linkages between chemical and hydrologic processes that yield the observed
112 concentration-discharge relationship, research has often focused on only one of the modeling
113 solutions (i.e. on either power-law model or mixing model).

114 The temporal patterns (dilution, constant, enrichment) or the mass balance (end-members mixing)
115 approaches are operational and widely used to describe $C - Q$ relationships. There are also other
116 compound methods in the literature, based on solute production models (Ibarra et al., 2016; Maher,
117 2011; Maher and Chamberlain, 2014) or on end-members models (Bao et al., 2017; Bouchez et al.,
118 2017; Zhi et al., 2019). However, none offers a parsimonious and generalized approach as proposed
119 in this paper. To date, no simple conceptual model still allows combining a dynamic of
120 concentrations with a dynamic of water flow. The new model proposed in this paper is trying to solve
121 this problem, by combining the two approaches (i.e., power-law and mixing model). The originality of

122 our approach is that we do not proceed by assuming an end-member concentration value to perform
 123 the baseflow-quickflow separation, but we use instead the inverse approach, an *a priori* assumption
 124 of the baseflow-quickflow using a hydrological hydrograph separation method to infer the source
 125 concentration values. All the parameters of the model proposed depend on the temporal variation of
 126 the stream discharge. Using the high frequency series of concentrations and discharges, observed
 127 during two years on the ORACLE-Orgeval observatory (France), we have tested the performances of
 128 this new model at different hydrological time scales and on a full range of flow and concentration.
 129 The first aim of this paper is to present the model and its validation on one long high-frequency time-
 130 series. The second aim is to discuss of its performances with regards to the classical models. Note
 131 that this paper is methodological: it does not focus on the different hydro-chemical processes
 132 involved in the composition of the stream solutes or on others watershed functioning considerations.

133 2. The combined mixing model

134 The combined model tested here associates a $C - Q$ relationship with a two-component mixing
 135 equation:

- 136 • As $C - Q$ relationship, we use the two-sided affine power scaling relationship (2S-APS),
 137 which we recently proposed to fit the $C - Q$ relationship, as a natural extension of the well-
 138 known and widely-used power law relationship (see Tunqui Neira et al., 2020);
- 139 • As mixing equation, we use the simplest of the existing schemes, i.e. a two-component
 140 equation distinguishing “base flow” and “quick flow” (e.g Pinder and Jones, 1969).

141 To combine the 2S-APS relationship with the two-component mixing equation (see Eq. (2) and Eq.
 142 (3), Table 1), we propose to write the concentrations of each of components in Eq. (3) (i.e. C_b and C_q)
 143 as a function of total discharge, using the same transformation:

$$C_b = \left(a_b + b_b Q^{\frac{1}{n}} \right)^n$$

$$C_q = \left(a_q + b_q Q^{\frac{1}{n}} \right)^n$$

144 The total discharge (and not the discharge of each of the two hydraulic components) was used to
 145 seek simplification, not increasing too much the number of parameters. We will discuss later the
 146 value of this simplification. Then, the mixing equation becomes:

$$C = \left(a_b + b_b Q^{\frac{1}{n}} \right)^n \frac{Q_b}{Q} + \left(a_q + b_q Q^{\frac{1}{n}} \right)^n \frac{Q_q}{Q} \quad \text{Eq. (4)}$$

147 The “combining” procedure merges the two most used forms of equations of the hydro-chemical
148 literature (i.e. Eq. (2) and Eq. (3)). The advantage of this new model is to improve the mixing
149 approach by introducing dynamic concentrations for the sources. Depending on the values taken by
150 parameters a and b in Eq. (4), three schematic situations can be identified, briefly described below
151 and illustrated by Figure 1.

152 2.1. Case 1: chemostatic components ($b_b = b_q = 0$)

153 In this case, C_b and C_q are constant and independent of river discharge. Thus, C is only influenced by
154 the discharge components dynamics (Q_b and Q_q) (see Case 1 in Figure 1). The case 1 represents the
155 chemostatic export characterizing a catchment controlled by large legacy stores (see Musolff et al.,
156 2015). It corresponds to the assumption of the classical mixing equation approach, where the C_b and
157 C_q values can assume for example the average of concentrations, measured respectively during the
158 dry and wet seasons (i.e. Saraiva Okello et al., 2018; Stewart et al., 2007; Zhang et al., 2013).

159 2.2. Case 2: single 2S-APS relationship ($a_b = a_q = a$ and $b_b = b_q = b$)

160 This case reduces to $C_b = C_q$ (Case 2 in Figure 1) and to the simple $C - Q$ relationship:

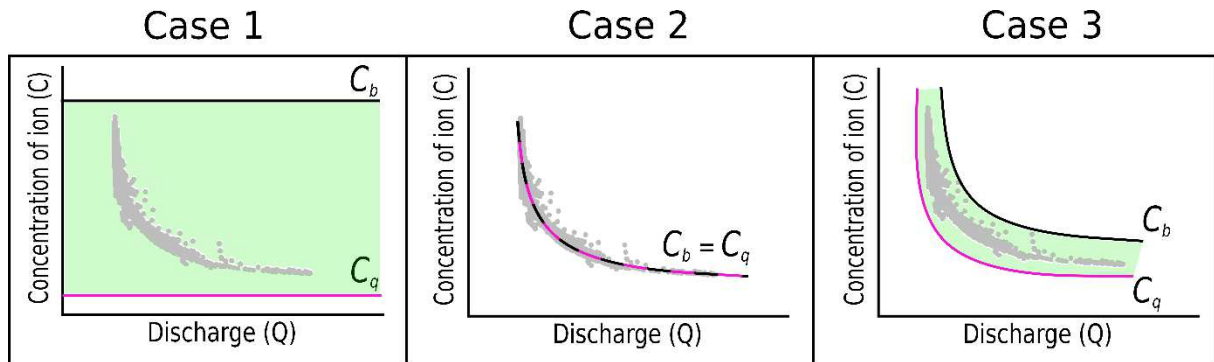
$$161 C = \left(a + bQ^{\frac{1}{n}} \right)^n$$

162 The observed concentration is only a function of discharge.

163 2.3. Case 3: General case (a and b are different)

164 This is the general case of the transformed mixing equation (Eq. (4)). Unlike the classical mixing
165 equation (chemostatic components), the C_b and C_q values are not constant but vary individually as a
166 function of stream flow (see Case 3 in Figure 1). The general case allows accounting for the temporal
167 variation of the chemical components. It allows connecting concentration with total discharge and
168 the hydrograph separation components (Q_b and Q_q , i.e. with $Q_q = Q - Q_b$, see Table 1).

169



170

171 Figure 1: The three hypothetical $C - Q$ scenarios due to the values taken by parameters a and b of
 172 Eq. (4): case 1, case 2 and case 3. The green hatching represents the probable C_{sim} yields by our
 173 model in the $C - Q$ space.

174 3. Material and methods

175 3.1. Study site and datasets

176 The combined model (Eq. (4)) was applied to the high-frequency hydro-chemical dataset measured
 177 at the Oracle-Orgeval Observatory (Tallec et al., 2015) by the *River Lab* (Floury et al., 2017). The
 178 Oracle-Orgeval Observatory is a small catchment located 70 km east of Paris, France. It is subjected
 179 to a temperate and oceanic climate, with annual average temperature of 11 ± 1 °C and a mean
 180 annual rainfall of 674 ± 31 mm (Tallec et al., 2013). The average measured streamflow at the
 181 Avenelles outlet (sub-catchment of 46 km² and location of the *River Lab* on the Oracle-Orgeval
 182 Observatory) is about 0.2 m³/s (1962 - 2017), with minimum flows in summer (< 0.1 m³/s) and floods
 183 up to 10 m³/s in winter and spring. With respect to geology, the catchment is underlain entirely by
 184 limestone rocks, with two aquifers: the shallower aquifer of the Brie limestone and the deeper
 185 Champigny limestone aquifer (Mouhri et al., 2013). Land use is mostly agricultural with few villages,
 186 and with intensive farming practices, mainly based on mineral nitrogen fertilization (Garnier et al.,
 187 2016). Nearly 60% of the surface of the catchment is drained with tile drains.

188 Among all ions measured every 30 minutes by the *River Lab* laboratory, we used 3 ions (see Table 2)
 189 whose behavior exhibit notable differences in the Orgeval catchment: sulfate, nitrate and chloride.
 190 Chloride mainly come from rain inputs during the wet season and to a smaller extent from fertilizers
 191 (Floury et al., 2018). Sulfate comes from the chemical weathering of gypsum, which makes it highly
 192 variable, depending on the season and the leaching of concerned localized underground layers
 193 (Floury et al., 2018; Mouchel et al., 2016). Nitrate mostly come from agricultural activities and
 194 fertilizers inputs, with specific seasonal leaching rates (Garnier et al., 2016). Finally, we also used
 195 electro-conductivity reflecting the presence of all ions in stream water (EC, see Table 2). The main
 196 data set (flow rates and chemical concentrations) covers the period between June 2015 and March
 197 2018 (Table 2), i.e. 20,700 measurements over 33 months. To perform a split-sample test, according

198 to Klemeš (1986), we divided the dataset into two periods. Table 2 presents a first period used for
 199 the model calibration and a second period for the model validation.

200 Table 2: High-frequency measurements of chemical concentrations (average, minimum, maximum
 201 values and difference between quantiles 90 and 10 divided by the mean (CV)) from the *River Lab* at
 202 the Avenelles outlet, Oracle-Orgeval Observatory (June 2015 to July 2017 for calibration period and
 203 August 2017 to March 2018 for validation period).

Solute	Unit	Calibration period (June 2015 to July 2017)			
		Mean	Min	Max	CV
Sodium	mg.L ⁻¹	13	2	17	0.12
Sulfate	mgS.L ⁻¹	19	2	32	0.19
Chloride	mg.L ⁻¹	30	4	40	0.15
EC	μS.cm ⁻¹	704	267	1015	0.11
Validation period (August 2017 to March 2018)					
Sodium	mg.L ⁻¹	13	3	17	0.24
Sulfate	mgS.L ⁻¹	18	3	26	0.27
Chloride	mg.L ⁻¹	29	4	40	0.29
EC	μS.cm ⁻¹	576	171	813	0.25

204

205 3.2. Hydrograph separation

206 To apply the combined mixing model (Eq. (4)), we have to separate the hydrograph in order to
 207 compute the values of base flow (Q_b) and quick flow (Q_q). To perform the baseflow-quickflow
 208 separation (because we do not know the source concentration values) we use an *a priori* assumption
 209 of the baseflow-quickflow separation to infer the source concentration values (and not the usual
 210 inverse approach).

211 In this study, we use the Recursive Digital Filter (RDF) hydrograph separation approach. The RDF
 212 approach, adapted in the late 1970s from the signal-processing theory, is widely applied for
 213 hydrograph separation. Indeed, RDF methods are computationally efficient, easily automated and
 214 applied to long continuous streamflow records (Chapman, 1991; Eckhardt, 2005). Among all the RDF-
 215 methods existing in literature (Brodie et al., 2007 p.62), we used the well-known Lyne-Hollick method
 216 (LH-RDF method) (Lyne and Hollick, 1979; Nathan and McMahon, 1990). Base flow is considered here
 217 as a low-frequency signal, and surface runoff as a high-frequency signal. By filtering out the high-
 218 frequency signal, the low-frequency signal (i.e. base flow) can be revealed (Longobardi and Loon,
 219 2018; Nathan and McMahon, 1990).

220 The LH-RDF method is defined as follows:

$$Q_{b(t+1)} = \min\left(\alpha_\tau Q_{b(t)} + \frac{1-\alpha_\tau}{2}(Q_{(t+1)} + Q_{(t)}), Q_{(t)}\right) \quad \text{Eq. (5)}$$

221 where Q_b , Q , α_τ and t are respectively, the baseflow, the total flow, the LH-RDF filter parameter and
222 the time.

223 Iterative application of the filter allows smoothing data and nullifying phase distortion. We have used
224 the forward-backward-forward application proposed by Nathan and McMahon (1990). The LH-RDF
225 method is characterized by one parameter (α_τ) which defines the speed of convergence of the filter.
226 It is a common practice (e.g. Longobardi et al., 2016; Zhang et al., 2017) to adapt the filter parameter
227 (α_τ) to the hydrological recession time constant of the catchment (τ). Otherwise, either Q_b or Q_q
228 would have an unwanted behavior on a seasonal time scale (too slow convergence of Q_b or too fast
229 decrease of Q_q during flood events). Another important reason for this adaptation, is that the default
230 value of α_τ (0.925, proposed by Nathan and McMahon, 1990) applied in small catchments controlled
231 by the regional scale factors such as slope and shape, has shown poor performance when computing
232 Q_b (Ladson et al., 2013; Zhang et al., 2017).

233 During seasons without significant recharge, stream flow may recess exponentially and follows the
234 form:

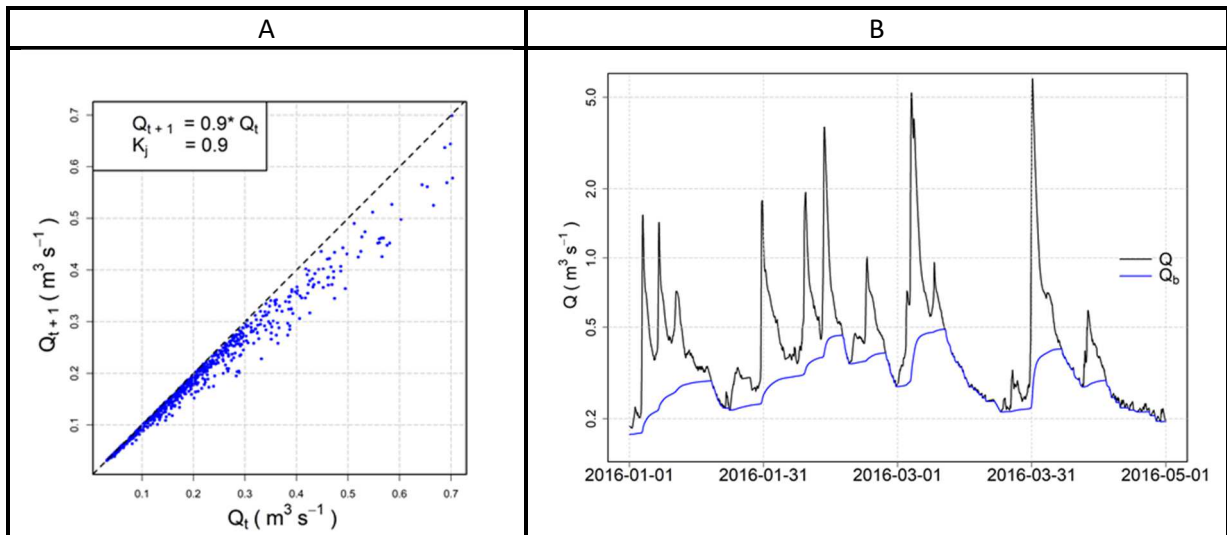
$$Q_{(t+\Delta t)} = Q_{(t)} \cdot \exp\left(-\frac{\Delta t}{\tau}\right) = Q_{(t)} \cdot K \quad \text{Eq. (6)}$$

235 where K is the so-called recession constant of the catchment.

236 The recession constant K can be obtained using the master recession curve (MRC) approach (Nathan
237 and McMahon, 1990). A linear regression (i.e. plot Q_{t+1} vs Q_t see Figure 2A) allows establishing the
238 recession constant K , which also represents the α_τ parameter of LH-RDF method (Eq. (5)). This
239 analysis was applied on daily flow data of the Avenelles station from January 1, 2000 to September,
240 2018 as follows: the daily stream flow data of several seasons were overlapped according to the day
241 of the year, starting from the beginning of June, and assuming that the stream flow decreases under
242 a continuous recession process for the period June-September. The K value obtained about 0.90 has
243 been calculated for a daily discharge (see Figure 2A). However, for high frequency discharge
244 measurements (i.e. time step of 30 min or 0.5 hours), this value must be transformed at the
245 appropriate time step as proposed by Eckhardt (2008) :

$$K_{0.5h} = (K_j)^{0.5/24} = 0.99$$

246 With the LH-RDF method (i.e., Eq. (5)) and the calibrated value of K (i.e. 0.99 for the Avenelles
247 catchment), we computed the values of the base flow (Q_b) and the quick flow (Q_q) (see example of
248 the results Figure 2B).



249 Figure 2: A. Scatter plot of daily discharge Q_{t+1} against Q_t during recession periods, Avenelles
 250 station. Dashed black line: line through origin with slope $K_j = 0.90$. B. Example of hydrograph
 251 separation obtained with the LH-RDF method and the constant recession value $K_{0.5h}$ (i.e., 0.99),
 252 Avenelles station, with blue line as the baseflow and black line the total flow.

253 We should not fail to mention here that the baseflow-quickflow methods have been widely criticized:
 254 see e.g. Beven (1991) for a vibrant indictment against them and the arguments raised by Pelletier
 255 and Andréassian (2019) in the discussion of their paper, for a careful use of them. We do not wish to
 256 enter this debate. We will not claim to have identified a precise physical pathway and in this paper,
 257 we will thus limit our ambition to the identification of what Pelletier and Andréassian (2020) have
 258 named the “not-too-delayed” and the “delayed-enough” flow components (baseflow and quickflow).

259 3.3. Calibration of n , a and b parameters

260 The combined mixing model (Eq. (4)) was applied with the baseflow calculated by the LH-RDF
 261 method. For each ion and the EC, we used the parameter n previously determined by Tunqui Neira et
 262 al. (2020) on the same data set, but without flow separation.

263 The extremely large number of values in this high-frequency dataset may cause problems for a
 264 robust identification of (a, b) parameters, over the full range of discharges using a simple linear
 265 regression. Indeed, the largest discharge values are in small numbers (in our dataset only 1% of
 266 discharge values are in the range $[2.6 \text{ m}^3 \text{s}^{-1}, 12.2 \text{ m}^3 \text{s}^{-1}]$, and they correspond to the lowest
 267 concentrations). To address this question, we successively tested a large number of (a, b) pairs from
 268 Eq. (4) (n remaining fixed). Each pair yields a series of simulated concentrations (C_{sim}) that can be
 269 compared with the observed concentrations (C_{obs}).

270 Among the many numerical criteria that could be used, we chose the bounded version of the Nash
 271 and Sutcliffe (1970) efficiency criterion $NSEB$ (Mathevet et al., 2006), which is commonly used in
 272 hydrological modeling. $NSEB$ can be computed on concentrations or on discharge-weighted

273 concentrations (which corresponds to the load). We chose the average of both, because we found
 274 that it allows more weight to be given to the lowest concentrations and thus to avoid the issue of
 275 under-representation of high-discharge/low-concentration measurement points. If other criteria
 276 exists to test the model performances, the $NSEB_{comb}$ has the advantage to privilege neither the load
 277 nor the concentration and integrates both in a single criterion. Table 3 presents the formula for these
 278 numerical criteria. We retained as optimal the pair of (a,b) that yielded the highest $NSEB_{comb}$ value
 279 (we explored in a systematic fashion the range [1–5] for a and [-1.2–1.2] for b).

280 Table 3: Numerical criteria used for optimization (C_{obs} – observed concentration, C_{sim} – simulated
 281 concentration, Q – observed discharge). The Nash and Sutcliffe (1970) efficiency (NSE) criterion is
 282 well known and widely used in the field of hydrology. The rescaling proposed by Mathevet et al.
 283 (2006) transforms NSE into NSEB, which varies between -1 and 1 (its optimal value). The advantage
 284 of this rescaled version is to avoid the occurrence of large negative values (the original NSE criterion
 285 varies in the range $[-\infty, 1]$).

$NSE_{conc} = 1 - \frac{\sum_t (C_{obs}^t - C_{sim}^t)^2}{\sum_t (C_{obs}^t - \overline{C_{obs}})^2}$	Eq. (7)
$NSEB_{conc} = \frac{NSE_{conc}}{2 - NSE_{conc}}$	Eq. (8)
$NSE_{load} = 1 - \frac{\sum_t (Q^t C_{obs}^t - Q^t C_{sim}^t)^2}{\sum_t (Q^t C_{obs}^t - \overline{Q C_{obs}})^2}$	Eq. (9)
$NSEB_{load} = \frac{NSE_{load}}{2 - NSE_{load}}$	Eq. (10)
$NSEB_{comb} = \frac{1}{2} (NSEB_{conc} + NSEB_{load})$	Eq. (11)

286
 287 For each ion and for EC, Table 4 presents the results obtained for the n, a and b parameters
 288 calibrated for the Avenelles sub-catchment and their corresponding optimal $NSEB_{comb}$ criterion.
 289 Although the parameters of the combined mixing model are shown here, we will not discuss them in
 290 this paper. Indeed, for now, without generalized studies covering several catchments it is not
 291 possible to interpret them physically. Note that for these reasons we have chosen to present Table 4
 292 in this section and not in the Results and discussion section.

293

294 Table 4: Values set for n in the combined model and (a_b, b_b) and (a_q, b_q) parameters for each case
 295 and solute. Values obtained for optimal $NSEB_{comb}$ criterion. Note that case 1 corresponds to
 296 chemostatic components, case 2 to the single 2S-APS relationship and case 3 to general case of the
 297 combined model. $NSEB_{comb}$ is optimum in 1.

Solute	n	Case	a_b	b_b	a_q	b_q	$NSEB_{comb}$
Sulfate	5	1	1.9	0	1.3	0	0.46
		2	2.2	-0.55	2.2	-0.55	0.69
		3	2.3	-0.7	2	-0.4	0.73
Nitrate	5	1	1.8	0	1.6	0	0.39
		2	1.8	-0.1	1.8	-0.1	0.41
		3	2.3	-1.2	2.1	-0.3	0.45
Chloride	3	1	3.3	0	1.6	0	0.52
		2	3.7	-1	3.7	-1	0.83
		3	3.6	-0.7	3.3	-0.8	0.86
EC	5	1	3.8	0	3.1	0	0.61
		2	4.2	-0.7	4.2	-0.7	0.77
		3	4.2	-0.7	3.9	-0.5	0.83

298

299 3.4. Performances of the model

300 We evaluate the combined model performance in calibration and validation mode. Due to the
 301 temporal density of the dataset, we have tested the performances of the models in calibration mode
 302 on widely different discharge ranges: over the entire calibration period and for selected storm events
 303 (from June 2015 to July 2017, see Table 2, Chapter 3.1). In validation mode, the performances have
 304 only been assessed over the entire period (from August 2017 to March 2018, see Table 2, Chapter
 305 3.1).

306 The *bias* and the standardized root mean square error (*sRMSE*) allow assessing respectively
 307 accuracy and precision of the combined mixing model. Table 5 presents the formula for these
 308 numerical criteria.

309 Table 5: Numerical criteria used for model performance comparison (C_{obs} – observed concentration,
 310 C_{sim} – simulated concentration, t – the time step, N – the number of observed concentration)

$$bias (\%) = 100 * \frac{\sum_t (C_{sim}^t - C_{obs}^t)}{\sum_t (C_{obs}^t)} \quad \text{Eq. (13)}$$

$$sRMSE (\%) = 100 * \frac{\sqrt{\frac{1}{N} \sum_t (C_{obs}^t - C_{cal}^t)^2}}{C_{obs}} \quad \text{Eq. (14)}$$

311

312 4. Results and discussion

313 4.1. Performance of the combined model over the entire period

314 Table 6 presents the evaluation of the combined mixing model, in calibration and validation mode
 315 over the entire period. Whatever the mode (calibration or validation) and whatever the solute
 316 species considered, the optimal *sRMSE* and *bias* values are obtained for the general case (Case 3,
 317 see Table 6). The results also show that a single 2S-APS relationship (Case 2) explains better the
 318 variations of the stream water concentrations than a mass balance equation with constant
 319 concentration components (Case 1) (see *sRMSE* and *bias* values from case 1 to case 2, Table 6).

320 Table 6: Values obtained for the *bias* and the standardized *RMSE* (*sRMSE*) for each case and solute,
 321 for an application of the combined model over the entire period. Note that case 1 corresponds to
 322 chemostatic components, case 2 to the single 2S-APS relationship and case 3 to general case of the
 323 combined model.

Solute	Case	Calibration mode		Validation mode	
		<i>sRMSE</i> %	<i>bias</i> %	<i>sRMSE</i> %	<i>bias</i> %
Sulfate	1	18.3	7.3	24.1	11.7
	2	11.5	3.8	14.8	-11.9
	3	11.3	0.4	10.0	-6.7
Nitrate	1	48.9	46.7	317	315
	2	31.8	30.4	267	266
	3	27.6	-11.2	144	119
Chloride	1	13.8	-4.5	20.0	-0.7
	2	6.3	-0.7	12.6	-3.3
	3	5.3	-0.3	12.1	-0.5
EC	1	10.3	1.5	16.8	7.9
	2	6.0	2.4	9.7	0.5
	3	5.2	-0.5	9.6	-2.7

324 This means that - within the adopted modelling framework and in as much as the hydrograph
 325 separation can be considered as hydrologically relevant - the concentrations of the flow components
 326 cannot be considered constant across time (i.e., the behavior of the Avenelles catchment is non-
 327 chemostatic). The stream water quality of the Avenelles sub-catchment appears strongly influenced
 328 by discharge processes.

329 The most evident improvement of the case 3 model is observed for chloride and EC (see *sRMSE* of 5
 330 % with a negative *bias* less than 0.5% in calibration mode, Table 6). Less importantly, sulfate
 331 *sRMSE* is also much improved when variable concentrations are introduced in the quick and slow
 332 components (see *sRMSE* around 10%, Case 3, in calibration and validation mode Table 6). In
 333 calibration mode, the sulfate *bias* is comparable to that obtained for chloride and EC (less than 0.5%,
 334 see Table 6).

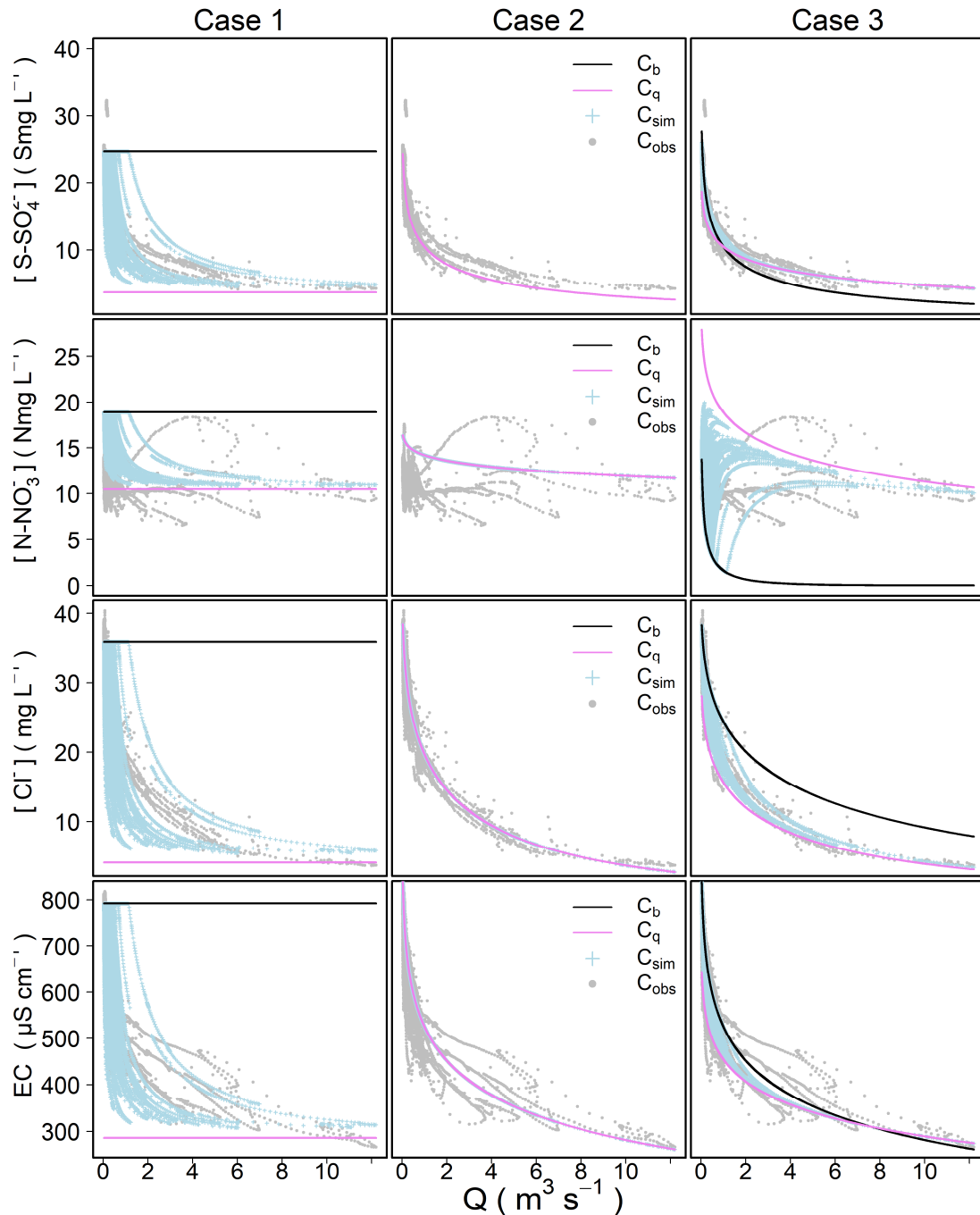
335 According to Flourey et al. (2018), chloride would come from the Brie aquifer with mainly external
336 input from rainfall during the wet season. According to Mouchel et al. (2016), the variability of
337 sulfate concentrations is related to the spatial heterogeneity of gypsum lenses and to the temporary
338 variability of the water table heights. The spatial and temporal variability of sulfate and chloride
339 concentrations is due to several chemo-dynamical processes, well modeled by the general case (case
340 3).

341 The lowest values of the general case are obtained for nitrate (see *sRMSE* about 27.6% and a *bias*
342 about -11.2%, case 3, in calibration mode, and both > 100 % in validation mode, Table 6). Nitrate
343 present the largest seasonal variability, strongly related to farming practices (Garnier et al., 2016)
344 and chemo-dynamic processes especially in the hyporheic-zone (Flourey et al., 2018). This larger
345 variability, either environmental or anthropogenic, cannot be simulated by our model, whatever the
346 case.

347 Figure 3 illustrates the simulations of the models, in the first period of calibration, in regard with the
348 observed time series. As theoretically expected (see Figure 1), the simulated concentrations are more
349 or less dispersed, from linear case 2 to very dispersed case 1 (see Figure 3). The case 3, as shown
350 previously by the *sRMSE* and the *bias* (see Table 6), is the one closest to the observed
351 concentrations. For the nitrate, while cases 1 and 2 seem to overestimate the concentrations, case 3
352 has the advantage, despite its poor performance, of covering all the nitrate concentration values.

353 A significant feature, common to almost all ions (except nitrate and sulfate) and cases of the model,
354 is the lower C_q concentration, compared to C_b (see black lines vs pink lines, Figure 3). For the nitrate
355 ion, C_q concentration becomes a more important contributor in the three cases, until becoming
356 preponderant in the case 3 (see nitrate, pink line Figure 3). For sulfate, C_q is the main contributor to
357 the stream concentration for discharges greater than $\sim 0.5 \text{ m}^3 \cdot \text{s}^{-1}$ (see sulfate, pink lines Figure 3).

358 This result confirm studies carried out at the Orgeval-ORACLE observatory (Billy et al., 2013; Flourey et
359 al., 2018; Garnier et al., 2016; Mouchel et al., 2016; Mouhri et al., 2013). Indeed, according to the
360 results, the principal contributor to the stream concentration in the Avenelles catchment would be
361 first groundwater (i.e. C_b) for chloride and EC. During the wet season (i.e. for the discharge beyond
362 $0.5 \text{ m}^3 \cdot \text{s}^{-1}$) for the sulfate and whatever the season for nitrate, the principal contributor is the
363 quickflow concentrations (i.e. C_q). However, the combined mixing model does not seem to be able to
364 consider the quickflow concentrations in a good way; i.e. integrating either the variations of the sub-
365 surface hydrological compartment or the variations of concentrations linked to this compartment.



366

367 Figure 3: Simulated concentration (C_{sim} , in blue) obtained of each case of our combined method
 368 plotted in the $C - Q$ space for the three ions and EC in the calibration period. In addition, we have
 369 plotted the computed values for C_b (black line) and C_q (pink line) for each case studied. Note that in
 370 case 2 the lines of C_b and C_q overlap each other. Grey points represent the observed concentrations.

371 4.2. Performances of the combined model for selected storm events

372 We selected two representative flood events of short duration, at the end of fall (November 2015,
 373 low groundwater level, low soil moisture and no contribution of tile drains, see Figure 4A) and during
 374 winter (February 2016, high groundwater level, high soil moisture and tile drains contribution, see
 375 Figure 4B). A longer wet season flood event of March 2016, well covered during 15 days by the

376 concentration data set, is also added (see Figure 4C). Figure 4 presents observed data and the three
377 cases of simulations with the combined mixing model as a function of time for the three events. For
378 the three selected storm events, Table 7 presents the *sRMSE* and *bias* values obtained in a
379 calibration mode with the three cases of the combined model.

380 At the event scale, except for nitrate during the dry season, we obtain the same results than with the
381 entire period: the case 3 shows the optimal *sRMSE* and *bias* values for all ions and EC (see *sRMSE*
382 and *bias* from case 1 to case 3, Table 7). The potential advantage of the general case of the
383 combined mixing model (case 3) is to decouple rising and decreasing flow periods with changing
384 Q_q/Q and Q_b/Q ratios (see hydrograph separation, Figure 4). The recursive filter is efficient in
385 separating the base flow from the quick flow. It simulates a quick response of the quick flow at the
386 beginning of flood periods, and a progressive increase of the base flow during the second part of
387 flood periods. Accordingly, the two components model improves the simulation of complex
388 concentration changes during flood events.

389 We notice that the best results are obtained for the longest flood period: the performances of the
390 case 3 are improved for the long event compared to short events (see *sRMSE* and *bias* from short
391 event to long event during wet period, Table 7). As shown in Figure 4C, because of the longer
392 duration of the flood, the recursive filter simulates important variations of the Q_q/Q_b ratio during the
393 event: during most of peak flow period, most of the discharge is due to the quick flow component,
394 while the quick flow component turns back to zero after day 10. As expected from the wider range of
395 Q_q/Q_b ratio, the model simulates more difference between concentrations during the rising part of
396 the event than the descending part (Figure 4C).

397 We also observe that during short events, the simulated concentrations are slightly shifted compared
398 to the observed concentration (see example for EC, Figure 4). This comes from the limit of our
399 hydrograph separation filter in a flood with a longer duration, the bias due to the time lag is less
400 significant and a wider distribution of the Q_q/Q_b ratio, as simulated by the RDF method, is expected
401 (see Figure 4).

402 During the wet and dry hydrological seasons, almost all ions behavior (except for nitrates) and EC
403 show a dilution pattern with water of lower concentration followed by a step of increasing
404 concentration (see dotted black line C_{obs} , Figure 4). A common pattern with three stages can be
405 observed in most storm events.

406 During the first stage we observed a slight increase in concentration during the initial increase in the
407 discharge; then, in the second stage, which is generally short in time, the concentrations strongly

408 decrease while the discharges quickly increase. In the third stage, both concentrations increase again
409 while discharges continue to decrease and once more reach a lower flow and higher concentration
410 (see Figure 4). To discuss the performances obtained for each case of our combined model, we use
411 the stages outlined above.

412 The first stage would correspond to a pre-event pattern during which the concentrations come
413 mostly from the groundwater pool (Evans and Davies, 1998; Rose et al., 2018). The following stages
414 correspond to the emergence of soil waters and/or runoff mixing in varying proportions during the
415 event (Evans and Davies, 1998; Rose et al., 2018) (see Figure 4). The contribution of these pools (and
416 the dilution that accompanies it) persists for some time after the peak flow, but in increasingly
417 weaker proportions compared with those of the groundwater. Note that in the dry season, for all
418 ions and EC, the soft dilution stage (first stage) is interrupted by rapid and abrupt dilution (Figure 4).
419 This last dilution would correspond to a larger proportion of rain water; it is indistinguishable during
420 the wet season, because rain water is mixed with water from tile drains during this period of the year
421 (Billy et al., 2013).

422 In all storm events (dry and wet), for all ions (except nitrate) and EC, case 3 best simulates the third
423 stage of the storm event (where the mixing of two end members is most evident, see Figure 4). In the
424 second stage of the wet season storm events (Figure 4B and C), we can observe that for the chloride
425 and sulfate ions and EC, the 3 cases have an adequate performance (although none manages to
426 simulate the maximum dilution point, see Figure 4). In the dry season storm event (Figure 4A), the
427 second stage of chloride and sulfate ions and EC is better simulated by case 2, followed nearby case
428 3; case 1 has the lowest performance of the simulations in this stage. Finally in the first stage of each
429 of the storm events for the chloride ion, sulfate ion and EC we note the same dynamics that occurred
430 with the second stage for case 1 and 2: case 3 performs better C_{obs} during the 2 storm events of the
431 wet season, and case 2 performs better in the storm event of the dry season (see Figure 4A).

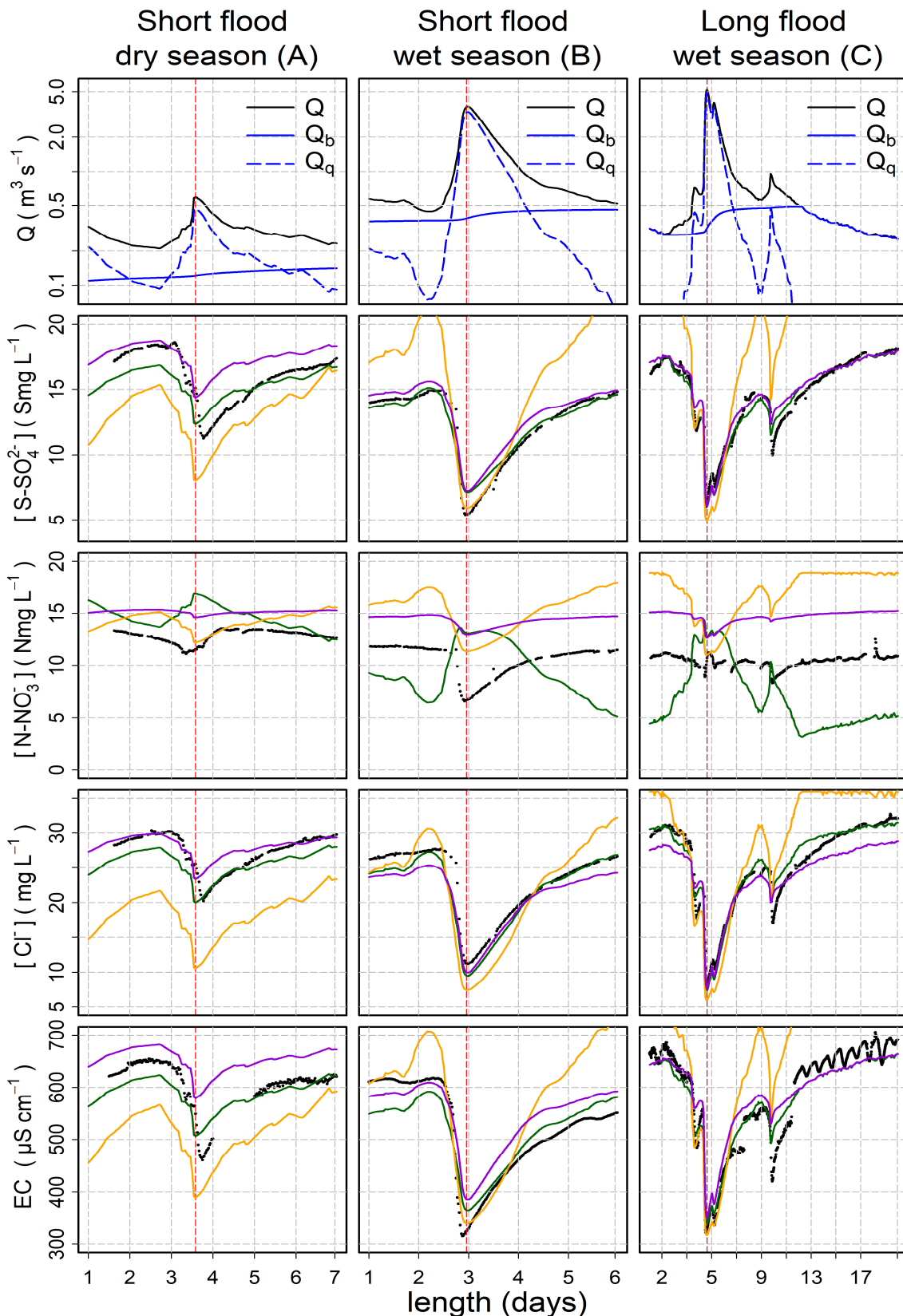
432 From this analysis we can deduce that in the dry season (Figure 4A), the first and second stages are
433 almost completely dominated by the groundwater pool (both in terms of concentration and
434 discharge), minimizing the other contributing pools (i.e. soil, runoff, represented by C_q and Q_q). For
435 this reason case 2 (assuming total discharge $Q \approx Q_{groundwater}$) has the better performance in
436 simulating the C_{obs} of chloride and sulfate ions in addition to EC. However, in the wet season events
437 (Figure 4B and C), and although in the first and second stage the groundwater pool is still the main
438 contributor, the contribution of the other pools (i.e. soil, runoff, represented by C_q and Q_q) are much
439 more notorious. Therefore, case 3 has better performances for chloride and sulfate ions and EC.

440 In the case of nitrates, we must acknowledge that the combined mixing model fails to fit the
 441 observed data whatever the event (see nitrates, Figure 4). Unlike other ions, nitrates are more
 442 concentrated in soils and drain water than in groundwater or rainwater. According to Garnier et al.
 443 (2014), in the Avenelles sub-catchment, sub-root nitrate concentrations average 22 mgN.L⁻¹, close to
 444 the average concentration observed in drains in the same area (26 mgN.L⁻¹). Nitrate concentrations
 445 in the Brie aquifer are only around 13.2 mgN.L⁻¹, whereas in rainfall they are about 0.75 mgN.L⁻¹
 446 (Mouchel et al., 2016; Flourey et al., 2018). For nitrate ions, a third component representing soil water
 447 would be needed as well as an appropriate parameterization of the seasonal nature of fertilizer
 448 applications. Several studies (Burns et al., 2019; Sebestyen et al., 2014) have demonstrated that for
 449 nitrate ion, more than two different concentration pools (i.e. groundwater, soil water, etc.) are
 450 involved in rising and declining limbs of hydrograph.

451 Table 7: Values obtained for standardized RMSE (*sRMSE*) and *bias* for each case and solute, for an
 452 application of the combined model on selected storm events. Note that case 1 corresponds to
 453 chemostatic components, case 2 to the single 2S-APS relationship and case 3 to general case of the
 454 combined model.

Solute	Case	Short flood event, dry season (Figure 4 A)		Short flood event, wet season (Figure 4 B)		Long flood event, wet season (Figure 4 C)	
		<i>sRMSE</i>	<i>bias</i>	<i>sRMSE</i>	<i>bias</i>	<i>sRMSE</i>	<i>bias</i>
		%	%	%	%	%	%
Sulfate	1	16.2	-19.0	16.9	22.0	32.3	37.7
	2	10.3	10.1	6.2	9.8	3.7	1.7
	3	7.9	-3.0	4.4	3.6	3.1	-0.5
Nitrate	1	13.1	10.1	37.9	45.6	58.8	64.4
	2	20.3	18.2	36.8	43.8	36.2	41.3
	3	21.2	16.5	34.4	9.4	40.2	-34.2
Chloride	1	28.4	-33.9	11.3	-8.9	16.3	13.8
	2	5.7	3.9	5.7	-7.1	7.0	-6.0
	3	6.4	-5.8	5.6	-6.2	4.9	1.3
EC	1	13.2	-15.0	8.7	8.6	16.3	16.9
	2	8.2	8.3	7.1	8.2	5.3	1.4
	3	4.2	-2.5	4.2	1.6	3.9	-1.1

455



456
 457 Figure 4: For the three selected storm events (short flood events during the dry season (November
 458 2015) (A) and wet season (February 2016) (B) and long flood event (March 2016) (C)), as a function of
 459 time (days): the flow and the hydrograph separation; the comparison for solutes and EC of observed
 460 (black line) and simulated concentrations from the Case 1 (orange line), the Case 2 (violet line) and
 461 the Case 3 (green line) of the combined model.

462 5. Conclusions

463 5.1. Synthesis

464 The new concentration-discharge model presented in this paper associates a *classical* concentration-
465 discharge relationship with a *classical* two-component mixing equation. The originality of our
466 approach lies in the fact that we do not proceed in the usual way by assuming a source concentration
467 value to perform the baseflow-quickflow separation: we use the inverse approach, and use an *a*
468 *priori* assumption of the baseflow-quickflow separation to infer the source concentration values.

469 Our approach allows a better estimation of streamflow ionic concentration series for most ions at
470 inter-annual scale, except for nitrate (which do not exhibit a clear $C - Q$ relationship), with improved
471 *bias* and *sRMSE* criteria. This shows the advantage of coupling a time dynamic hydrological model
472 with static $C - Q$ relations for each of the flow components.

473 5.2. Limits of our approach

474 The first limit of our approach is apparent for the nitrate case: nitrate are known to be poorly
475 described by concentration-discharge relationships, and our model does not allow improving much
476 the issue. The two-member hydrograph separation tested here may be a limiting factor: for nitrate
477 ions, the two components hydrograph separation (C_q and C_b) seems to be inadequate, while for
478 sulfate, chloride and EC, it seems sufficient. The base flow separation method used in this article
479 carries naturally its share of uncertainties due to the arbitrary and speculative hypotheses used in its
480 conception (Beven, 1991; Brutsaert, 2008; Cheng et al., 2016) . The "simple" separation into two
481 components cannot explain the complexity of the behavior of the nitrate ions. Indeed, several
482 studies (Miller et al., 2017; Probst, 1985) have shown that the introduction of a third component
483 (which would come to represent the soil pool component) is a prerequisite to represent the behavior
484 of nitrate.

485 We also need to underline that we acknowledge that our approach remains conceptual, and we do
486 not claim to have identified 'physically' the water masses that produce the quick and base flow.

487 5.3. Further perspectives

488 As the model does not simulate the decoupling of discharge and concentration patterns, due to a
489 short time lag explained by hydraulic mechanisms; doing so, would be an interesting extension of this
490 work. An isotope study would be helpful to this aim.

491 Another priority would be to test the implementation of our model elsewhere, especially on high-
492 frequency water quality stations (Kirchner et al., 2004). Different types of chemical signatures,
493 reflecting different types of hydrological and hydro-chemical functions underlying the transfer

494 processes, could be studied in detail, including different flow decomposition concepts and may be
495 other $C - Q$ relations possibly including specific seasonal features.

496 Another alternative would be to test this new methodology with other hydrograph separation
497 methods than the RDF Lyne-Hollick to prove their validity (e.g. the methods developed by Eckhardt,
498 2005; Pelletier and Andréassian, 2020).

499 If high-frequency measurements become more and more available for science (Kirchner et al., 2004),
500 their implementation at large scale still difficult and low frequencies measurements (i.e. daily,
501 weekly) remain the rule (Moatar et al., 2017). Thus, it remains important to continue methodological
502 development through modelling which allow us, with a limited number of measurements and a good
503 representativeness, to assess the quality of rivers in large scale, in the framework of low frequency
504 monitoring. Ultimately, further developments of the combined mixing model should go in this
505 direction.

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